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Transmittal

TO:

Delaney Peterson
Anchor QEA, LLC
720 Olive Way, Suite 1900
Seattle, WA 98101

DATE: 11/26/2019	GTX NO: 310685
RE: Gasco PDI	

COPIES	DATE	DESCRIPTION
	11/26/2019	November 2019 Laboratory Test Report

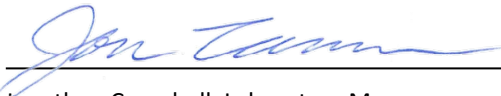
REMARKS:

CC:

SIGNED:


Barbara Sanchez, Assistant Laboratory Manager

APPROVED BY :


Jonathan Campbell, Laboratory Manager

November 26, 2019

Delaney Peterson
Anchor QEA, LLC
720 Olive Way, Suite 1900
Seattle, WA 98101

RE: Gasco PDI (GTX-310685)

Dear Delaney:

Enclosed are the test results you requested for the above referenced project. GeoTesting Express, Inc. (GTX) received eight samples from you on 11/1/2019. These samples were labeled as follows:

Sample Number
PDI-022SC-B-5.5-7.5-191016
PDI-031SC-B-8.9-10.9-191017
PDI-057SC-B-06-08-191023
PDI-059SC-B-06-08-191016
PDI-069SC-B-10-12-191016
PDI-083SC-B-08-10-191022
PDI-097SC-B-02-04-191017
PDI-099SC-B-02-04-191022

GTX performed the following tests on these samples:

8 ASTM D2216 - Moisture Content
8 ASTM D4318 - Atterberg Limits
2 ASTM D6913 - Sieve Analysis
6 ASTM D6913/D7928 - Grain Size Analysis - Sieve and Hydrometer
8 ASTM D854 - Specific Gravity

A copy of your test request is attached.

The results presented in this report apply only to the items tested. This report shall not be reproduced except in full, without written approval from GeoTesting Express. The remainder of these samples will be retained for a period of sixty (60) days and will then be discarded unless otherwise notified by you. Please call me if you have any questions or require additional information. Thank you for allowing GeoTesting Express the opportunity of providing you with testing services. We look forward to working with you again in the future.



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Respectfully yours,

A handwritten signature in black ink, appearing to read "Barbara Sanchez", written in a cursive style.

Barbara Sanchez
Assistant Laboratory Manager



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Geotechnical Test Report

11/26/2019

GTX-310685

Gasco PDI

Prepared for:

Anchor QEA, LLC



Client:	Anchor QEA, LLC	Project No:	GTX-310685
Project:	Gasco PDI		
Location:			
Boring ID: ---	Sample Type: ---	Tested By:	ckg
Sample ID: ---	Test Date: 11/19/19	Checked By:	bfs
Depth : ---	Test Id: 529668		

Moisture Content of Soil and Rock - ASTM D2216

Boring ID	Sample ID	Depth	Description	Moisture Content, %
---	PDI-2SC-B-5.5-7.5-1910	---	Moist, dark gray sand	10.7
---	PDI-SC-B-8.9-10.9-1910	---	Moist, dark gray sand	16.0
---	PDI-57SC-B-06-08-19102	---	Wet, dark gray clay	77.2
---	PDI-59SC-B-06-08-19101	---	Moist, dark grayish brown silty sand	38.4
---	PDI-69SC-B-10-12-19101	---	Moist, very dark gray silt	67.2
---	PDI-83SC-B-08-10-19102	---	Moist, dark gray clay	76.2
---	PDI-97SC-B-02-04-19101	---	Wet, dark gray silt	86.8
---	PDI-99SC-B-02-04-19102	---	Moist, very dark gray clay	79.6

Notes: Temperature of Drying : 110° Celsius



Client:	Anchor QEA, LLC	Project No:	GTX-310685
Project:	Gasco PDI		
Location:			
Boring ID: ---	Sample Type: ---	Tested By:	ckg
Sample ID: ---	Test Date: 11/19/19	Checked By:	bfs
Depth : ---	Test Id: 529676		

Specific Gravity of Soils by ASTM D854

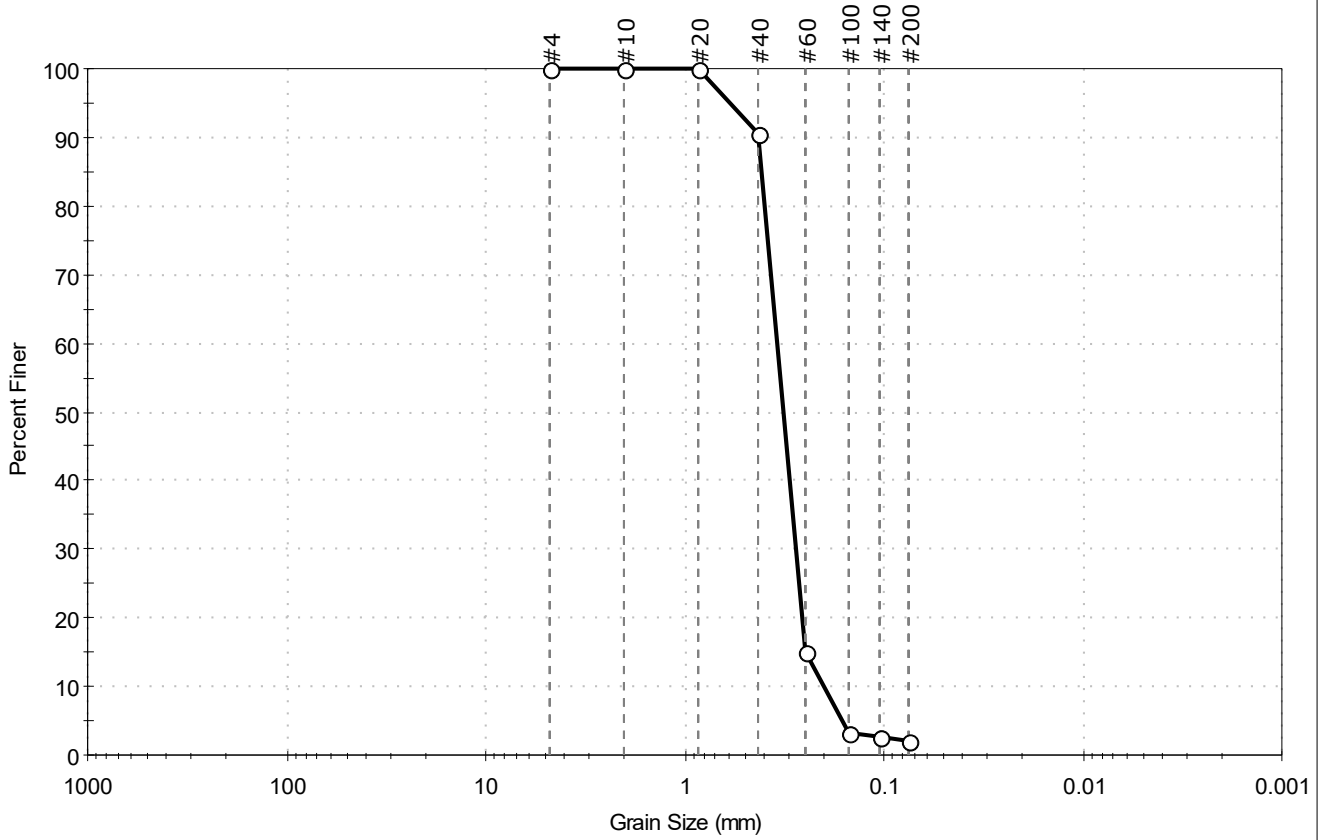
Boring ID	Sample ID	Depth	Visual Description	Specific Gravity	Comment
---	PDI- C-B-5.5-7.5-19	---	Moist, dark gray sand	2.75	
---	PDI- C-B-8.9-10.9-19	---	Moist, dark gray sand	2.75	
---	PDI- SC-B-06-08-191	---	Wet, dark gray clay	2.71	
---	PDI- SC-B-06-08-191	---	Moist, dark grayish brown silty sand	2.80	
---	PDI- SC-B-10-12-191	---	Moist, very dark gray silt	2.73	
---	PDI- SC-B-08-10-191	---	Moist, dark gray clay	2.65	
---	PDI- SC-B-02-04-191	---	Wet, dark gray silt	2.66	
---	PDI- SC-B-02-04-191	---	Moist, very dark gray clay	2.71	

Notes: Specific Gravity performed by using method B (oven dried specimens) of ASTM D854
 Moisture Content determined by ASTM D2216.



Client: Anchor QEA, LLC	Project No: GTX-310685
Project: Gasco PDI	
Location:	
Boring ID: ---	Sample Type: bag
Sample ID: PDI-022SC-B-5.5-7.5-191	Tested By: ckg
Depth: ---	Test Date: 11/19/19
	Checked By: bfs
Test Comment: ---	Test Id: 529663
Visual Description: Moist, dark gray sand	
Sample Comment: ---	

Particle Size Analysis - ASTM D6913/D7928



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	0.0	97.8	2.2

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
#4	4.75	100		
#10	2.00	100		
#20	0.85	100		
#40	0.42	90		
#60	0.25	15		
#100	0.15	3		
#140	0.11	3		
#200	0.075	2.2		

Coefficients	
D ₈₅ = 0.4090 mm	D ₃₀ = 0.2779 mm
D ₆₀ = 0.3431 mm	D ₁₅ = 0.2500 mm
D ₅₀ = 0.3198 mm	D ₁₀ = 0.2015 mm
C _u = 1.703	C _c = 1.117

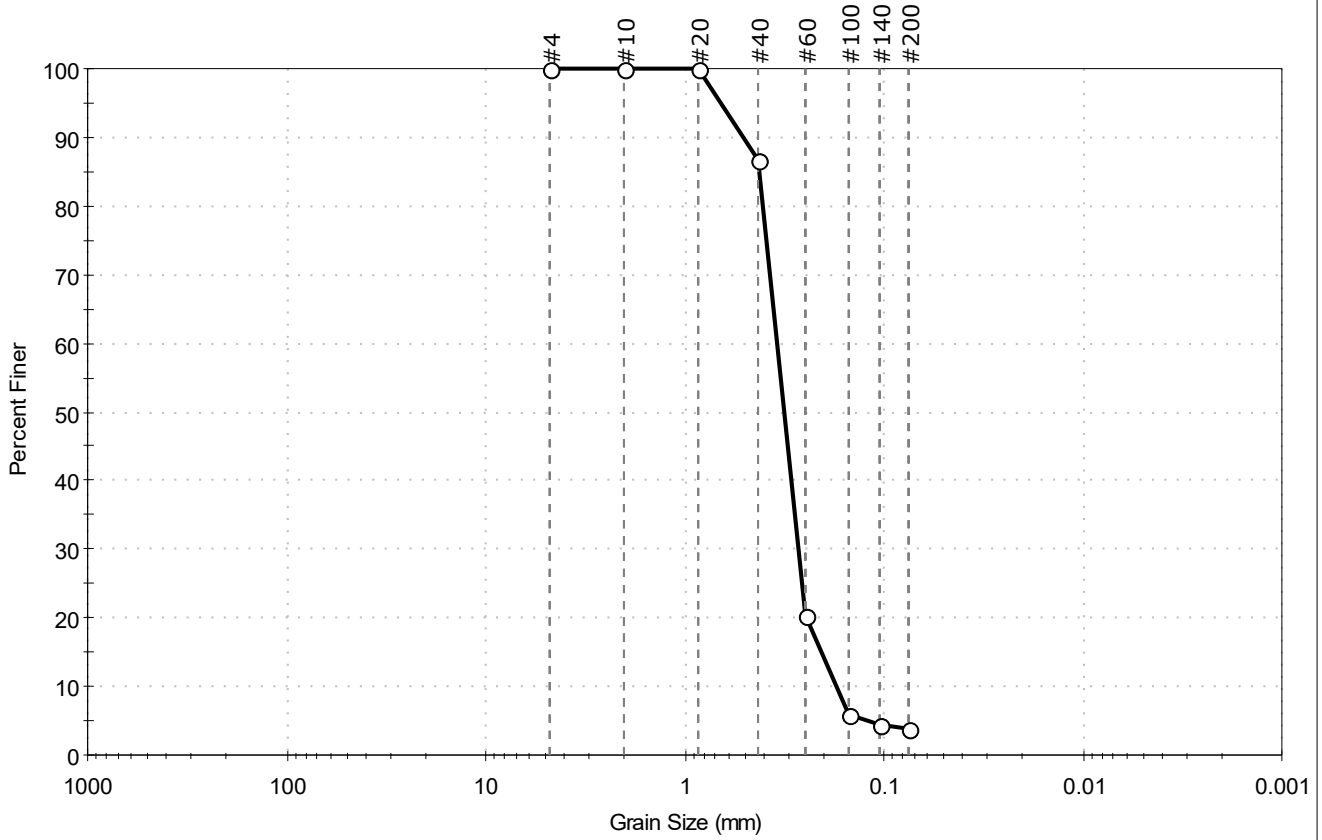
Classification	
ASTM	Poorly graded SAND (SP)
AASHTO	Fine Sand (A-3 (1))

Sample/Test Description
Sand/Gravel Particle Shape : ---
Sand/Gravel Hardness : ---



Client: Anchor QEA, LLC
 Project: Gasco PDI
 Location: Project No: GTX-310685
 Boring ID: --- Sample Type: bag Tested By: ckg
 Sample ID: PDI-031SC-B-8.9-10.9-19 Test Date: 11/19/19 Checked By: bfs
 Depth: --- Test Id: 529661
 Test Comment: ---
 Visual Description: Moist, dark gray sand
 Sample Comment: ---

Particle Size Analysis - ASTM D6913/D7928



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	0.0	96.1	3.9

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
#4	4.75	100		
#10	2.00	100		
#20	0.85	100		
#40	0.42	87		
#60	0.25	20		
#100	0.15	6		
#140	0.11	4		
#200	0.075	3.9		

Coefficients

D ₈₅ = 0.4188 mm	D ₃₀ = 0.2702 mm
D ₆₀ = 0.3432 mm	D ₁₅ = 0.2076 mm
D ₅₀ = 0.3169 mm	D ₁₀ = 0.1740 mm
C _u = 1.972	C _c = 1.223

Classification

ASTM Poorly graded SAND (SP)

AASHTO Fine Sand (A-3 (1))

Sample/Test Description

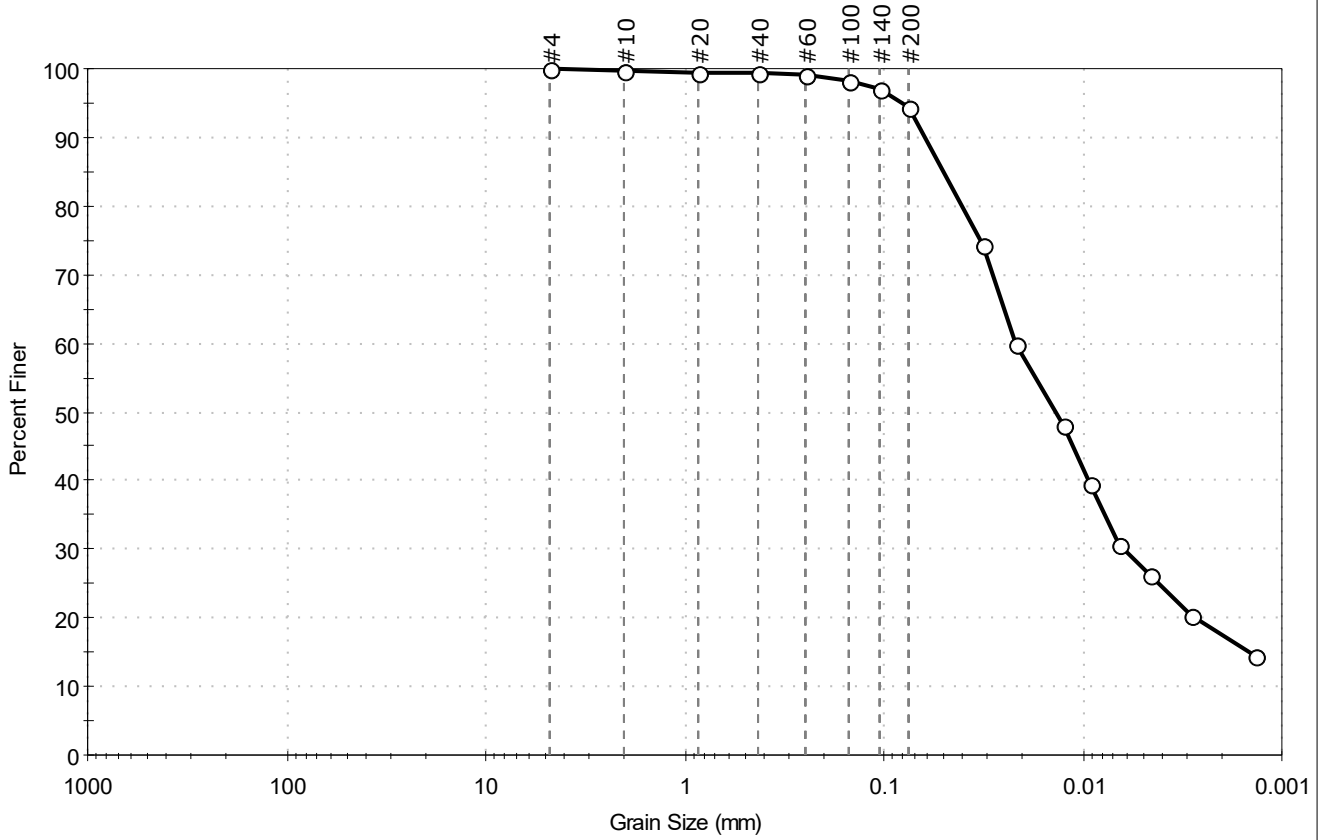
Sand/Gravel Particle Shape : ---

Sand/Gravel Hardness : ---



Client: Anchor QEA, LLC
 Project: Gasco PDI
 Location: Project No: GTX-310685
 Boring ID: --- Sample Type: bag Tested By: ckg
 Sample ID: PDI-057SC-B-06-08-1910 Test Date: 11/19/19 Checked By: bfs
 Depth: --- Test Id: 529658
 Test Comment: ---
 Visual Description: Wet, dark gray clay
 Sample Comment: Sample contains organics

Particle Size Analysis - ASTM D6913/D7928



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	0.0	5.5	94.5

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
#4	4.75	100		
#10	2.00	100		
#20	0.85	100		
#40	0.42	99		
#60	0.25	99		
#100	0.15	98		
#140	0.11	97		
#200	0.075	94		
Hydrometer	Particle Size (mm)	Percent Finer	Spec. Percent	Complies
---	0.0320	74		
---	0.0215	60		
---	0.0126	48		
---	0.0091	39		
---	0.0065	31		
---	0.0047	26		
---	0.0029	20		
---	0.0014	15		

Coefficients	
D ₈₅ = 0.0502 mm	D ₃₀ = 0.0062 mm
D ₆₀ = 0.0216 mm	D ₁₅ = 0.0015 mm
D ₅₀ = 0.0137 mm	D ₁₀ = N/A
C _u = N/A	C _c = N/A

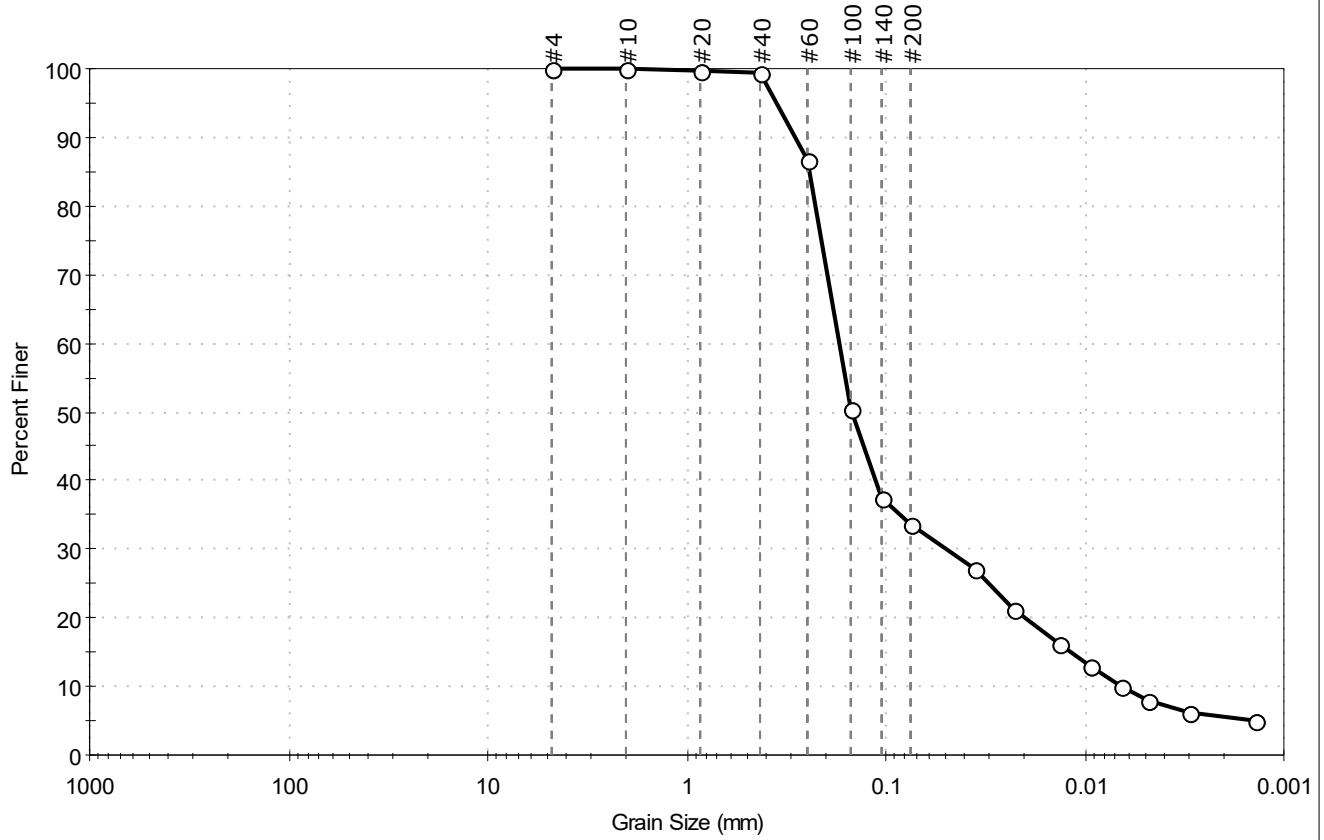
Classification	
ASTM	Fat CLAY (CH)
AASHTO	Clayey Soils (A-7-6 (49))

Sample/Test Description	
Sand/Gravel Particle Shape : ---	
Sand/Gravel Hardness : ---	
Dispersion Device : Apparatus A - Mech Mixer	
Dispersion Period : 1 minute	
Est. Specific Gravity : 2.65	
Separation of Sample: #200 Sieve	



Client: Anchor QEA, LLC	Project No: GTX-310685
Project: Gasco PDI	
Location:	
Boring ID: ---	Sample Type: bag
Sample ID: PDI-059SC-B-06-08-1910	Tested By: ckg
Test Date: 11/19/19	Checked By: bfs
Depth: ---	Test Id: 529664
Test Comment: ---	
Visual Description: Moist, dark grayish brown silty sand	
Sample Comment: ---	

Particle Size Analysis - ASTM D6913/D7928



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	0.0	66.4	33.6

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
#4	4.75	100		
#10	2.00	100		
#20	0.85	100		
#40	0.42	99		
#60	0.25	87		
#100	0.15	51		
#140	0.11	37		
#200	0.075	34		
Hydrometer	Particle Size (mm)	Percent Finer	Spec. Percent	Complies
---	0.0356	27		
---	0.0229	21		
---	0.0133	16		
---	0.0095	13		
---	0.0067	10		
---	0.0048	8		
---	0.0030	6		
---	0.0014	5		

Coefficients	
D ₈₅ = 0.2437 mm	D ₃₀ = 0.0492 mm
D ₆₀ = 0.1713 mm	D ₁₅ = 0.0117 mm
D ₅₀ = 0.1477 mm	D ₁₀ = 0.0066 mm
C _u = 25.955	C _c = 2.141

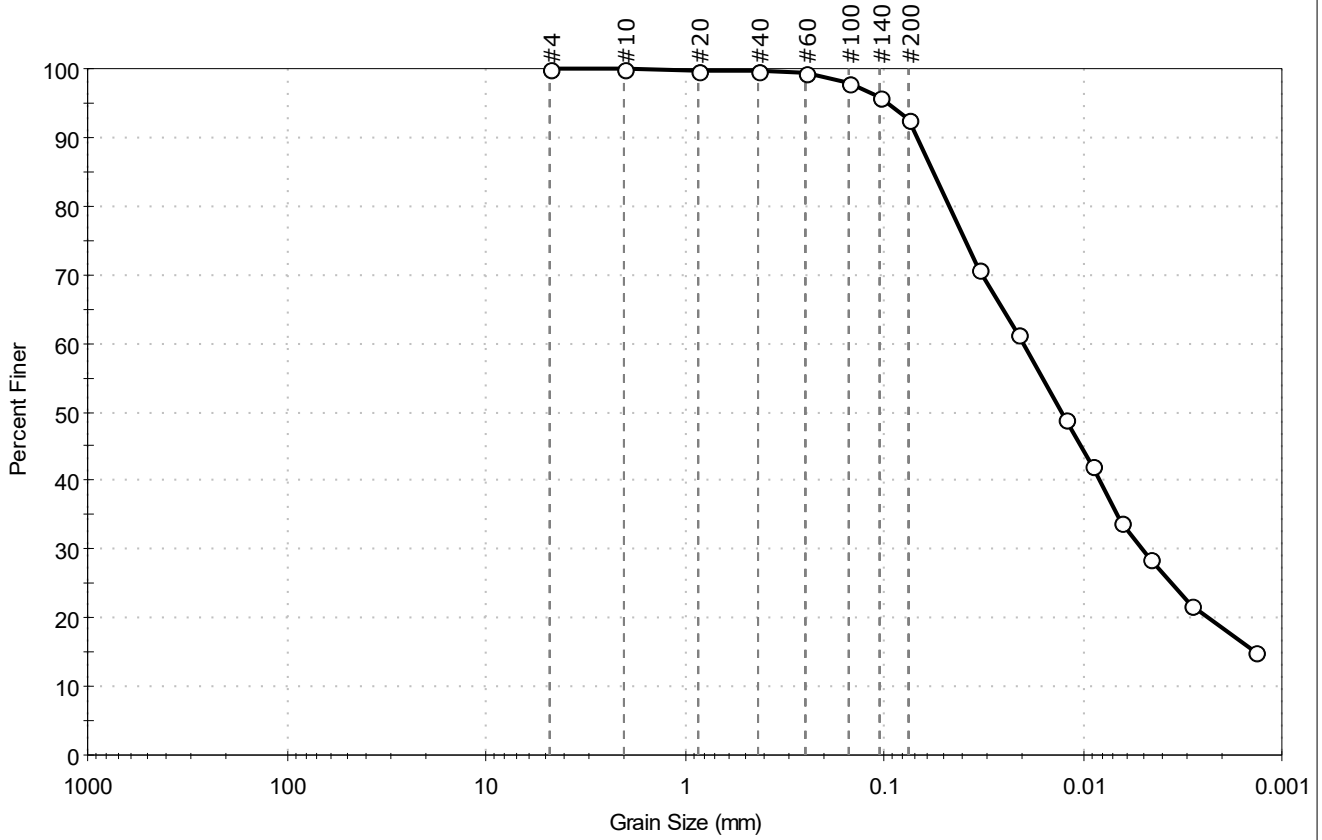
Classification	
ASTM	Silty SAND (SM)
AASHTO	Silty Gravel and Sand (A-2-4 (0))

Sample/Test Description
Sand/Gravel Particle Shape : ---
Sand/Gravel Hardness : ---
Dispersion Device : Apparatus A - Mech Mixer
Dispersion Period : 1 minute
Est. Specific Gravity : 2.65
Separation of Sample: #200 Sieve



Client: Anchor QEA, LLC
 Project: Gasco PDI
 Location: Project No: GTX-310685
 Boring ID: --- Sample Type: bag Tested By: ckg
 Sample ID: PDI-069SC-B-10-12-1910 Test Date: 11/19/19 Checked By: bfs
 Depth: --- Test Id: 529665
 Test Comment: ---
 Visual Description: Moist, very dark gray silt
 Sample Comment: ---

Particle Size Analysis - ASTM D6913/D7928



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	0.0	7.4	92.6

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
#4	4.75	100		
#10	2.00	100		
#20	0.85	100		
#40	0.42	100		
#60	0.25	99		
#100	0.15	98		
#140	0.11	96		
#200	0.075	93		
Hydrometer	Particle Size (mm)	Percent Finer	Spec. Percent	Complies
---	0.0330	71		
---	0.0210	61		
---	0.0124	49		
---	0.0090	42		
---	0.0065	34		
---	0.0046	29		
---	0.0029	22		
---	0.0014	15		

Coefficients	
D ₈₅ = 0.0563 mm	D ₃₀ = 0.0050 mm
D ₆₀ = 0.0199 mm	D ₁₅ = 0.0014 mm
D ₅₀ = 0.0129 mm	D ₁₀ = N/A
C _u = N/A	C _c = N/A

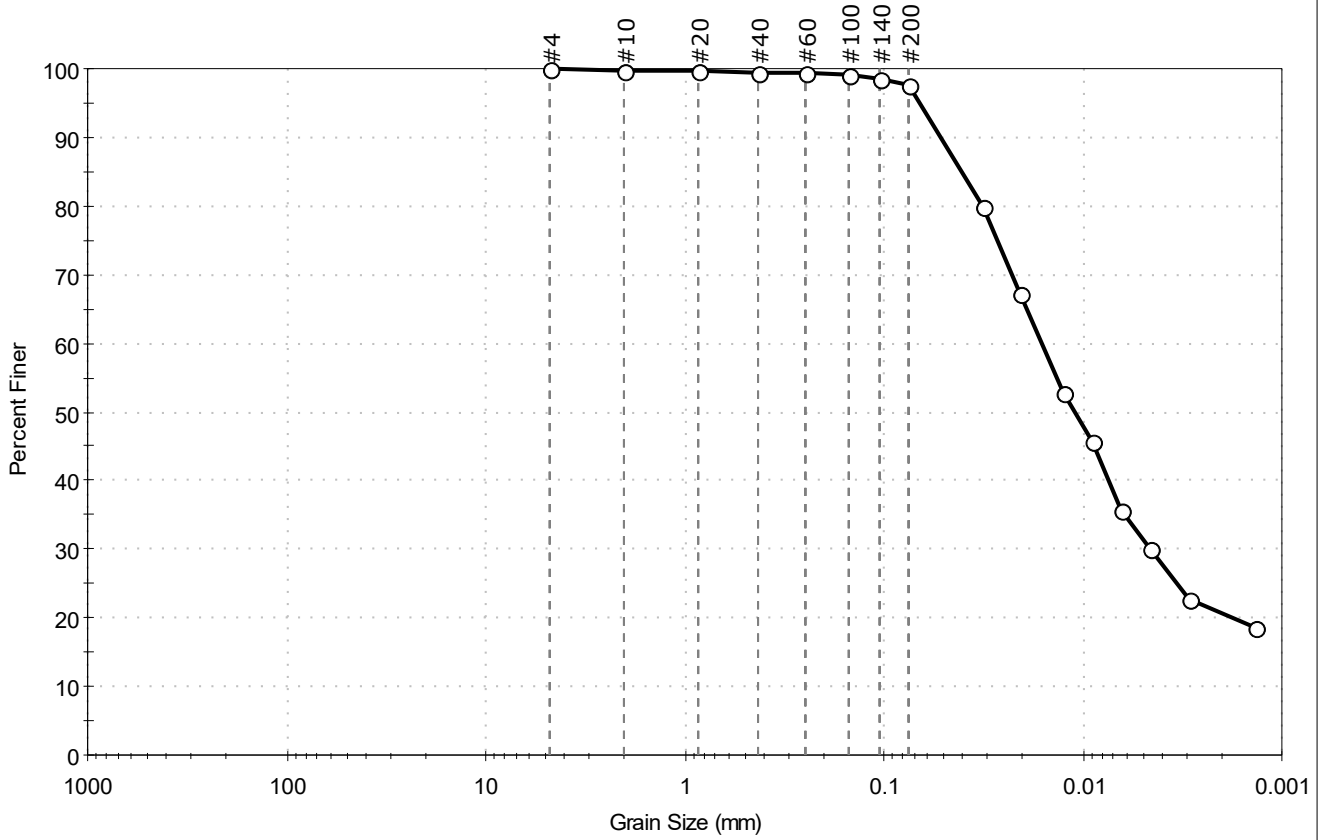
Classification	
ASTM	Elastic SILT (MH)
AASHTO	Clayey Soils (A-7-5 (40))

Sample/Test Description	
Sand/Gravel Particle Shape : ---	
Sand/Gravel Hardness : ---	
Dispersion Device : Apparatus A - Mech Mixer	
Dispersion Period : 1 minute	
Est. Specific Gravity : 2.65	
Separation of Sample: #200 Sieve	



Client: Anchor QEA, LLC
 Project: Gasco PDI
 Location: Project No: GTX-310685
 Boring ID: --- Sample Type: bag Tested By: ckg
 Sample ID: PDI-083SC-B-08-10-1910 Test Date: 11/19/19 Checked By: bfs
 Depth: --- Test Id: 529659
 Test Comment: ---
 Visual Description: Moist, dark gray clay
 Sample Comment: Sample contains organics

Particle Size Analysis - ASTM D6913/D7928



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	0.0	2.5	97.5

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
#4	4.75	100		
#10	2.00	100		
#20	0.85	100		
#40	0.42	99		
#60	0.25	99		
#100	0.15	99		
#140	0.11	99		
#200	0.075	98		
Hydrometer	Particle Size (mm)	Percent Finer	Spec. Percent	Complies
---	0.0320	80		
---	0.0208	67		
---	0.0125	53		
---	0.0089	46		
---	0.0065	36		
---	0.0046	30		
---	0.0029	23		
---	0.0014	19		

Coefficients	
D ₈₅ = 0.0408 mm	D ₃₀ = 0.0046 mm
D ₆₀ = 0.0161 mm	D ₁₅ = N/A
D ₅₀ = 0.0109 mm	D ₁₀ = N/A
C _u = N/A	C _c = N/A

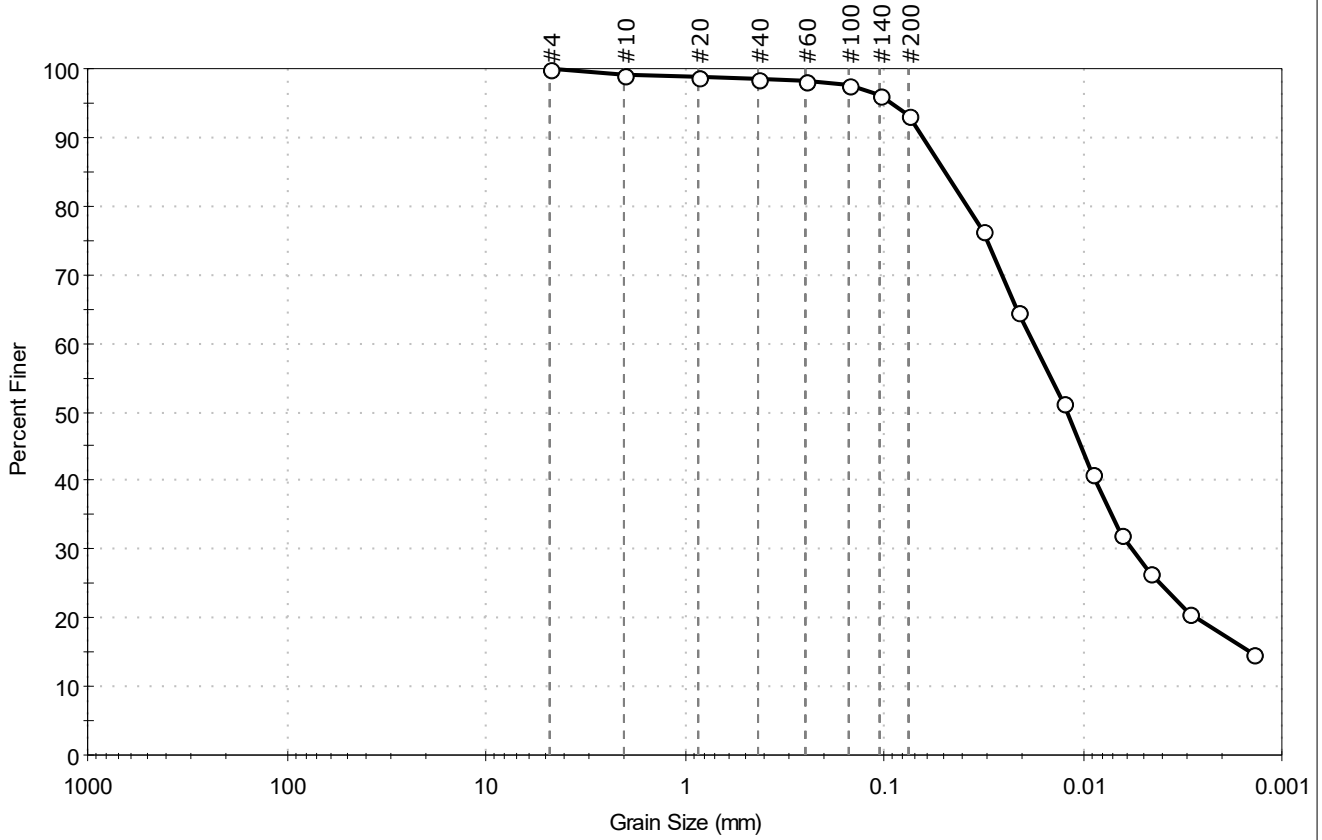
Classification	
ASTM	Fat CLAY (CH)
AASHTO	Clayey Soils (A-7-5 (55))

Sample/Test Description	
Sand/Gravel Particle Shape : ---	
Sand/Gravel Hardness : ---	
Dispersion Device : Apparatus A - Mech Mixer	
Dispersion Period : 1 minute	
Est. Specific Gravity : 2.65	
Separation of Sample: #200 Sieve	



Client: Anchor QEA, LLC	Project No: GTX-310685
Project: Gasco PDI	
Location:	
Boring ID: ---	Sample Type: bag
Sample ID: PDI-097SC-B-02-04-1910	Tested By: ckg
Test Date: 11/19/19	Checked By: bfs
Depth: ---	Test Id: 529662
Test Comment: ---	
Visual Description: Wet, dark gray silt	
Sample Comment: Sample contains organics	

Particle Size Analysis - ASTM D6913/D7928



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	0.0	6.7	93.3

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
#4	4.75	100		
#10	2.00	99		
#20	0.85	99		
#40	0.42	98		
#60	0.25	98		
#100	0.15	98		
#140	0.11	96		
#200	0.075	93		
Hydrometer	Particle Size (mm)	Percent Finer	Spec. Percent	Complies
---	0.0318	76		
---	0.0212	65		
---	0.0125	51		
---	0.0091	41		
---	0.0065	32		
---	0.0047	26		
---	0.0030	21		
---	0.0014	15		

Coefficients	
D ₈₅ = 0.0493 mm	D ₃₀ = 0.0057 mm
D ₆₀ = 0.0177 mm	D ₁₅ = 0.0014 mm
D ₅₀ = 0.0120 mm	D ₁₀ = N/A
C _u = N/A	C _c = N/A

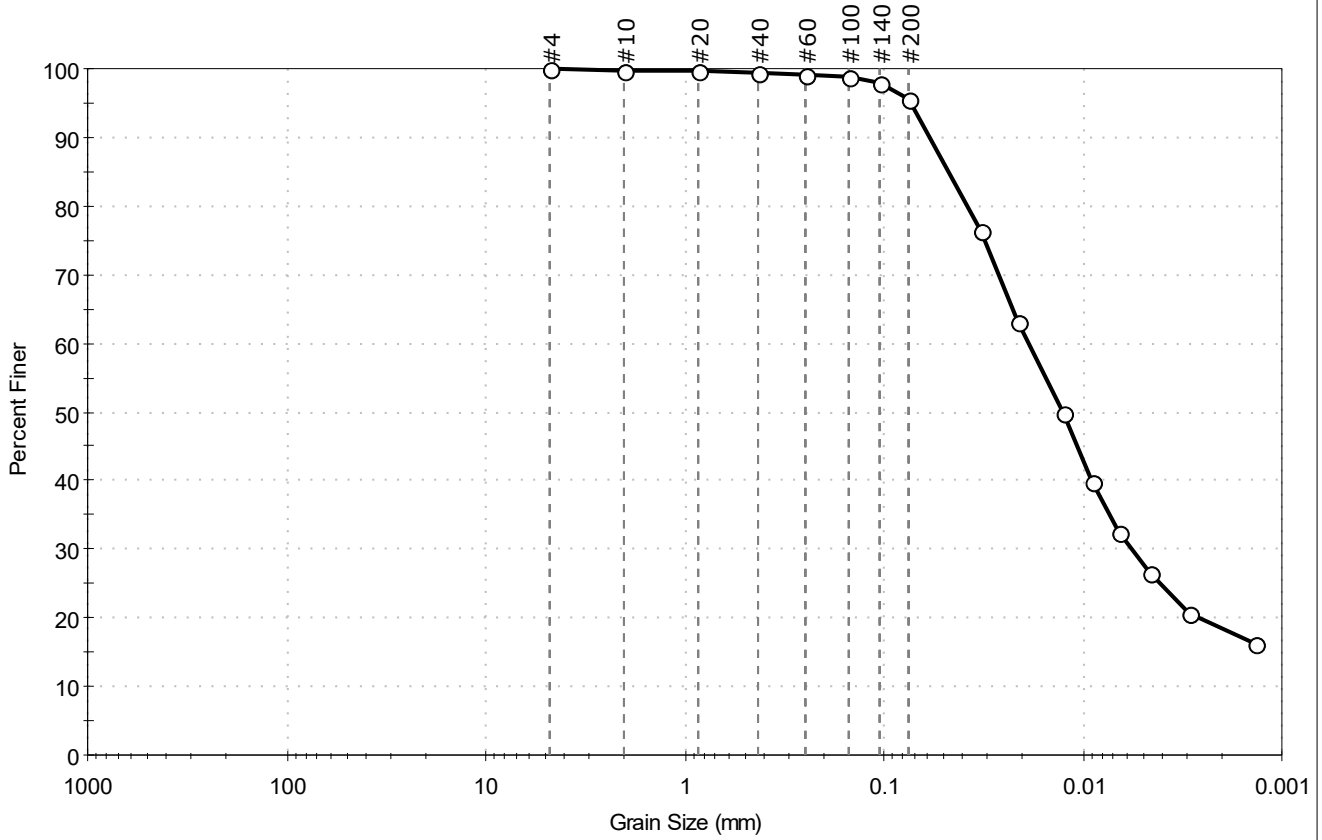
Classification	
ASTM	Elastic SILT (MH)
AASHTO	Clayey Soils (A-7-5 (39))

Sample/Test Description
Sand/Gravel Particle Shape : ---
Sand/Gravel Hardness : ---
Dispersion Device : Apparatus A - Mech Mixer
Dispersion Period : 1 minute
Est. Specific Gravity : 2.65
Separation of Sample: #200 Sieve



Client: Anchor QEA, LLC	Project: Gasco PDI	Location:	Project No: GTX-310685
Boring ID: ---	Sample Type: bag	Tested By: ckg	Checked By: bfs
Sample ID: PDI-099SC-B-02-04-1910	Test Date: 11/19/19	Depth: ---	Test Id: 529660
Test Comment: ---	Visual Description: Moist, very dark gray clay	Sample Comment: ---	

Particle Size Analysis - ASTM D6913/D7928



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	0.0	4.3	95.7

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
#4	4.75	100		
#10	2.00	100		
#20	0.85	100		
#40	0.42	99		
#60	0.25	99		
#100	0.15	99		
#140	0.11	98		
#200	0.075	96		
Hydrometer	Particle Size (mm)	Percent Finer	Spec. Percent	Complies
---	0.0326	76		
---	0.0214	63		
---	0.0126	50		
---	0.0091	40		
---	0.0065	32		
---	0.0047	26		
---	0.0029	21		
---	0.0014	16		

Coefficients	
D ₈₅ = 0.0472 mm	D ₃₀ = 0.0057 mm
D ₆₀ = 0.0188 mm	D ₁₅ = N/A
D ₅₀ = 0.0126 mm	D ₁₀ = N/A
C _u = N/A	C _c = N/A

Classification	
ASTM	Fat CLAY (CH)
AASHTO	Clayey Soils (A-7-5 (54))

Sample/Test Description	
Sand/Gravel Particle Shape : ---	
Sand/Gravel Hardness : ---	
Dispersion Device : Apparatus A - Mech Mixer	
Dispersion Period : 1 minute	
Est. Specific Gravity : 2.65	
Separation of Sample: #200 Sieve	



Client:	Anchor QEA, LLC		
Project:	Gasco PDI		
Location:		Project No:	GTX-310685
Boring ID:	---	Sample Type:	bag
Sample ID:	PDI-022SC-B-5.5-7.5-191	Tested By:	cam
Depth :	---	Test Date:	11/18/19
		Checked By:	bfs
		Test Id:	529655
Test Comment:	---		
Visual Description:	Moist, dark gray sand		
Sample Comment:	---		

Atterberg Limits - ASTM D4318

Sample Determined to be non-plastic

Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	22SC-B-5.5-7.5-191	---	---	11	n/a	n/a	n/a	n/a	Poorly graded SAND (SP)

10% Retained on #40 Sieve
 Dry Strength: NONE
 Dilatancy: RAPID
 Toughness: n/a
 The sample was determined to be Non-Plastic



Client:	Anchor QEA, LLC		
Project:	Gasco PDI		
Location:		Project No:	GTX-310685
Boring ID:	---	Sample Type:	bag
Sample ID:	PDI-031SC-B-8.9-10.9-19	Tested By:	cam
Depth :	---	Test Date:	11/18/19
		Checked By:	bfs
		Test Id:	529653
Test Comment:	---		
Visual Description:	Moist, dark gray sand		
Sample Comment:	---		

Atterberg Limits - ASTM D4318

Sample Determined to be non-plastic

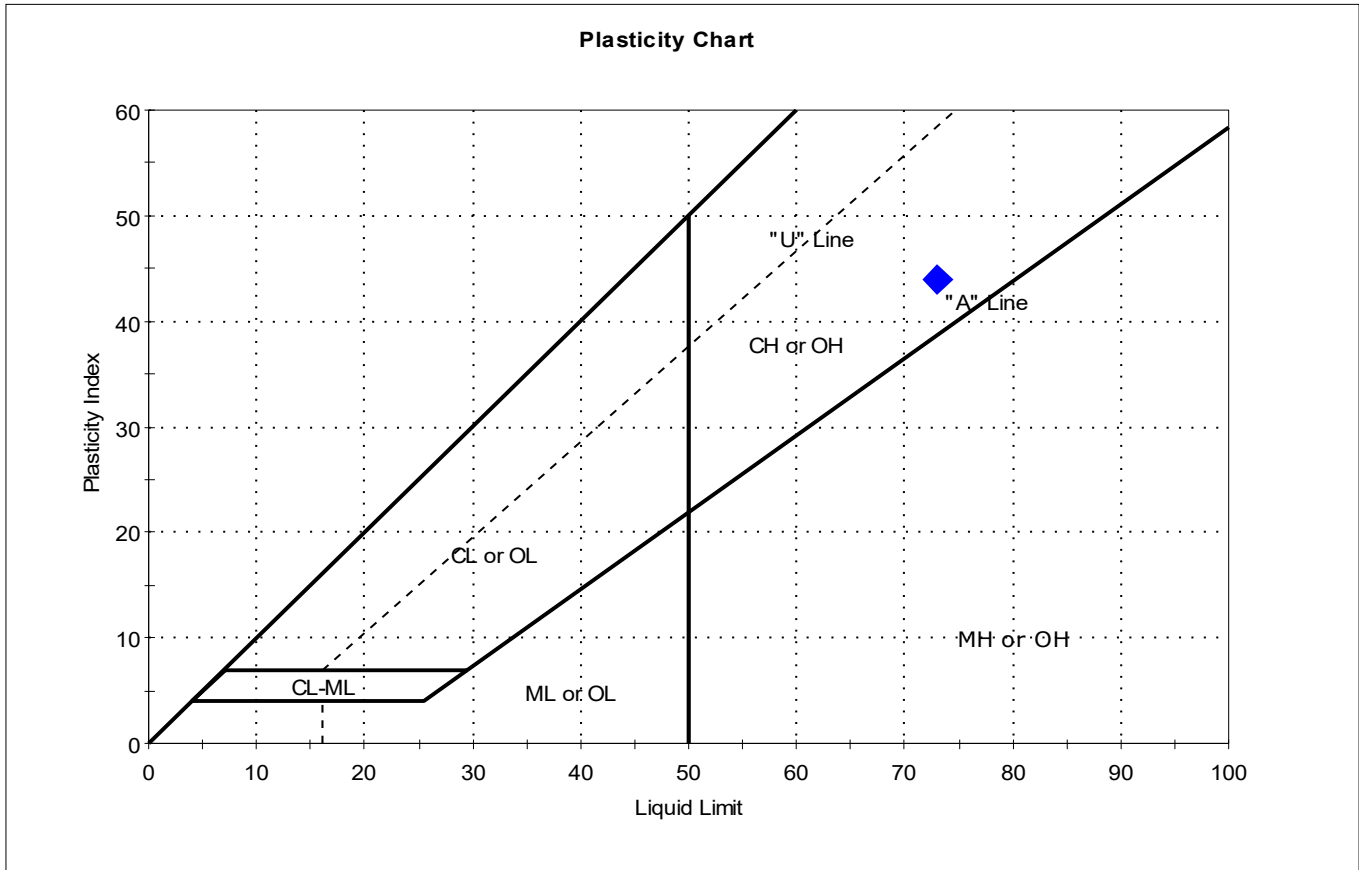
Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	31SC-B-8.9-10.9-1	---	---	16	n/a	n/a	n/a	n/a	Poorly graded SAND (SP)

13% Retained on #40 Sieve
 Dry Strength: NONE
 Dilatancy: RAPID
 Toughness: n/a
 The sample was determined to be Non-Plastic



Client: Anchor QEA, LLC	Project: Gasco PDI	Location:	Project No: GTX-310685
Boring ID: ---	Sample Type: bag	Tested By: cam	Checked By: bfs
Sample ID: PDI-057SC-B-06-08-1910	Test Date: 11/21/19	Depth: ---	Test Id: 529650
Test Comment: ---	Visual Description: Wet, dark gray clay	Sample Comment: Sample contains organics	

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	PDI-057SC-B-06-08-19	---	---	77	73	29	44	1.1	Fat CLAY (CH)

Sample Prepared using the WET method
 1% Retained on #40 Sieve
 Dry Strength: HIGH
 Dilatancy: SLOW
 Toughness: LOW



Client:	Anchor QEA, LLC		
Project:	Gasco PDI		
Location:		Project No:	GTX-310685
Boring ID:	---	Sample Type:	bag
Sample ID:	PDI-059SC-B-06-08-1910	Tested By:	cam
Depth :	---	Test Date:	11/19/19
		Checked By:	bfs
		Test Id:	529656
Test Comment:	---		
Visual Description:	Moist, dark grayish brown silty sand		
Sample Comment:	---		

Atterberg Limits - ASTM D4318

Sample Determined to be non-plastic

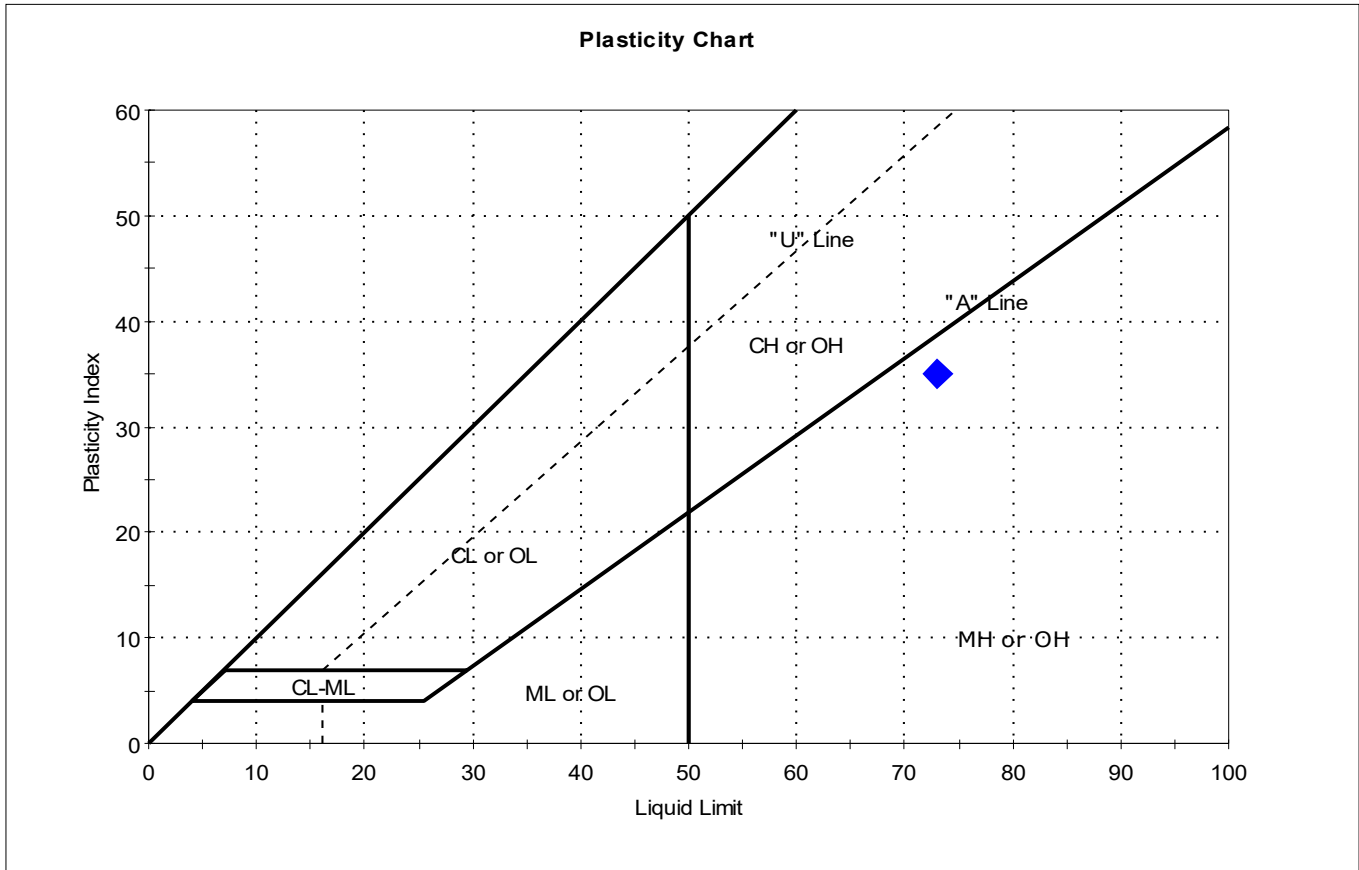
Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	059SC-B-06-08-19	---	---	38	n/a	n/a	n/a	n/a	Silty SAND (SM)

1% Retained on #40 Sieve
 Dry Strength: MEDIUM
 Dilatancy: RAPID
 Toughness: n/a
 The sample was determined to be Non-Plastic



Client: Anchor QEA, LLC	Project: Gasco PDI	Location:	Project No: GTX-310685
Boring ID: ---	Sample Type: bag	Tested By: cam	Checked By: bfs
Sample ID: PDI-069SC-B-10-12-19	Test Date: 11/20/19	Depth: ---	Test Id: 529657
Test Comment: ---	Visual Description: Moist, very dark gray silt	Sample Comment: ---	

Atterberg Limits - ASTM D4318



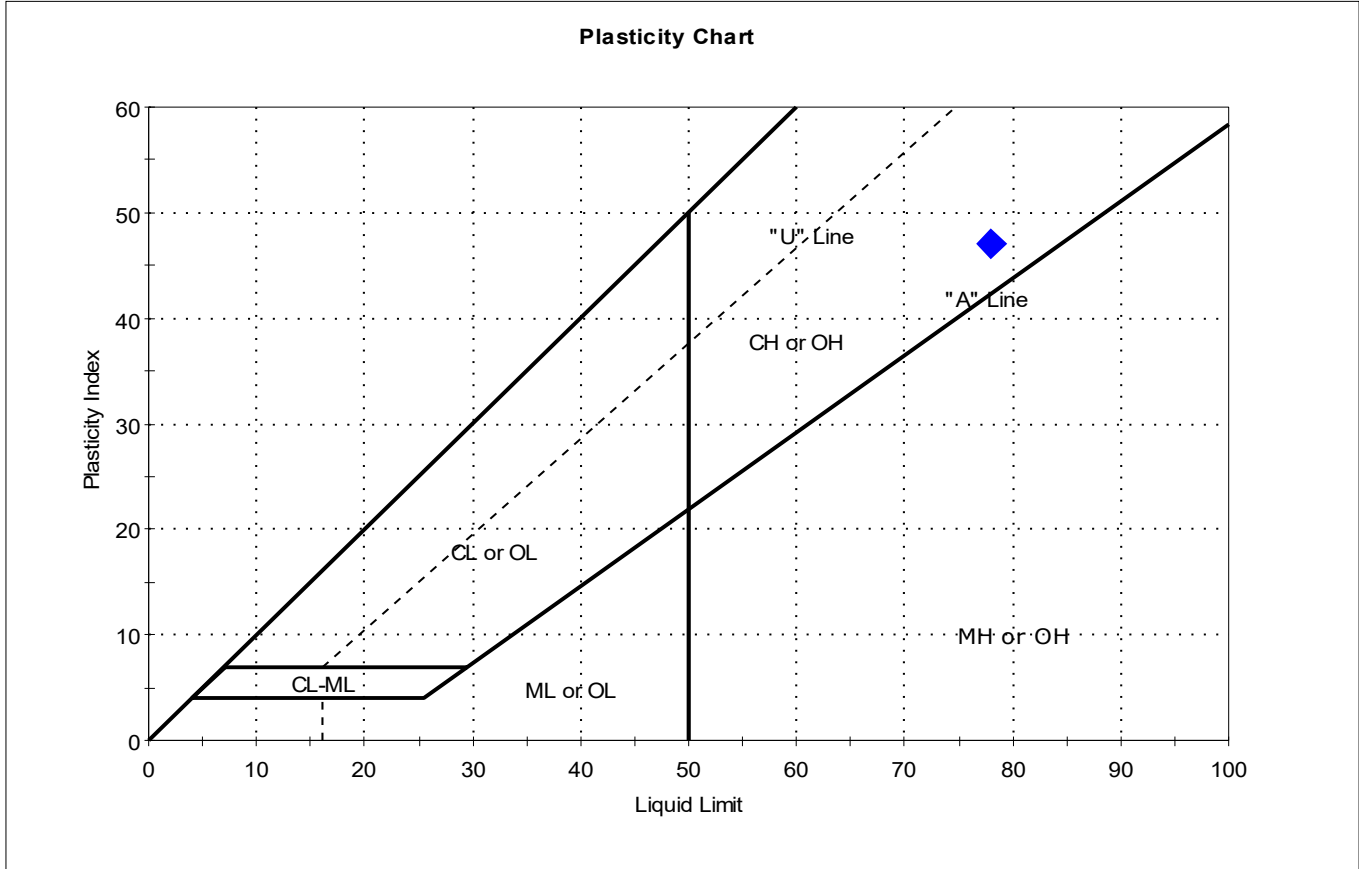
Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	PDI-069SC-B-10-12-19	---	---	67	73	38	35	0.8	Elastic SILT (MH)

Sample Prepared using the WET method
 0% Retained on #40 Sieve
 Dry Strength: VERY HIGH
 Dilatancy: SLOW
 Toughness: LOW



Client:	Anchor QEA, LLC		
Project:	Gasco PDI		
Location:		Project No:	GTX-310685
Boring ID:	---	Sample Type:	bag
Sample ID:	PDI-083SC-B-08-10-1910	Tested By:	cam
Depth:	---	Test Date:	11/20/19
		Checked By:	bfs
		Test Id:	529651
Test Comment:	---		
Visual Description:	Moist, dark gray clay		
Sample Comment:	Sample contains organics		

Atterberg Limits - ASTM D4318



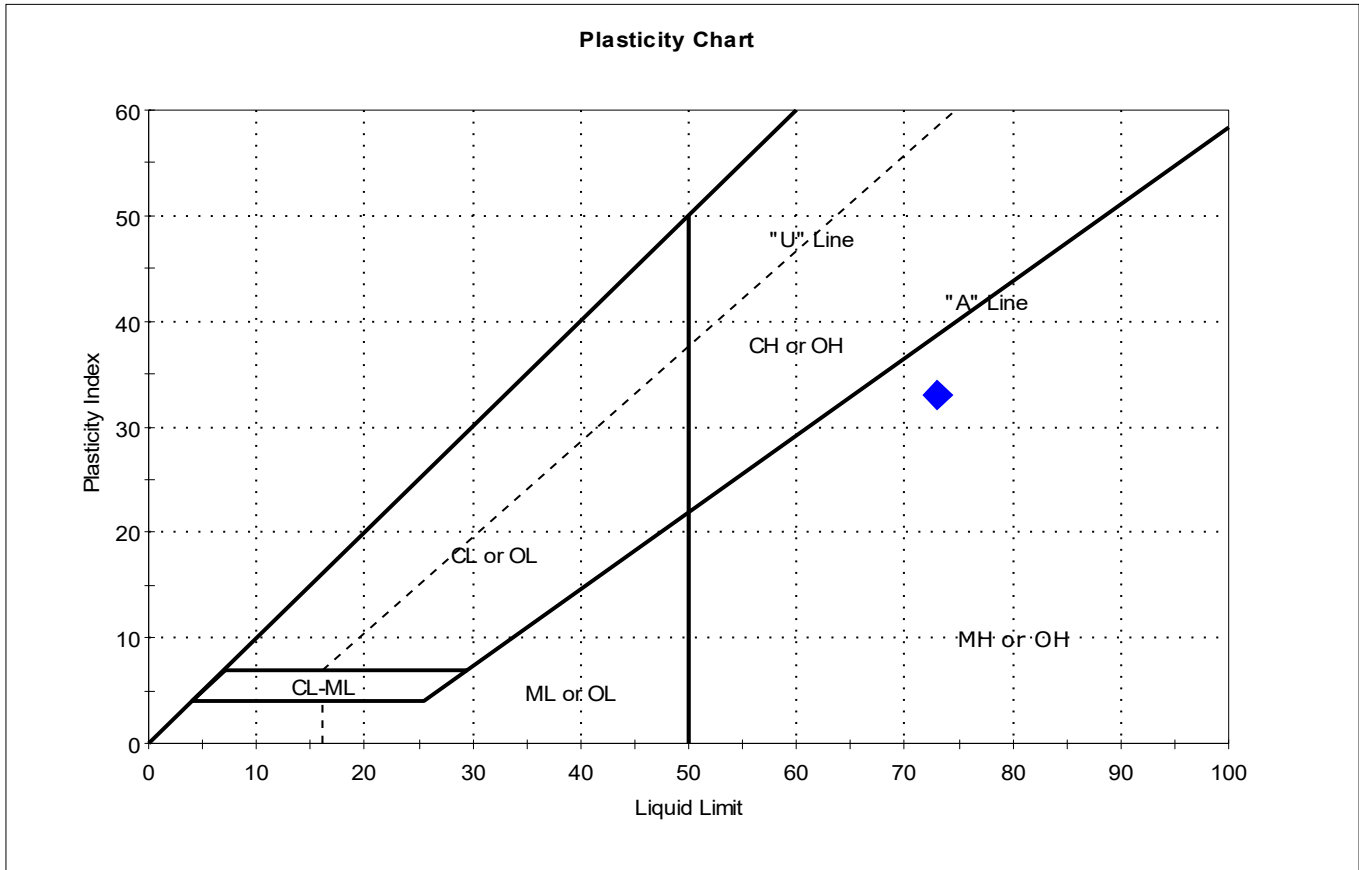
Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	083SC-B-08-10-19	---	---	76	78	31	47	1	Fat CLAY (CH)

Sample Prepared using the WET method
 1% Retained on #40 Sieve
 Dry Strength: VERY HIGH
 Dilatancy: SLOW
 Toughness: LOW



Client: Anchor QEA, LLC	Project: Gasco PDI	Location:	Project No: GTX-310685
Boring ID: ---	Sample Type: bag	Tested By: cam	Checked By: bfs
Sample ID: PDI-097SC-B-02-04-1910	Test Date: 11/19/19	Test Id: 529654	
Depth: ---			
Test Comment: ---			
Visual Description: Wet, dark gray silt			
Sample Comment: Sample contains organics			

Atterberg Limits - ASTM D4318



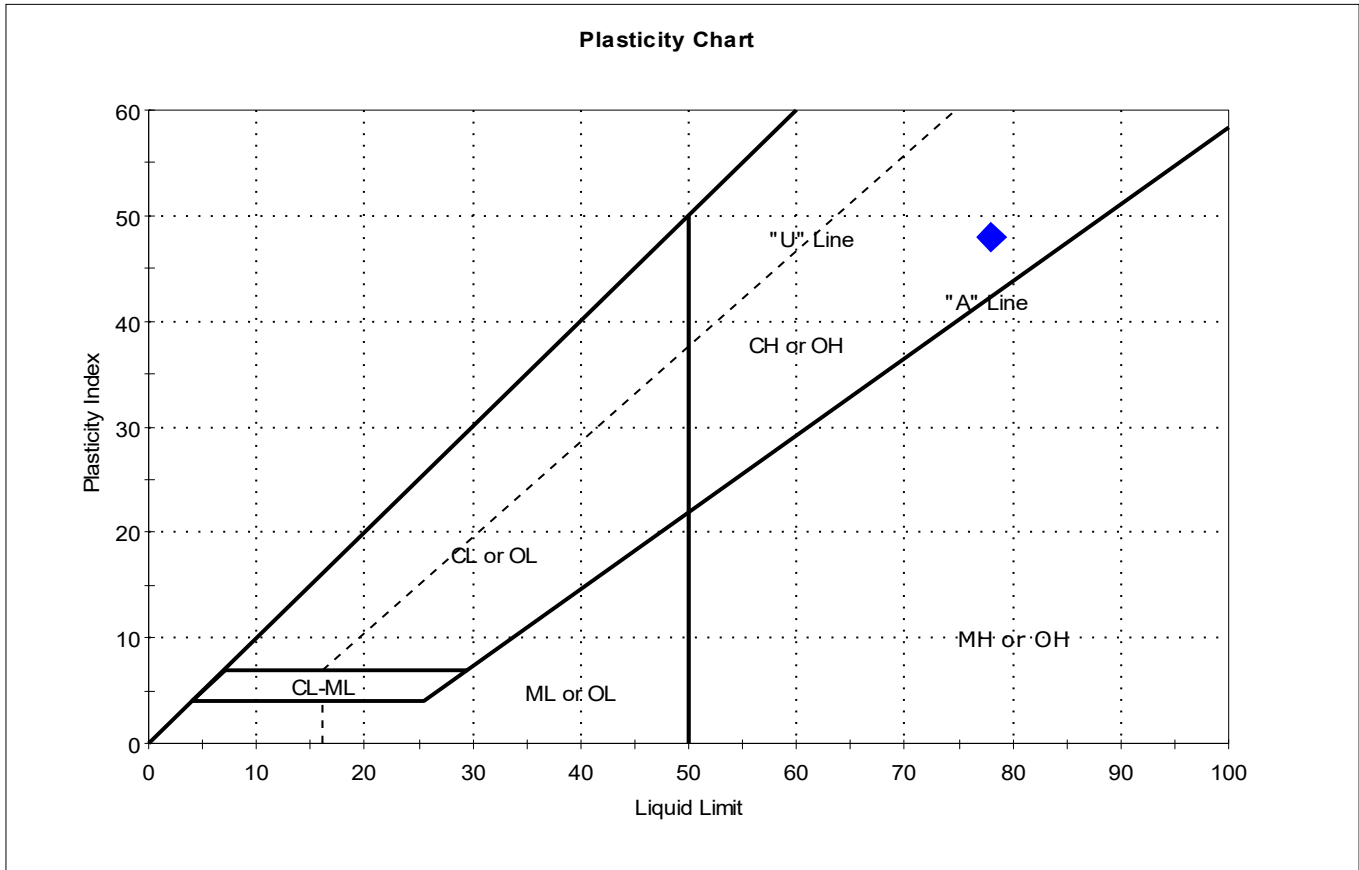
Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	PDI-097SC-B-02-04-19	---	---	87	73	40	33	1.4	Elastic SILT (MH)

Sample Prepared using the WET method
 2% Retained on #40 Sieve
 Dry Strength: HIGH
 Dilatancy: SLOW
 Toughness: LOW



Client: Anchor QEA, LLC	Project: Gasco PDI	Location:	Project No: GTX-310685
Boring ID: ---	Sample Type: bag	Tested By: cam	Checked By: bfs
Sample ID: PDI-099SC-B-02-04-1910	Test Date: 11/20/19	Test Id: 529652	
Depth: ---			
Test Comment: ---			
Visual Description: Moist, very dark gray clay			
Sample Comment: ---			

Atterberg Limits - ASTM D4318



Symbol	Sample ID	Boring	Depth	Natural Moisture Content, %	Liquid Limit	Plastic Limit	Plasticity Index	Liquidity Index	Soil Classification
◆	PDI-099SC-B-02-04-19	---	---	80	78	30	48	1	Fat CLAY (CH)

Sample Prepared using the WET method
 1% Retained on #40 Sieve
 Dry Strength: VERY HIGH
 Dilatancy: SLOW
 Toughness: LOW



1201 3rd Avenue, Suite 2600, Seattle, WA 98101

ENVIRONMENTAL SAMPLE CHAIN OF CUSTODY

COC ID: NWGEO-201910.
Sample Custodian: CO, SN, BJ, SS
Lab: Geotesting Express

POC: # Delaney Peterson (360-715-2707) Project: Gasco PDI Client: NW Natural
1605 Cornwall Avenue, Bellingham, WA 98225

COC Sample Number	Field Sample ID	Sample Type	Matrix	Collected Date	Time	Containers #	Lab QC* <input type="checkbox"/>	Test Request	Method	TAT**	Preservative
001	PDI-057SC-B-06-08-191023	N	SE	10/23/2019	12:46	1	<input type="checkbox"/>	Atterberg Limits	D4318	30	4°C
								Grain Size	D6913/D7928	30	4°C
								Moisture Content	D2216	30	4°C
								Specific gravity	D854	30	4°C

Comment:

Received By:	Relinquished By:	Received By:	Relinquished By:
Signature:	Signature:	Signature:	Signature:
Print Name: Scott Ferguson	Print Name: Scott Ferguson	Print Name: Scott Ferguson	Print Name: Scott Ferguson
Company: Anchor OEA	Company: Anchor OEA	Company: Anchor OEA	Company: Anchor OEA
Date/Time: 10/29/19 08:15	Date/Time: 11/16/19 18:00	Date/Time: 11/16/19 18:00	Date/Time: 11/16/19 18:00

* Lab QC Requested for sample when box is checked ** TAT = Turn Around Time in DAYS # POC = Project Point of Contact



1201 3rd Avenue, Suite 2600, Seattle, WA 98101

ENVIRONMENTAL SAMPLE CHAIN OF CUSTODY

COC ID: NWGEO-20191022-162549
Sample Custodian: CO, SN, BJ, SS
Lab: Geotesting Express

POC: # Delaney Peterson (360-715-2707) Project: Gasco PDI
1605 Cornwall Avenue, Bellingham, WA 98225 Client: NW Natural

COC Sample Number	Field Sample ID	Sample Type	Matrix	Collected Date	Collected Time	Containers #	Lab QC*	Test Request	Method	TAT**	Preservative
001	PDI-083SC-B-08-10-191022	N	SE	10/22/2019	14:05	1	<input type="checkbox"/>	Atterberg Limits Grain Size Moisture Content Specific gravity	D4318 D6913/D7928 D2216 D854	30 30 30 30	4°C 4°C 4°C 4°C
002	PDI-099SC-B-02-04-191022	N	SE	10/22/2019	10:48	1	<input type="checkbox"/>	Atterberg Limits Grain Size Moisture Content Specific gravity	D4318 D6913/D7928 D2216 D854	30 30 30 30	4°C 4°C 4°C 4°C

Comment:

Relinquished By:		Received By:		Relinquished By:		Received By:	
Signature	Print Name	Signature	Print Name	Signature	Print Name	Signature	Print Name
	Ben Johnson		Scott Ferguson				
	Anchorage		M. King				
10/29/19 09:15		11/16/19 12:00					

* Lab QC Requested for sample when box is checked ** TAT = Turn Around Time in DAYS # POC = Project Point of Contact



1201 3rd Avenue, Suite 2600, Seattle, WA 98101

ENVIRONMENTAL SAMPLE CHAIN OF CUSTODY

COC ID: NWGEO-20191017-123936

Sample Custodian: SN

Lab: Geotesting Express

POC: # Delaney Peterson (360-715-2707) Project: Gasco PDI

1605 Cornwall Avenue, Bellingham, WA 98225 Client: NW Natural

COC Sample Number	Field Sample ID	Sample Type	Matrix	Collected Date	Time	Containers #	Lab QC* <input type="checkbox"/>	Test Request	Method	TAT**	Preservative
001	PDI-031SC-B-8.9-10.9-191017	N	SE	10/17/2019	9:06	1	<input type="checkbox"/>	Atterberg Limits Grain Size Moisture Content Specific gravity	D4318 D6913/D7928 D2216 D854	30 30 30 30	4°C 4°C 4°C 4°C
002	PDI-097SC-B-02-04-191017	N	SE	10/17/2019	10:46	1	<input type="checkbox"/>	Atterberg Limits Grain Size Moisture Content Specific gravity	D4318 D6913/D7928 D2216 D854	30 30 30 30	4°C 4°C 4°C 4°C

Comment:

Relinquished By:	Received By:	Relinquished By:	Received By:
Signature: <i>[Signature]</i>	Signature: <i>[Signature]</i>	Signature: <i>[Signature]</i>	Signature: <i>[Signature]</i>
Print Name: Ben Johnson	Print Name: Scott Ferguson	Print Name: <i>[Print Name]</i>	Print Name: <i>[Print Name]</i>
Company: Archer OEA	Company: GTH	Company: <i>[Company]</i>	Company: <i>[Company]</i>
Date/Time: 10/29/19 09:15	Date/Time: 11/16/19 18:00	Date/Time: <i>[Date/Time]</i>	Date/Time: <i>[Date/Time]</i>

* Lab QC Requested for sample when box is checked ** TAT = Turn Around Time in DAYS # POC = Project Point of Contact



1201 3rd Avenue, Suite 2600, Seattle, WA 98101

ENVIRONMENTAL SAMPLE CHAIN OF CUSTODY

COC ID: NWGEO-20191016-143858
Sample Custodian: CO, SN, BJ, DL
Lab: Geotesting Express

POC: # Delaney Peterson (360-715-2707) **Project:** Gasco PDI
 1605 Cornwall Avenue, Bellingham, WA 98225 **Client:** NW Natural

COC Sample Number	Field Sample ID	Sample Type	Matrix	Collected Date	Collected Time	# Containers	Lab QC*	Test Request	Method	TAT**	Preservative
001	PDI-022SC-B-5-7.5-191016	N	SE	10/16/2019	13:41	1	<input type="checkbox"/>	Atterberg Limits Grain Size Moisture Content Specific gravity	D4318 D6913/D7928 D2216 D854	30	4°C
002	PDI-059SC-B-06-08-191016	N	SE	10/16/2019	7:57	1	<input type="checkbox"/>	Atterberg Limits Grain Size Moisture Content Specific gravity	D4318 D6913/D7928 D2216 D854	30	4°C
003	PDI-069SC-B-10-12-191016	N	SE	10/16/2019	10:38	1	<input type="checkbox"/>	Atterberg Limits Grain Size Moisture Content Specific gravity	D4318 D6913/D7928 D2216 D854	30	4°C

Comment:

Received By:	Relinquished By:	Received By:	Relinquished By:
Signature: <i>[Signature]</i>	Signature: <i>[Signature]</i>	Signature: <i>[Signature]</i>	Signature: <i>[Signature]</i>
Print Name: Ben Johnson	Print Name: Scott Ferguson	Print Name: <i>[Signature]</i>	Print Name: <i>[Signature]</i>
Company: Anchor OEA	Company: GTX	Company: <i>[Signature]</i>	Company: <i>[Signature]</i>
Date/Time: 10/29/19 12:15	Date/Time: 10/16/19 12:00	Date/Time: <i>[Signature]</i>	Date/Time: <i>[Signature]</i>

* Lab QC Requested for sample when box is checked ** TAT = Turn Around Time in DAYS # POC = Project Point of Contact

WARRANTY and LIABILITY

GeoTesting Express (GTX) warrants that all tests it performs are run in general accordance with the specified test procedures and accepted industry practice. GTX will correct or repeat any test that does not comply with this warranty. GTX has no specific knowledge as to conditioning, origin, sampling procedure or intended use of the material.

GTX may report engineering parameters that require us to interpret the test data. Such parameters are determined using accepted engineering procedures. However, GTX does not warrant that these parameters accurately reflect the true engineering properties of the *in situ* material. Responsibility for interpretation and use of the test data and these parameters for engineering and/or construction purposes rests solely with the user and not with GTX or any of its employees.

GTX's liability will be limited to correcting or repeating a test which fails our warranty. GTX's liability for damages to the Purchaser of testing services for any cause whatsoever shall be limited to the amount GTX received for the testing services. GTX will not be liable for any damages, or for any lost benefits or other consequential damages resulting from the use of these test results, even if GTX has been advised of the possibility of such damages. GTX will not be responsible for any liability of the Purchaser to any third party.

Commonly Used Symbols

A	pore pressure parameter for $\Delta\sigma_1 - \Delta\sigma_3$	S_r	Post cyclic undrained shear strength
B	pore pressure parameter for $\Delta\sigma_3$	T	temperature
CAI	CERCHAR Abrasiveness Index	t	time
CIU	isotropically consolidated undrained triaxial shear test	U, UC	unconfined compression test
CR	compression ratio for one dimensional consolidation	UU, Q	unconsolidated undrained triaxial test
CSR	cyclic stress ratio	u_a	pore gas pressure
C_c	coefficient of curvature, $(D_{30})^2 / (D_{10} \times D_{60})$	u_e	excess pore water pressure
C_u	coefficient of uniformity, D_{60}/D_{10}	u, u_w	pore water pressure
C_c	compression index for one dimensional consolidation	V	total volume
C_a	coefficient of secondary compression	V_g	volume of gas
c_v	coefficient of consolidation	V_s	volume of solids
c	cohesion intercept for total stresses	V_s	shear wave velocity
c'	cohesion intercept for effective stresses	V_v	volume of voids
D	diameter of specimen	V_w	volume of water
D	damping ratio	V_o	initial volume
D_{10}	diameter at which 10% of soil is finer	v	velocity
D_{15}	diameter at which 15% of soil is finer	W	total weight
D_{30}	diameter at which 30% of soil is finer	W_s	weight of solids
D_{50}	diameter at which 50% of soil is finer	W_w	weight of water
D_{60}	diameter at which 60% of soil is finer	w	water content
D_{85}	diameter at which 85% of soil is finer	w_c	water content at consolidation
d_{50}	displacement for 50% consolidation	w_f	final water content
d_{90}	displacement for 90% consolidation	w_l	liquid limit
d_{100}	displacement for 100% consolidation	w_n	natural water content
E	Young's modulus	w_p	plastic limit
e	void ratio	w_s	shrinkage limit
e_c	void ratio after consolidation	w_o, w_i	initial water content
e_o	initial void ratio	α	slope of q_f versus p_f
G	shear modulus	α'	slope of q_f versus p_f'
G_s	specific gravity of soil particles	γ_t	total unit weight
H	height of specimen	γ_d	dry unit weight
H_R	Rebound Hardness number	γ_s	unit weight of solids
i	gradient	γ_w	unit weight of water
I_S	Uncorrected point load strength	ϵ	strain
$I_{S(50)}$	Size corrected point load strength index	ϵ_{vol}	volume strain
H_A	Modified Taber Abrasion	ϵ_h, ϵ_v	horizontal strain, vertical strain
H_T	Total hardness	μ	Poisson's ratio, also viscosity
K_o	lateral stress ratio for one dimensional strain	σ	normal stress
k	permeability	σ'	effective normal stress
LI	Liquidity Index	σ_c, σ'_c	consolidation stress in isotropic stress system
m_v	coefficient of volume change	σ_h, σ'_h	horizontal normal stress
n	porosity	σ_v, σ'_v	vertical normal stress
PI	plasticity index	σ'_{vc}	Effective vertical consolidation stress
P_c	preconsolidation pressure	σ_1	major principal stress
p	$(\sigma_1 + \sigma_3) / 2, (\sigma_v + \sigma_h) / 2$	σ_2	intermediate principal stress
p'	$(\sigma'_1 + \sigma'_3) / 2, (\sigma'_v + \sigma'_h) / 2$	σ_3	minor principal stress
p'_c	p' at consolidation	τ	shear stress
Q	quantity of flow	ϕ	friction angle based on total stresses
q	$(\sigma_1 - \sigma_3) / 2$	ϕ'	friction angle based on effective stresses
q_f	q at failure	ϕ'_r	residual friction angle
q_o, q_i	initial q	ϕ_{ult}	ϕ for ultimate strength
q_c	q at consolidation		