

☐ Construction

WASHINGTON UTILITIES AND TRANSPORTATION COMMISSION

	DOCKET NO. TR-			
Skagit County	PERMINAL HA GOLVETHIJET OF			
Petitioner,	PETITION TO CONSTRUCT OR RECONSTRUCT A RAILROAD-			
vs.	HIGHWAY GRADE SEPARATION (OVERCROSSING OR			
Burlington Northern Santa Fe Railway	UNDERCROSSING)			
Respondent	USDOT CROSSING NO.: 967619S			
The Petitioner asks the Washington Utilities and Transportation Commission (UTC) to approve				

of a railroad-highway grade separation (overcrossing or undercrossing¹) as described in this petition. *RCW 81.53.060*.

⊠ Reconstruction

Section 1 – Petitioner's Information

Skagit County
Petitioner
Pala. Rolls, county ENGINEER Signature
1800 Continental Place
Street Address
Mount Vernon, WA 98273
City, State and Zip Code
Same
Mailing Address, if different than the street address
Forrest Jones
Contact Person Name
360-416-1422 <u>forrestj@co.skagit.wa.us</u>
Contact Phone Number and Email Address

¹ An overcrossing means any point or place where a highway crosses a railroad by passing above the same, or any point or place where one railroad crosses another railroad not at grade. An undercrossing means any point or place where a highway crosses a railroad by passing under the same, or any point or place where one railroad crosses another not at grade. *RCW 81.53.010*

$Section\ 2-Respondent's\ Information$

Burlington Northern Santa Fe Railway				
Respondent				
2454 Occidental Avenue South, Suite 2D Street Address				
Seattle, WA 98134				
City, State and Zip Code				
Same Mailing Address, if different than the street address Stephen Semenick, Manager of Public Projecs				
Contact Person Name				
206-625-6413 Stephen.Semenick@BNSF.com				
Contact Phone Number and Email Address				
Section 3 – Proposed or Existing Crossing Location				
1. Existing highway/roadway Old Highway 99 North				
2. GPS location Lat: 48.51194 N. Long: 122.33750 W				
3. Railroad mile post (nearest tenth) MP 74.62				
4. City Burlington County Skagit				

Section 4 – Current Highway Traffic Information

1. Name of highway Old Highway 99 North			
2. Road authority Skagit County			
3. Average annual daily traffic (AADT)6,000			
4. Number of lanes 2			
5. Roadway speed 35 MPH			
6. Is the crossing part of an established truck route? Yes X No			
7. If so, trucks are what percent of total daily traffic? <u>11-16</u>			
8. Is the crossing part of an established school bus route? Yes X No			
9. If so, how many school buses travel over the crossing each day?4			
Section 5 – Crossing Traffic Information			
Section 5 Crossing 2 agreement			
Name of railroad(s) operating at crossing: Burlington Northern Santa Fe, Amtrak			
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Name of railroad(s) operating at crossing: Burlington Northern Santa Fe, Amtrak Type of railroad at crossing			
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1. Name of railroad(s) operating at crossing: Burlington Northern Santa Fe, Amtrak 2. Type of railroad at crossing Common Carrier □ Logging □ Industrial Passenger □ Excursion 5. Type of tracks at crossing Main Line □ Siding or Spur			
1. Name of railroad(s) operating at crossing: Burlington Northern Santa Fe, Amtrak 2. Type of railroad at crossing ☑ Common Carrier ☐ Logging ☐ Industrial ☑ Passenger ☐ Excursion 5. Type of tracks at crossing ☑ Main Line ☐ Siding or Spur 6. Number of tracks at crossing ☐ 1			
1. Name of railroad(s) operating at crossing: Burlington Northern Santa Fe, Amtrak 2. Type of railroad at crossing ☑ Common Carrier ☐ Logging ☐ Industrial ☑ Passenger ☐ Excursion 5. Type of tracks at crossing ☑ Main Line ☐ Siding or Spur 6. Number of tracks at crossing ☐ 1 7. Average daily train traffic, freight ☐ 18			
1. Name of railroad(s) operating at crossing: Burlington Northern Santa Fe, Amtrak 2. Type of railroad at crossing ☑ Common Carrier ☐ Logging ☐ Industrial ☑ Passenger ☐ Excursion 5. Type of tracks at crossing ☑ Main Line ☐ Siding or Spur 6. Number of tracks at crossing ☐ 1 7. Average daily train traffic, freight ☐ 18 Authorized freight train speed ☐ 59 ☐ Operated freight train speed ☐ 50 ☐			

Section 6 – Description of Crossing Construction/Reconstruction

1. Describe in detail the reasons for constructing or reconstructing a grade separation at this location (attach additional information sheets to petition as needed):
The existing structure was built in 1936 and the 1,183 foot overpass being comprised of mostly a timber structure with the main span being steel girders. The overpass is structurally deficient with a rating of 3 out of a possible 100. The bridge/overpass was fitted with temporary shoring to allow the current truck traffic to keep using it, or it would have resulted in a load restriction of 8 tons. Due to the age and deteriorating timber structure with 9 red tagged bridge piles and caps, in addition to numerous other yellow tagged timbers. The County has determined for public safety and the increasing cost of maintaining the overpass, it should be replaced. In addition, this is alternative route for I-5 during emergency closures and construction detours. The overpass also provides access to 3 gravel pits, a concrete plant, and asphalt plant that use the overpass on a daily basis. This route also provides access to a truck stop/fueling station that use to overpass from the north for easier access.
2. How far is the nearest alternate access across the tracks from the crossing?
7.8 Miles
3. Describe the alternate access route, including distance and driving time:
The alternate access across the tracks would be to drive to the north and Bow Hill Road to access I-5 and then back south to Cook Road and the intersection of Old Highway 99 North. The driving time for this route is 12 to 15 minutes depending on signals.
There is a parallel road to the east, but it is restricted to trucks due to surface conditions and width, and would require routing trucks over an already busy (12,000) AADT at grade crossing on Cook Road.
4. If new construction, will the proposed crossing eliminate the need for one or more existing crossings? Yes No N/A _X_
5. If so, identify the crossing(s) by USDOT number and state the distance and direction from the proposed crossing.
USDOT No. 967619S, the grade separated crossing would be at the existing location.

to the state of th
6. If the grade separation is replacing an existing at-grade crossing, describe what will happen
with the existing crossing during construction of the grade separation, as well as what will
happen with the crossing surface, signage, and signal equipment once the grade separation is
complete.
This crossing is replacing an existing grade separated crossing.
Tims crossing is replacing all existing grade separated exessing.
6. Who is responsible for long-term maintenance of the grade separation?
6. Who is responsible for long-term maintenance of the grade separation.
critical Publication of the examples
Skagit County Public Works is responsible for all maintenance of the overpass.

Section 7 – Illustration of Crossing

Attach a diagram, map, or other illustration showing the location of the railroad and the proposed/existing location of the crossing. If this is a reconstruction, include design-level drawings of the proposed changes to the grade separation.

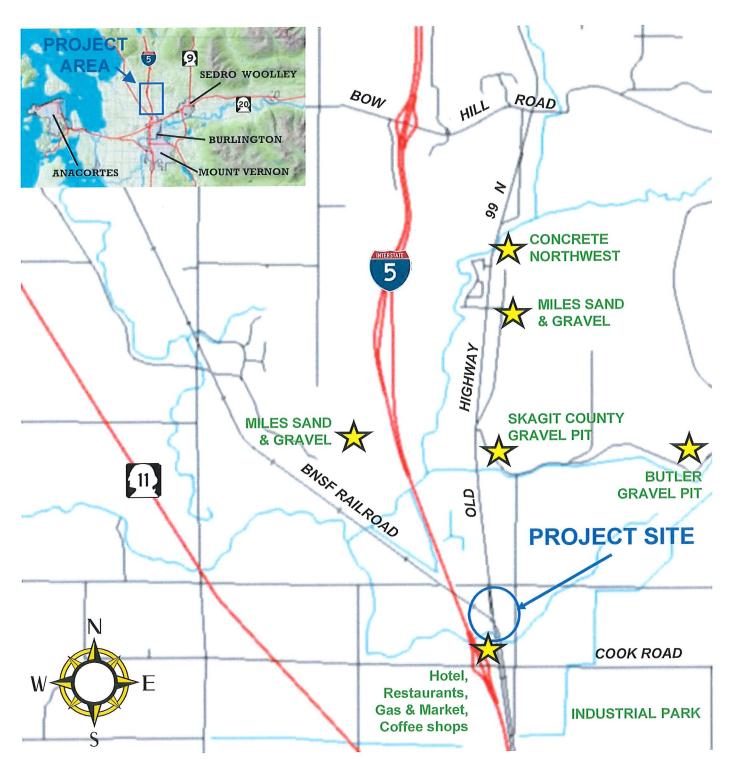
If this is a new grade separation, include the parcels of private property located on both sides of the proposed crossing for a distance of 500' from the crossing and the name and mailing address of each property owner.

Section 8 – Waiver of Hearing by Respondent

Waiver of Hearing
The undersigned represents the Respondent in this petition to construct a highway-rail grade separation.
We have investigated the conditions at the crossing. We are satisfied the conditions are the same as described by the Petitioner in this docket. We do not oppose the proposed grade-separated crossing and consent to a decision by the commission without a hearing.
Dated at, Washington, on the day of, 20
October, 20 18.
Burlington Northern Santa Fe Railway Printed name of Respondent
Signature of Respondent's Representative
Manager Public Projects Title
Title
206-625-6152 stephen.semenick@bnsf.com Phone number and email address
2454 Occidental Ave S, Suite 2D
Seattle, WA 98134 Mailing address

BURLINGTON NORTHERN OVERPASS REPLACEMENT PROJECT

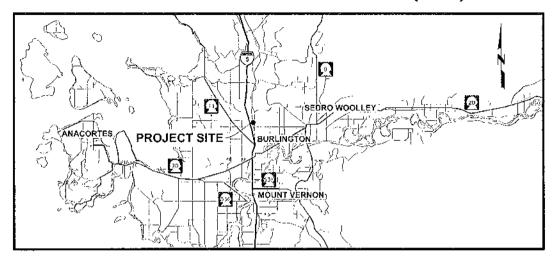


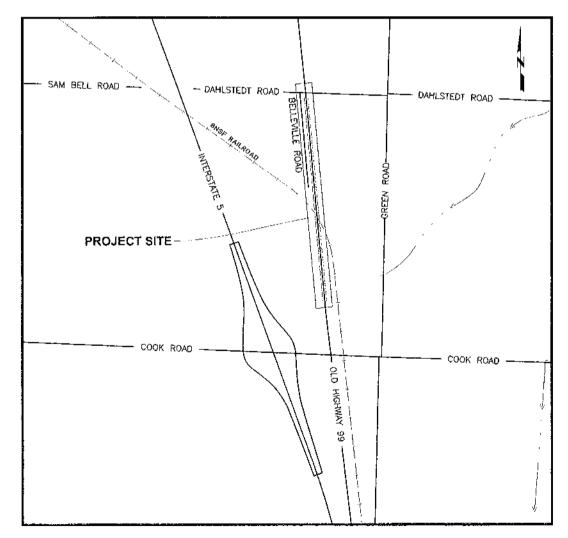


BURLINGTON NORTHERN OVERPASS PROJECT

ES50510-8

FEDERAL AID NUMBER BRS-M291(006)





VICINITY MAP NTS

SKAGIT COUNTY OFFICIALS

BOARD OF COMMISSIONERS

- •HSA JANICKI, CHAIR
- RON WESEN, COMMISSIONER
- · KENNETH A. DARLSTEDT, COMMISSIONER PUBLIC WORKS

*DAN BERENISON, DIRECTOR

Fala. Elik

10.11.16

PLAN SHEET INDEX -

CONTINUED

PIER 3 CAP REINFORCÉMENT PLAM (5-26) PIER 3 RUN-ORCEMENT SECTIONS 1 (S-27) PIER 3 REINFORGEMENT SECTIONS 2 (S-28)

PIFR 3 OFTAILS (S-25)

DECK SECTIONS (S-41)

BARUST 1 OF 3 (S-58) BARLIST 2 OF 3 (5 57) BARLIST 2 OF 3 (S-58) SEW LAYOUT (5-59) SKW 1 IN EVAINOR (\$-60) SEW 2 DIEVATION (S-81) SCW 3 ELEVATION (S 62)

SEW 4 FORVAIVON (S 63)

VAULT DETA/LS (S-68)

SEW TYPICAL SECTIONS (S-65) SEW DETAILS (S-60)

TABUES (S-64)

BRICCE APPROACH SLAB 1 OF 2 (S-42) BRIDGE APPROACH SLAB 2 OF 2 (S-43) EXPANSION JOINT DETAILS 1 OF 2 (S. 44). EXPANSION JOINT DETAILS 2 OF 2 (S-45) TRAFFIC BARRIER 1 OF 3 (\$-46) TRAFFIC BARRIER 2 OF 3 (S-47) TRAFFIC BARRIER 3 OF 3 (S-48) PEDESTRIAN BARRIER 1 OF 4 (S-49) PEDESTRIAN BARRIER 2 OF 4 (S-50) PEDESTRIAN BARRIER 3 OF 4 (S. 51) PEDESTRIAN BARRIER 4 OF 4 (S-52) BRIDGE RAILING TYPE CHAIN LINK FENCE (\$-53)

BROGE RAIFING TYPE S-SB 1 OF 2 (S-54) BRIDGE RAIFING TYPE SISB 2 OF 2 (\$-55)

SEW 5 AND 6 ELEVATION AND ELEVATION

SEW TRAFFIC BARRIER, SEW PEDESTRIAN BARRIER AND SEW COPING (\$-97)

1301 Fifth Avenue, Suite 1200

Seattle, Washington 98101-2677 (206) 357-5600 FAX: (206) 357-5601

FRAMING PLAN 1 OF 2 (\$ 29) FRAMINO PLAN 2 OF 2 (S 30) TYPICAL BRIDGE SECTION (S-31) WF83G C/RDERS 1 OF 4 (S-32) WF83G GIRDERS 2 OF 4 (S-33) WH83C CIRDERS 3 OF 4 (S-34) WE830 CRIDERS 4 OF 4 (S-35) INTERMEDIATE DIAPHRAGM DETAILS (S-36) DECK REINFORCING PLAN 2 OF 4 (S-38) DECK REINFORCING PLAN 3 OF 4 (9-39) DECK REINFORCING PLAN 4 OF 4 (S. 40)

SKAGIT COUNTY PUBLIC WORKS 1800 CONTINENTAL PLACE MOUNT VERNON, WA 98272-5625 (360) 416-1400 FAX (360) 336 9473



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WNSHIP	WNSHIP 35 N, RANGE 04 E, W.M.	

1 INCH SCALE BAR ADJUST SCALE ACCORDINGLY

1 OF 117

PLAN SHEET INDEX

	PLAN SHEET INDEX
SHEET	TITLE _
1	COVER SHEET (CV=01)
2	OUANTITIES (CV-02)
3	LEGEND AND ABBREVIATIONS (CV-03)
4	SURVEY (SV 01)
5	RIGHT-CF-WAY (SV-02)
6	RIGHT-OF-WAY (SV 03)
1	RIGITI-OF-WAY (SVO4)
8	RICHT-OF-WAY (SV-05)
9	RIGHTOFWAY (SV05)
10	RIGHT-OF-WAY (SV 07)
7.1	RIGHT- OFWAY (SV-08)
12	TESC GENERAL NOTES (SP-01)
13	FXISTING CONDITIONS, SITE PREP, & TESC
	SIA "8" 92 H09 TO S7A "8" 94 H20 (SP Q2)
14	EXISTING CONDITIONS, SITE PREP. & TESC
	STA "B" 94+20 TO STA "B" 98+50 (SP-03)
15	EXISTING COMDITIONS, SITE PREP, & TESC
16	SIA "B" 98+50 TO SIA "B" 102+80 (SP-04)
.0	EXISTING CONDITIONS, SITE PREP, & PROC.
17	STA "B" 102+80 TO STA "B" 107+TO (SP-05) EXISTING CONDITIONS, SITE PREP. & TEST
	STA "B" 107+10 TO STA "B" 111+40 (SP -06)
18	EXISTING CONDITIONS, SITE PREP. & 1650
	SIA "B" 111470 TO SIA "B" 113+28 (SP-07)
19	ACCESS AND PHASING PLAN
	PHASE 1 (AC-01)
20	ACCESS AND PHASING PLAN
21	PHASE 2 (AC-02) TYPICAL SECTIONS
21	SHEET 1 (TS 01)
22	TYPICAL SECTIONS
	SHEET 2 (IS 02)
23	MAINTENANCE ROAD CROSS SECTION
	SHEET 1 (TS-03)
24	RESIDENT ACCUSS ROAD CROSS SECTION
	SUB (2 (IS -04)
25	PLAN & PROFILE STA "B" 92+09 FO STA "B" 94+20 (89-01)
25	71AN & PROFILE
23	STA "B" 94+20 (O STA "B" 98+50 (PP-02)
27	PLAN & PROFILE
_	31A "B" 98+50 TO NO2+80 (PP 03)
28	PLAN & PROFILE
	SIA "B" 102∓80 tO SIA "B" 107+10 (PP-04)
29	PLAN & PROFILE
20	STA "8" 107±10 70 STA "8" (11±40 (PP 05)
30	PLAN & PROFILE STA "8" #11#40 TO STA "8" (13428 (PP=06)
18	PLAN & PROFILE MAINTEMANCE ROAD
J1	STA "3" 92:109 TO STA "B" 94+20 (PP-07)
32	PLAN & PROFILE MAINTENANCE ROAD
	STAT "S" QALEO TO STA "D" QSUSO (PPLOS)

STAT "8" 94+20 TO STA "8" 98+50 (PP-08)

PLAN SHEET INDEX -CONTINUED

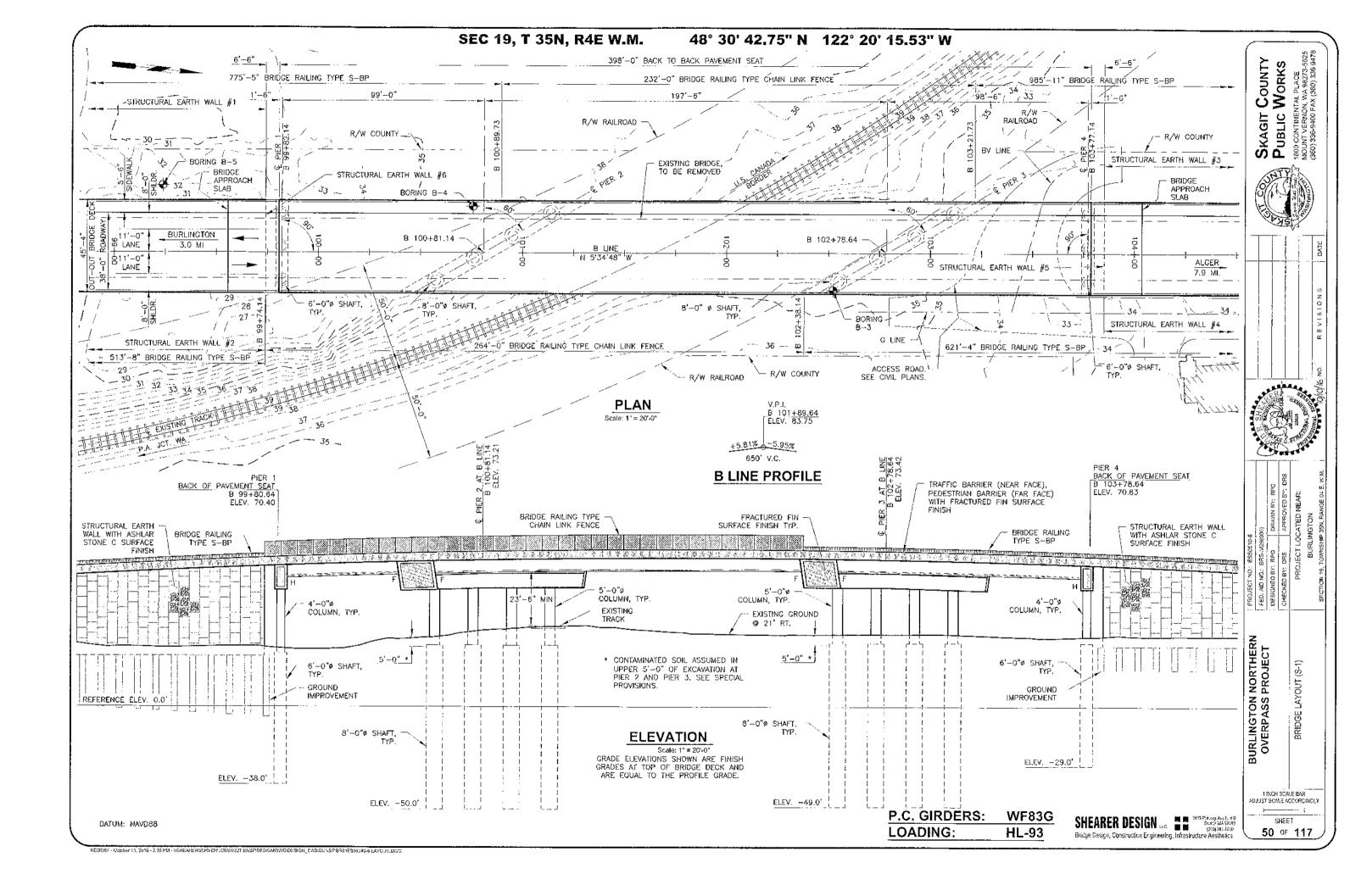
SHEET	TITLE	SHEET 74
33	CHANMELIZATION & SIGNING PLAN	75
	STA "B" 92+09 TO STA "B" 94+20 (CH-01)	76
34	CHANMELIZATION & SIGNING HI AM	77
	STA "B" 94+20 (O S!A "B" 98+50 (CH-02)	73
35	CHANNELIZATION & SIGNING PLAN	79
	STA "B" 98+50 (O STA "B" 102+80 (CB 03)	80
36	CHANMELIZATION & SIGNANG PLAN STA "B" 102+80 10 SPA "B" 107+10 (CH 04)	81
, ,	CBANMETISATION % SIGNING BLVM	82
37	STA "B" 1071-10 TO STA "B" 117+40 (CH-05)	33
38	CHANMELIZATION & SIONING PLAM	84
30	STA "B" (11149) TO STA "B" (113+28 (CH-08)	85
39	RAILROAD ALIGNMENT (RR -01)	36
49	RAILROAD PROFILL (RR 02)	87
41	DRAIMAGE DETAILS SHELL ((DD-01)	88
12	DRAMACE OFTAILS SHEET 2 (DD-02)	89
43	DRAINAGE DETAILS SHEET 2 (DD-02)	90
44	DRAINACE DETAILS SHEET 4 (DD-04	90 91
45	DETOUR PLAN SHEET 1 (DP-01)	92
46	DEFOUR PLAN SHEET 2 (DP -02)	92
47	DETOUR PLAN SHELF 3 (DP-93)	95 94
48	. ,	
10 49	DEFOUR PLAN SHEET 4 (DP-04) WEBLAND RESTORATION PLAN	95 96
49	SULLI 1 (WR-01)	
50	BRIGGE DYOUL (S-1)	97
51	HRIDGE CENERAL NOTES (S-2)	98
52	CONSTRUCTION SEQUENCE 1 OF 5 (S 3)	99
53	CONSTRUCTION SEQUENCE 2 OF 5 (S-4)	100
54	CONSTRUCTION SEQUENCE 3 OF 5 (S=5)	101
55	CONSTRUCTION SEQUENCE 5 OF 5 (5-5)	102
55	CONSTRUCTION SEQUENCE 5 OF 5 (S=7)	10.5
57	, ,	104
58	FOUNDATION LAYOUT (S=8)	105
59	SHALL DETAILS PER 1 AND 4 (\$ -9)	106
60 60	SHAFT DETAILS PIR 2 AND 3 (S-10)	107
51	COLUMN DEMILS (S-11)	108
	PER 1 AND 4 PLAN (S=12)	109
62	PIER 1 AND 4 PERVATION (S. 13)	110
63	PIER 1 AND 4 SECTIONS (S-14)	111
64	PER 1 AND 4 DETAILS : OF 2 (S 15)	112
65	PSR 1 AMD 4 OFFAIRS 2 OF 2 (S-16)	113
66	PER 2 PLAN (3-17)	
67	PYER 2 ELEVATION (S +8)	114
68	PVER 2 DETAILS (S. 19)	· 115
69	PIER 2 CAP REIMFORGEMENT PLAN (S-20)	116
70	PIER 2 REINFORGEMENT SECTIONS 1 (S-21)	
71	PIER 2 REINFORCEMENT SECTIONS 2 (S 22)	117
72	PER 3 POAM (S-93)	
7.5	PIER 3 ELEVATION (S-24)	
	CALL PWO (2)	
	BUSINESS DAYS	

1--800 424 5555

NAVD 88



🕡 BergerABAM



CONSTRUCTION NOTES:

- 1. ALL MATERIAL AND WORKMANSHIP SHALL CONFORM TO THE WASHINGTON STATE. DEPARTMENT OF TRANSPORTATION (WSDOT) 2016 STANDARD SPECIFICATIONS FOR ROAD. BRIDGE AND MUNICIPAL CONSTRUCTION AND AMENDMENTS, THE CURRENT (WSDOT) STANDARD PLANS, AND SKAGIT COUNTY STANDARDS UNLESS INDICATED OTHERWISE BY THE CONTRACT DOCUMENTS.
- 2. UNDERCROUND UTILITIES MAY EXIST IN THE AREA OF CONSTRUCTION. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO CONTACT ANY AND ALL UTILITIES IN THE AREA AND FIELD VERIFY THESE LOCATIONS PRIOR TO CONSTRUCTION. THE ONE-CALL NUMBER FOR UNDERGROUND UTILITIES IS: 1-800-424-5555. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO NOTIFY THE ENGINEER PROMPTLY OF ANY CONFLICT.
- 3. THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING THE INTEGRITY OF ABJACENT UTILITIES WHICH MAY INCLUDE, BUT ARE NOT LIMITED TO: WATER, SEWER, STORM SEWER, TRAFFIC CONTROL SIGNS, POWER, TELEPHONE, CABLE TV. IRRIGATION. AND STREET LIGHTING.
- 4. THE CONTRACTOR IS REQUIRED TO HAVE A COMPLETE SET OF THE APPROVED ROAD AND BRIDGE PLANS ON SITE WHENEVER CONSTRUCTION IS IN PROCRESS.
- 5. IF THE CONTRACTOR DISCOVERS ANY DISCREPANCIES BETWEEN THE PLANS AND THE EXISTING CONDITIONS ENCOUNTERED, THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE
- 6. STORM WATER POLLUTION PREVENTION PLAN SHALL BE INSTALLED PRIOR TO STARTING ANY FARTHWORK
- ACCESS FOR AREA RESIDENTS AND BUSINESSES SHALL BE MAINTAINED AT ALL TIMES. THE CONTRACTOR SHALL NORFY RESIDENTS AND BUSINESSES 48 HOURS IN ADVANCE ANY WORK AFFECTING ACCESS OR SERVICE AND SHALL MINIMIZE INTERRUPTIONS. TO DRIVEWAYS FOR RESIDENTS AND BUSINESSES ADJACENT TO THE PROJECT.
- 8. ALL OUT AND FILL SLOPES SHALL BE SEEDED AND FERTILIZED FOR EROSION CONTROL. THE CONTRACTOR IS RESPONSIBLE FOR SLOPE EROSION PROTECTION UNTIL VEGETATION
- 9. UNLESS OTHERWISE SHOWN IN THE PLANS, THE CONCRETE COVER MEASURED FROM THE FACE OF THE CONCRETE TO THE FACE OF ANY REINFORCING STEEL SHALL BE 21/2 INCHES AT THE TOP OF THE BRIDGE SLAB, 1 INCH AT THE BOTTOM THE BRIDGE SLAB, 3 INCHES FOR CONCRETE CAST AGAINST FARTH, 2 INCHES FOR CONCRETE FORWED AND BACKFILLED, 13%" AT STIRRUP TIE BARS AND 2 INCHES AT ALL OTHER LOCATIONS.
- 10. PROVIDE % INCH CHAMFER AT ALL EXPOSED CONCRETE EDGES.
- 11. ALL DIMENSIONS ON THESE DRAWING SHALL TAKE PRECEDENCE OVER INDICATED SCALE. DRAWINGS SHALL NOT BE SCALED FOR CONSTRUCTION PURPOSES, NO LENGTH OR SHAPE IS IMPLIED UNLESS IT IS DIMENSIONED.

GENERAL NOTES

AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS 2014 7TH EDITION

SEISMIC DESIGN PER AASHTO GUIDE SPECIFICATIONS FOR LIFED SEISMIC BRIDGE DESIGN, 2011 2ND EDITION WITH 2012-2015 INTERIM REVISIONS.

WASHINGTON DEPARTMENT OF TRANSPORTATION BRIDGE DESIGN MANUAL, APRIL 2015 EDITION.

DESIGN INCLUDES 25 PSF FOR FUTURE WEARING SURFACE. DEAD ! OAD:.....

TIVE LOAD:...... FI: 93

SEISMIC PARAMETERS:..... SITE CLASS "O"

PGA = 0.32g (0.0 SEC.) SS = 0.73q (0.2 SEC.)

= 0.24g (1.0 SEC.)

EARTHQUAKE RESISTING

SYSTEM:..

THE BRIDGE IS DESIGNED AS TYPE 1: DUCTILE SUBSTRUCTURE WITH ESSENDALLY FLASTIC SUPERSTRUCTURE

PLASTIC HINGING AT THE TOP OF THE COLUMN ADJACENT TO THE CAP BEAM, AND AT THE BOTTOM OF THE COLUMN AT THE

TOP OF SHAFT.

ASTM AZOS REINFORCEMENT:....

SPACE REINFORCING STEFT, EVENLY UNLESS OTHERWISE NOTED.

CONCRETE:..... CLASS 4000 WITH AIR IN COLUMNS, FOOTINGS, PIER 2 AND

PIER 3 CAP BEAMS, BARRIERS, AND APPROACH SLABS.

CLASS 6000 WITH AIR IN PIER 1 AND PIER 4 CAP BEAMS

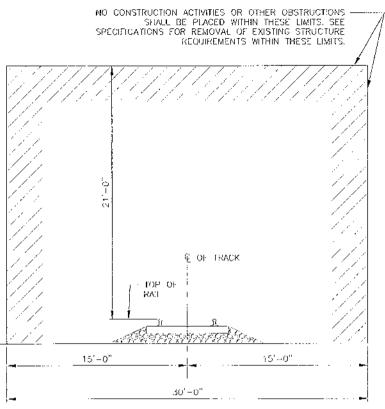
CLASS 4000D IN DECK.

CLASS 4000P IN SHAFTS.

PRECAST CONCRETE:..... SEE GIRDER DRAWINGS

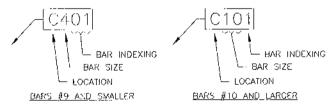
RAILROAD NOTES:

- ANY SHORING SYSTEM THAT IMPACTS THE RAILROAD'S OPERATION AND/OR SUPPORTS THE RAILROAD'S EMBANKMENT SHALL BE DESIGNED AND CONSTRUCTED PER RAILROAD GUIDELINES FOR TEMPORARY SHORING.
- ALL DEMOLITION WITHIN THE RAILROAD'S RICHT-OF-WAY AND/OR DEMOLITION THAT MAY IMPACT THE RAILROAD'S TRACKS OR OPERATIONS. SHALL COMPLY WITH THE RALROAD'S DEMOLITION REQUIREMENTS.
- ERECTION OVER THE RAILROAD'S TRACK SHALL BE PLANNED SUCH THAT IT ENABLES THE TRACK(S) TO REMAIN OPEN TO TRAFFIC PER RALROAD REQUIREMENTS.
- 4. THE ELEVATION OF THE EXISTING TOP-OF-RAIL PROFILE SHALL BE VERIFIED BEFORE BEGINNING CONSTRUCTION, ALL DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE RAILROAD PRIOR TO CONSTRUCTION.
- 5. THE CONTRACTOR MUST SUBMIT A PROPOSED METHOD OF EROSION AND SEDIMENT CONTROL AND HAVE THE METHOD APPROVED BY THE RAILROAD PRIOR TO BEGINNING ANY GRADING ON THE PROJECT SITE
- FOR RAILROAD COORDINATION PLEASE REFER TO SHE RAILROAD'S COORDINATION REQUIREMENTS AS PART OF THE SPECIFICATIONS OR SPECIAL PROVISIONS OF THE PROJECT.
- TEMPORARY CONSTRUCTION CLEARANCES, INC: UDING FALSEWORK CLEARANCES, SHALL COMPLY WITH THE MINIMUM CONSTRUCTION CLEARANCE ENVELOPE SHOWN THIS SHEET, SEE SPECIFICATIONS FOR REMOVAL OF EXISTING STRUCTURE REQUIREMENTS WITHIN THESE LIMITS.
- ALL PERMANENT CLEARANCES SHALL BE VERIFIED BEFORE PROJECT CLOSEOUT, SEE THE SPECIFICATIONS FOR VERIFICATION DOCUMENT.
- FOR RAILROAD STOPPAGE REQUIREMENTS AND DATES SEE SPECIAL PROVISIONS.



MINIMUM CONSTRUCTION CLEARANCE ENVELOPE

LOOKING NORMAL TO RAILROAD



BAR MARK DEFINITION SCALE: N.T.S.

C = COLUMN

D = DECK

P = PIFR

S = SHAFT

SW - SIDEWALK

ELEVATION

EQUAL

FO

ABBF	REVIATIONS		
ð:	AND	F	FIXED
()	ΑT	GTR	GUTTER
Ę	CENTERLINE	Ħ	HINGE
	DEGREE	HQR.	HORIZONTAL
Ø	DIAMETER	MAX.	MAXIMUM
P	PLATE	MIN.	MINIMUM.
ABT.	TUORA	N.T.S.	MO1 TO SCALE
ABUT.	ABUIMENT	O.C.	ON CENTER
APPROX.	·	S.E.W.	STRUCTURAL CARTH WALL
.L.C	CONSTRUCTION JOINT	SPA.	SPACING
CLR.	C. FAR	STA	STATION/STATIONING
EA.	EACH	STD.	STANDARD
E.F.	EACH FACE	TYP.	TYPICAL

1 INCH SCALE BAR VERT. VESTICAL ADJUST SCALE ACCORDINGLY SHEARER DESIGN 51 of 117 Bridge Design, Construction Engineering, Infrastructure Aestnetics

BURLINGTON I OVERPASS F

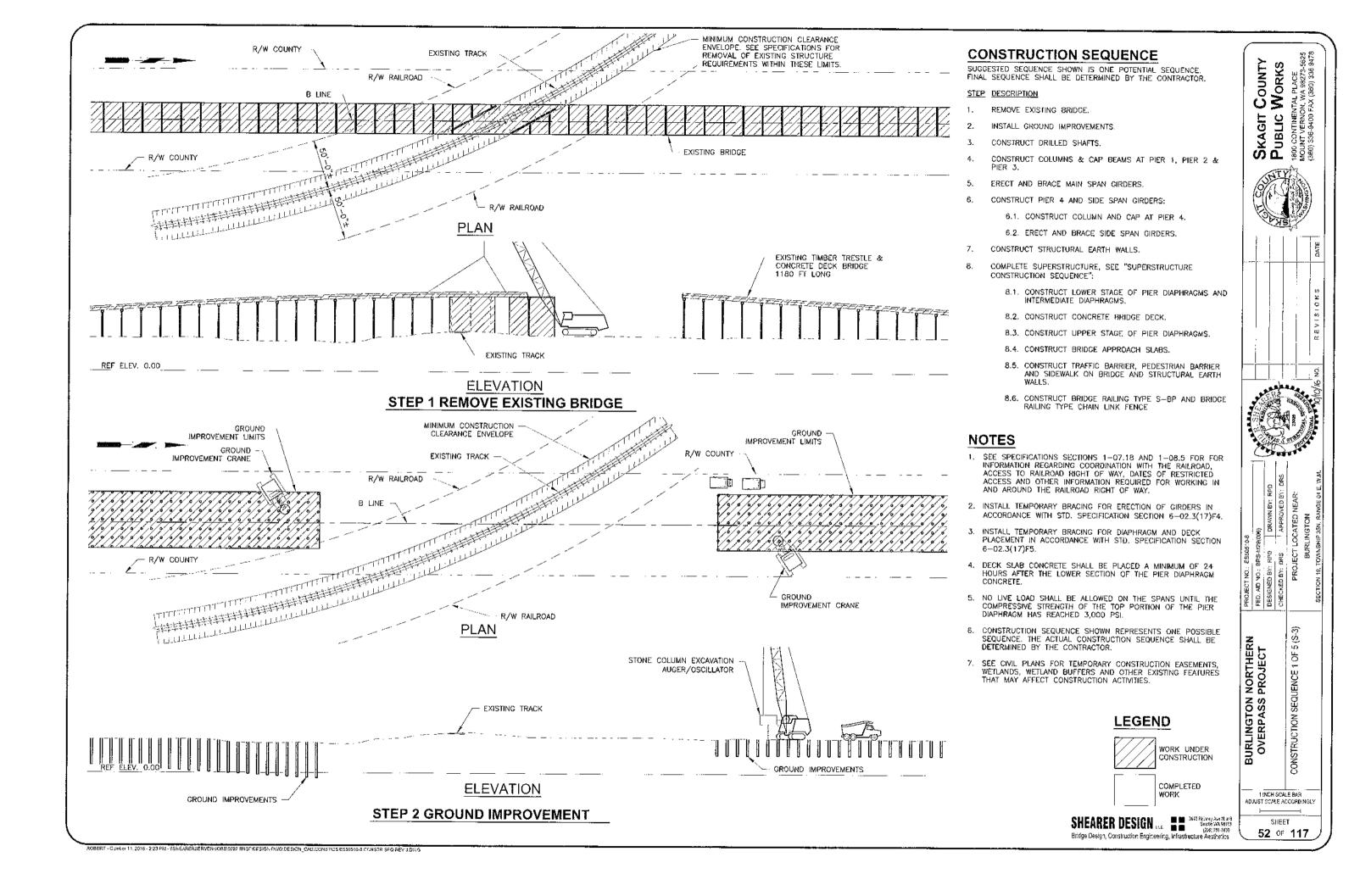
SHEET

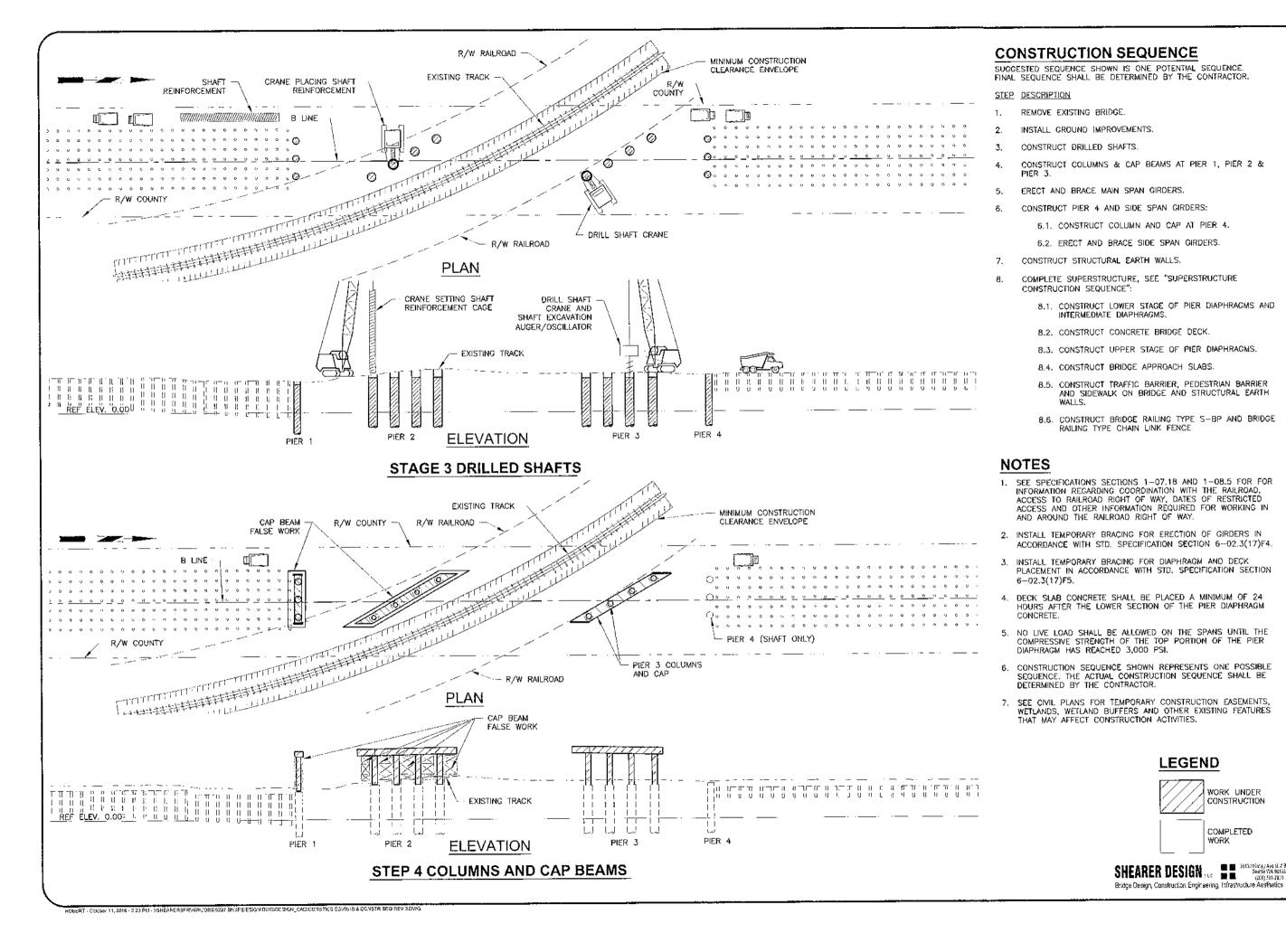
COUNTY WORKS

KAGIT

UBLIC

∆ <u>C</u> 65 (58)





SKAGIT COUNTY
PUBLIC WORKS
1800 CONTINENTAL PLACE
MOUNT VERNON, WA 98273-5625
(780) 376-6400 642, 450) 376-9428

O TO THE REPORT OF THE PARTY OF

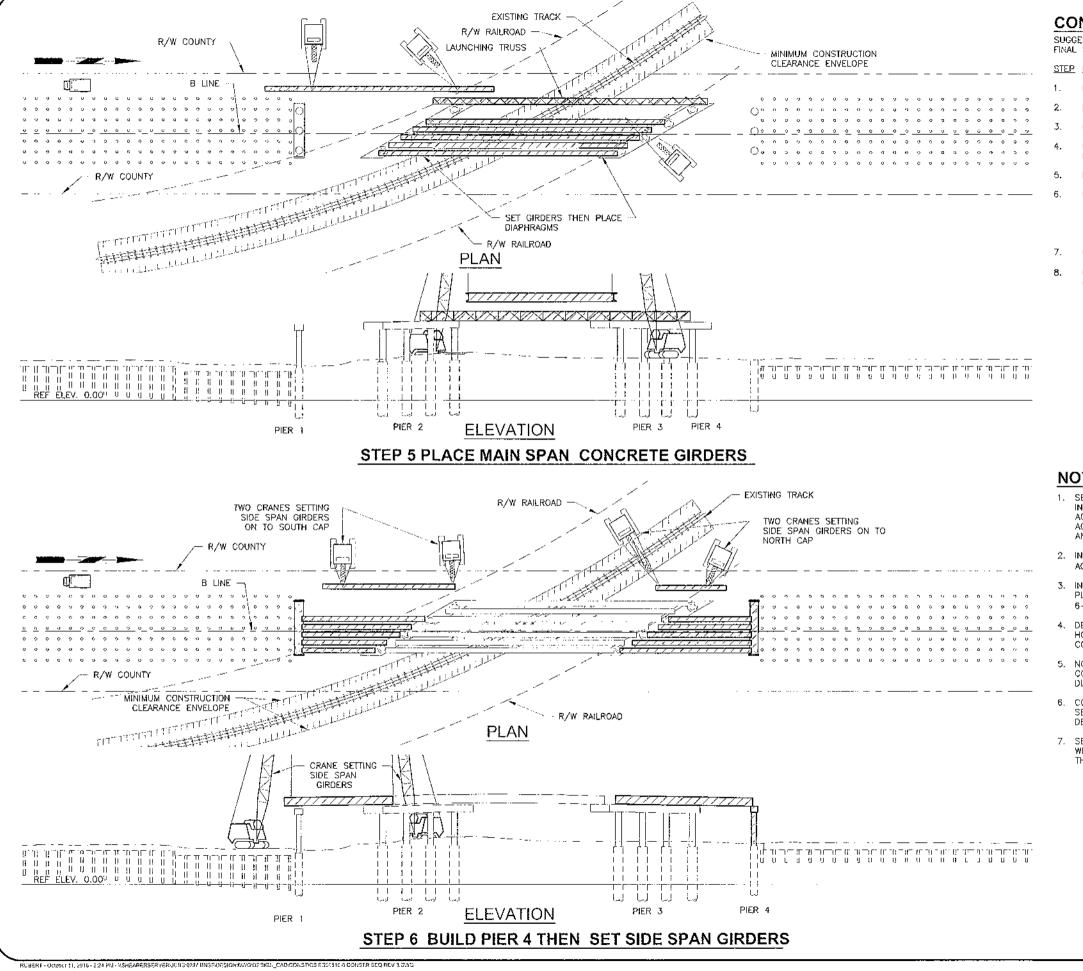
DRAWN BY: RPD
APPROVED BY: DRS
LOCATED NEAR:
RUINGTON

FED. AID NOT: BRS-M204006)
DESIGNED RY: RPD | DRAWN
CHECKED BY: DRS | APPRO
PROJECT LOCATED
BURLINGTON

BURLINGTON NORTHERN
OVERPASS PROJECT
CONSTRUCTION SEQUENCE 2 OF 5 (S-4)

BOKEIN OVER CONSTRUCT

SHEET 53 OF 117



CONSTRUCTION SEQUENCE

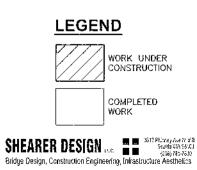
SUGGESTED SEQUENCE SHOWN IS ONE POTENTIAL SEQUENCE. FINAL SEQUENCE SHALL BE DETERMINED BY THE CONTRACTOR.

STEP DESCRIPTION

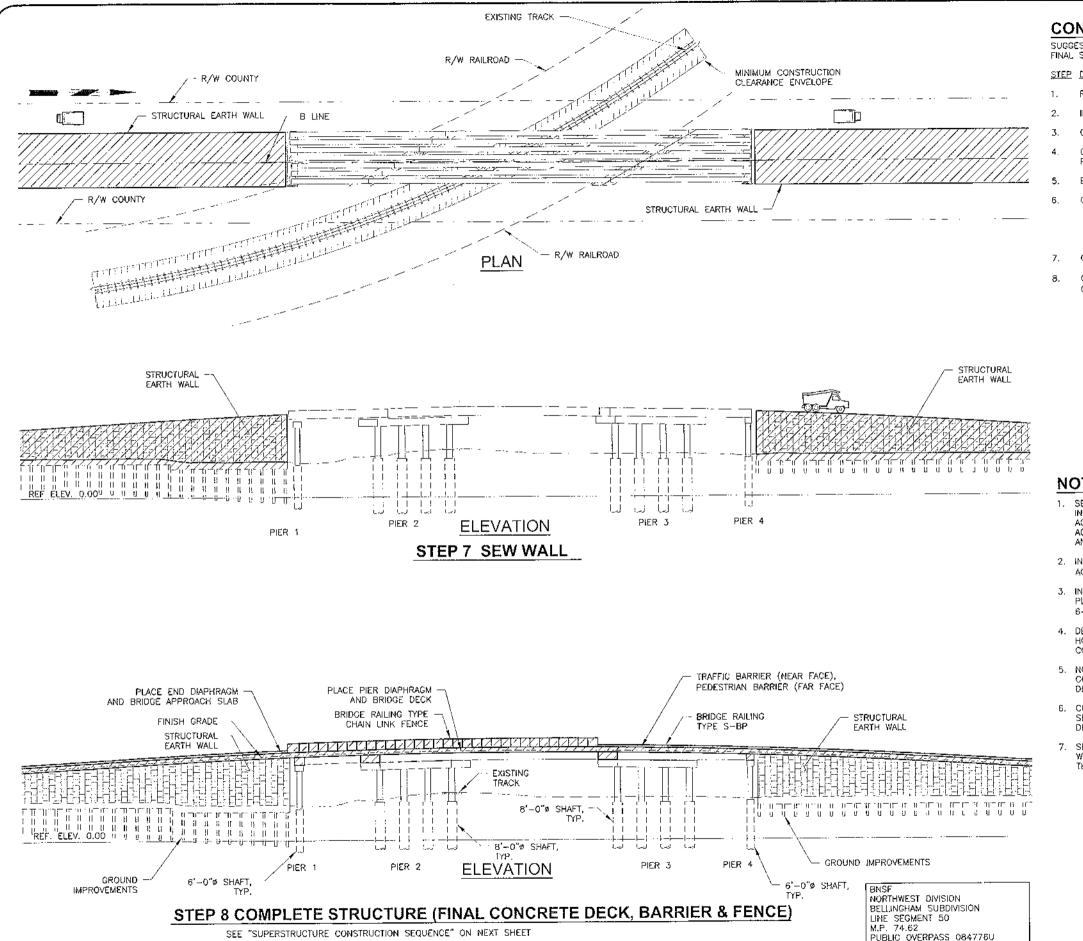
- 1. REMOVE EXISTING BRIDGE.
- INSTALL GROUND IMPROVEMENTS.
- CONSTRUCT DRILLED SHAFTS.
- 4. CONSTRUCT COLUMNS & CAP BEAMS AT PIER 1, PIER 2 & PIER 3.
- 5. ERECT AND BRACE MAIN SPAN GIRDERS.
- 6. CONSTRUCT PIER 4 AND SIDE SPAN GIRDERS:
 - 6.1. CONSTRUCT COLUMN AND CAP AT PIER 4.
 - 6.2. ERECT AND BRACE SIDE SPAN GIRDERS.
- 7. CONSTRUCT STRUCTURAL EARTH WALLS.
- 8. COMPLETE SUPERSTRUCTURE, SEE "SUPERSTRUCTURE CONSTRUCTION SEQUENCE":
 - 8.1. CONSTRUCT LOWER STAGE OF PIER DIAPHRAGMS AND INTERMEDIATE DIAPHRAGMS.
 - 8.2. CONSTRUCT CONCRETE BRIDGE DECK.
 - 8.3. CONSTRUCT UPPER STAGE OF PIER DIAPHRAGMS.
 - 8.4. CONSTRUCT BRIDGE APPROACH SLABS.
 - 8.5. CONSTRUCT TRAFFIC BARRIER, PEDESTRIAN BARRIER AND SIDEWALK ON BRIDGE AND STRUCTURAL EARTH WALLS.
 - 8.6. CONSTRUCT BRIDGE RAILING TYPE S-BP AND BRIDGE RAILING TYPE CHAIN LINK FENCE

NOTES

- SEE SPECIFICATIONS SECTIONS 1-07.18 AND 1-08.5 FOR FOR INFORMATION REGARDING COORDINATION WITH THE RALLROAD, ACCESS TO RAILROAD RIGHT OF WAY, DATES OF RESTRICTED ACCESS AND OTHER INFORMATION REQUIRED FOR WORKING IN AND AROUND THE RAILROAD RIGHT OF WAY.
- INSTALL TEMPORARY BRACING FOR ERECTION OF GIRDERS IN ACCORDANCE WITH STD. SPECIFICATION SECTION 6--02.3(17)F4.
- 3. INSTALL TEMPORARY BRACING FOR DIAPHRAGM AND DECK PLACEMENT IN ACCORDANCE WITH STD. SPECIFICATION SECTION 6-02-3(17)F5
- 4. DECK SLAB CONCRETE SHALL BE PLACED A MINIMUM OF 24 HOURS AFTER THE LOWER SECTION OF THE PIER DIAPHRAGM CONCRETE.
- NO LIVE LOAD SHALL BE ALLOWED ON THE SPANS UNTIL THE COMPRESSIVE STRENGTH OF THE TOP PORTION OF THE PIER DIAPHRAGM HAS REACHED 3,000 PSI.
- CONSTRUCTION SEQUENCE SHOWN REPRESENTS ONE POSSIBLE SEQUENCE. THE ACTUAL CONSTRUCTION SEQUENCE SHALL BE DETERMINED BY THE CONTRACTOR.
- SEE CIVIL PLANS FOR TEMPORARY CONSTRUCTION EASEMENTS, WETLANDS, WETLAND BUFFERS AND OTHER EXISTING FEATURES THAT MAY AFFECT CONSTRUCTION ACTIVITIES.







CONSTRUCTION SEQUENCE

SUGGESTED SEQUENCE SHOWN IS ONE POTENTIAL SEQUENCE. FINAL SEQUENCE SHALL BE DETERMINED BY THE CONTRACTOR.

STEP DESCRIPTION

- REMOVE EXISTING BRIDGE.
- INSTALL GROUND IMPROVEMENTS.
- CONSTRUCT DRILLED SHAFTS.
- CONSTRUCT COLUMNS & CAP BEAMS AT PIER 1, PIER 2 &
- ERECT AND BRACE MAIN SPAN GIRDERS.
- CONSTRUCT PIER 4 AND SIDE SPAN CIRDERS:
 - 6.1. CONSTRUCT COLUMN AND CAP AT PIER 4.
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- CONSTRUCT STRUCTURAL EARTH WALLS.
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 - 8.5. CONSTRUCT TRAFFIC BARRIER, PEDESTRIAN BARRIER AND SIDEWALK ON BRIDGE AND STRUCTURAL EARTH
 - 8.6. CONSTRUCT BRIDGE RAILING TYPE S-BP AND BRIDGE RAILING TYPE CHAIN LINK FENCE

NOTES

BURLINGTON, SKAGIT COUNTY, WA

- SEE SPECIFICATIONS SECTIONS 1-07.18 AND 1-08.5 FOR FOR INFORMATION REGARDING COORDINATION WITH THE RAILROAD, ACCESS TO RAILROAD RIGHT OF WAY, DATES OF RESTRICTED ACCESS AND OTHER INFORMATION REQUIRED FOR WORKING IN AND AROUND THE RAILROAD RIGHT OF WAY.
- 2. INSTALL TEMPORARY BRACING FOR ERECTION OF GIRDERS IN ACCORDANCE WITH STD. SPECIFICATION SECTION 6-02.3(17)F4.
- INSTALL TEMPORARY BRACING FOR DIAPHRAGM AND DECK PLACEMENT IN ACCORDANCE WITH STD. SPECIFICATION SECTION 6-02.3(17)F5.
- 4. DECK SLAB CONCRETE SHALL BE PLACED A MINIMUM OF 24 HOURS AFTER THE LOWER SECTION OF THE PIER DIAPHRAGM CONCRETE
- 5. NO LIVE LOAD SHALL BE ALLOWED ON THE SPANS UNTIL THE COMPRESSIVE STRENGTH OF THE TOP PORTION OF THE PIER DIAPHRAGM HAS REACHED 3,000 PSI.
- 6. CONSTRUCTION SEQUENCE SHOWN REPRESENTS ONE POSSIBLE SEQUENCE. THE ACTUAL CONSTRUCTION SEQUENCE SHALL BE DETERMINED BY THE CONTRACTOR.
- SEE CIVIL PLANS FOR TEMPORARY CONSTRUCTION EASEMENTS, WETLANDS, WETLAND BUFFERS AND OTHER EXISTING FEATURES THAT MAY AFFECT CONSTRUCTION ACTIVITIES.



SHEARER DESIGN to 6. 1 1901 Process, America 8 1901 Pr Bridge Design, Construction Engineering, Infrastructure Aesthetics

COUNTY SKAGIT PUBLIC

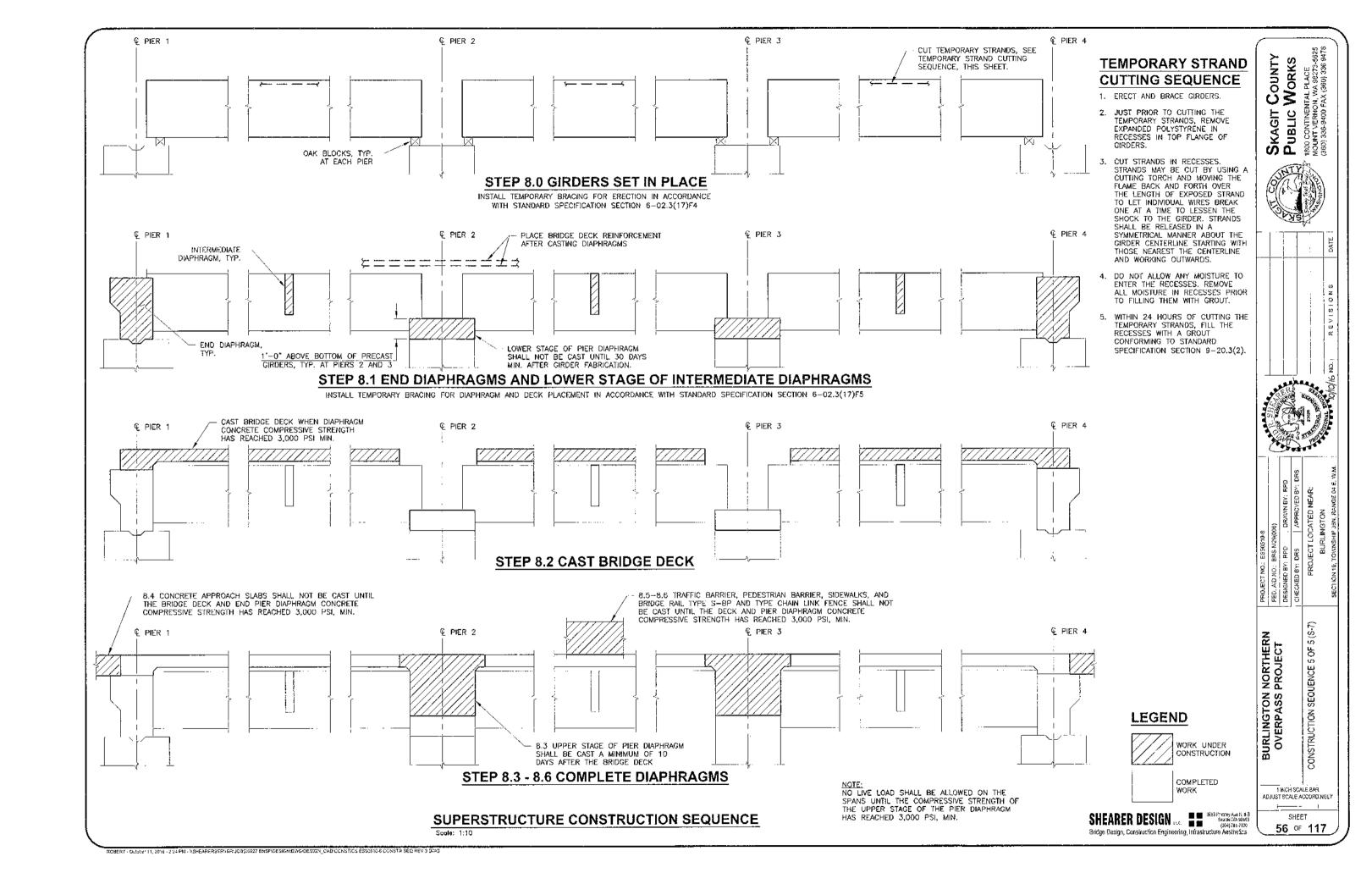


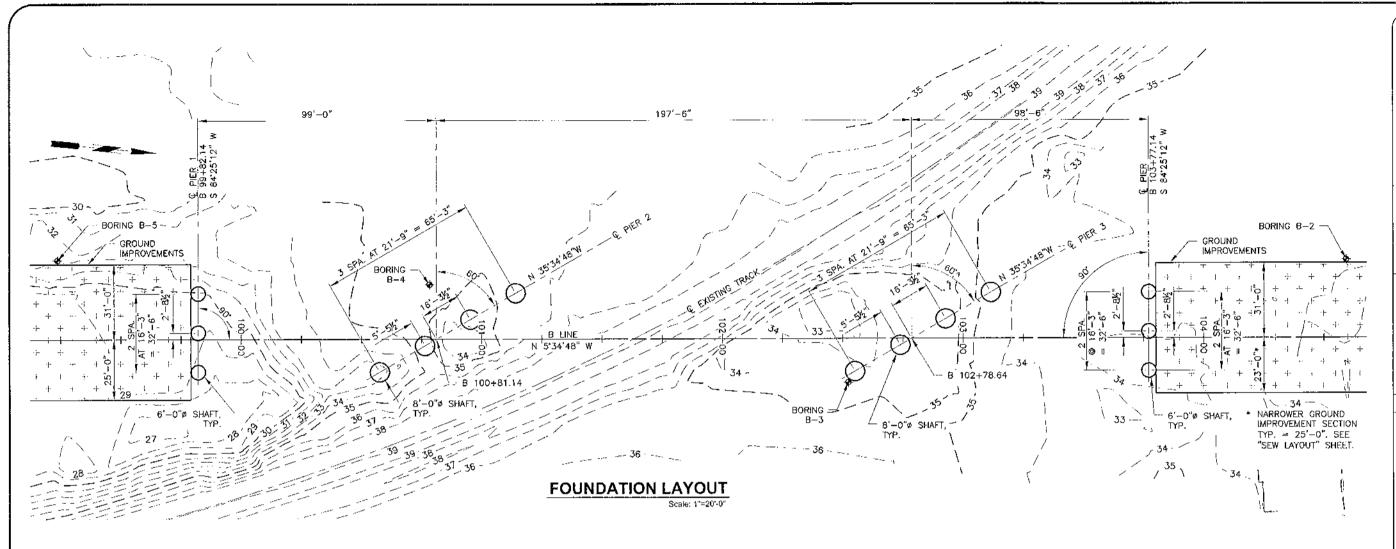
NORTHERN
PROJECT

BURLINGTON I

LINCH SCALE BAR ADJUST SCALE ACCORDINGLY

> SHEET 55 of 117

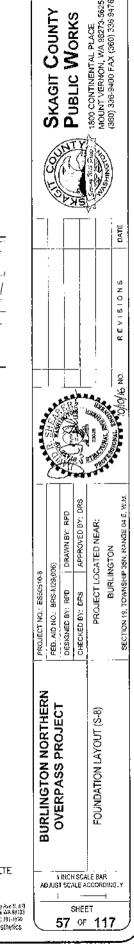


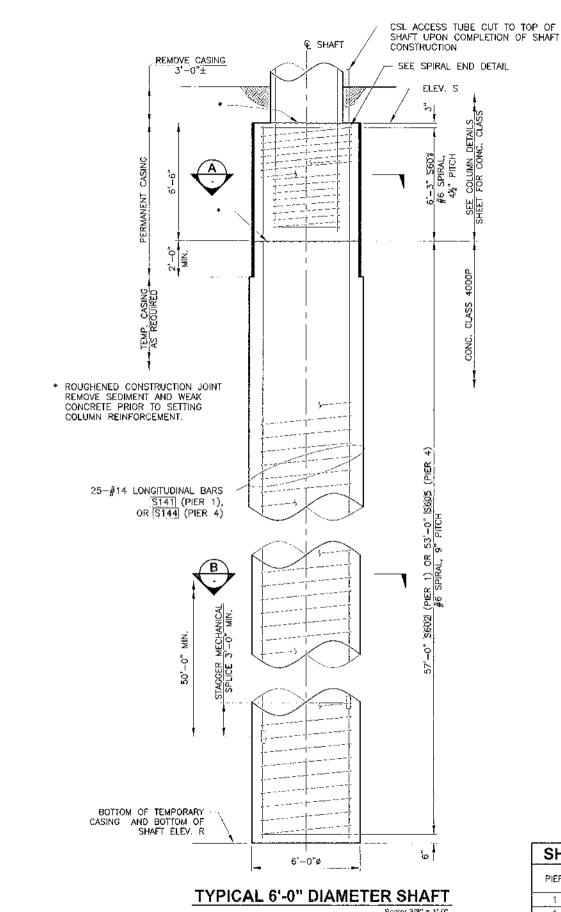


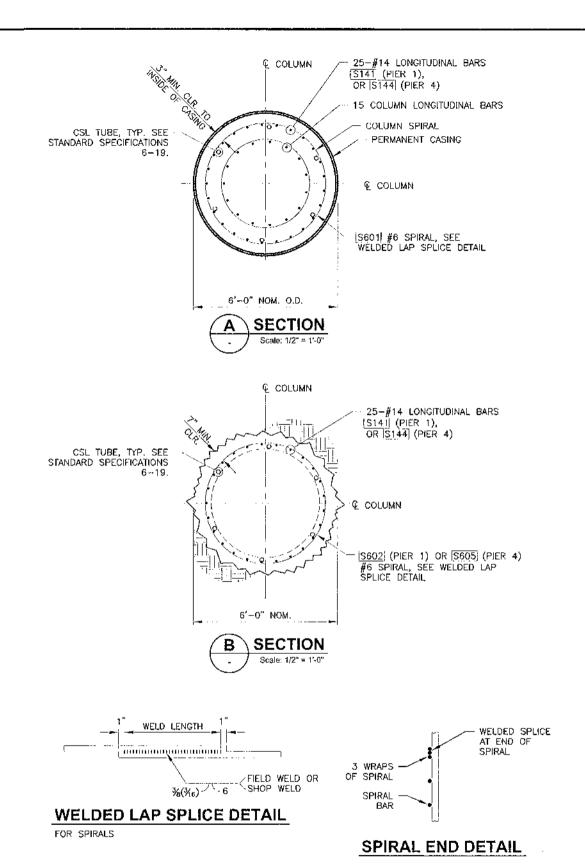
SHAFT FOUNDATION INFORMATION						
PIER NO.	SHAFT DIAMETER, FT	TIP OF DRILLED SHAFT ELEVATION, FT	NOMINAL SHAFT RESISTANCE, TONS			
			STRENGTH 1 LIMIT STATE		EXTREME I LIMIT STATE	
			SKIN FRICTION RESISTANCE	END BEARING RESISTANCE	SKIN FRICTION RESISTANCE	END BEARING RESISTANCE
1	6'-0"	-38.0	400	331	728	662
2	8'0"	-50.0	647	754	1176	1508
3	8'-0"	-49.0	686	754	1248	1508
4	6'-0"	-29.0	357	424	649	848

<u>NOTE:</u>
SEE "SEW LAYOUT" SHEET FOR COMPLETE
LIMITS OF GROUND IMPROVEMENTS.

SHEARER DESIGN LLD. Desire Aventua Assensia (24) Bridge Design, Construction Engineering, Infrastructure Aesthetics







SHAFT ELEVATIONS ELEVATION (FT) --38.0 26.0 31.0 -29.0

NOTE: CONTRACTOR SHALL DESIGN, FURNISH AND INSTALL ALL CASINGS. SEE SPECIAL PROVISIONS.

BURLINGTON NORTHERN OVERPASS PROJECT

1 MICH SCALE BAR ADJUST SCALE ACCORDINGLY

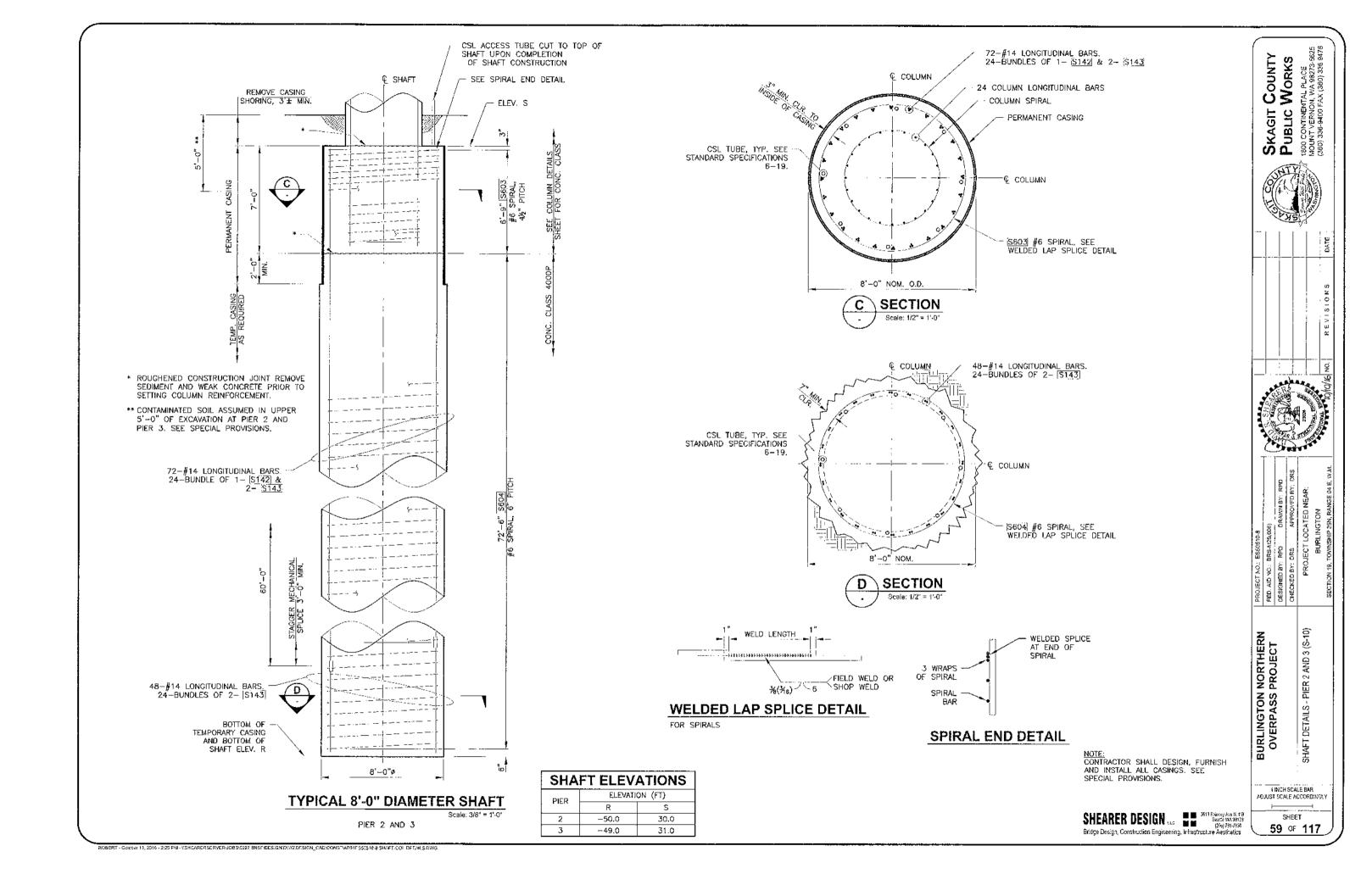
SHEET

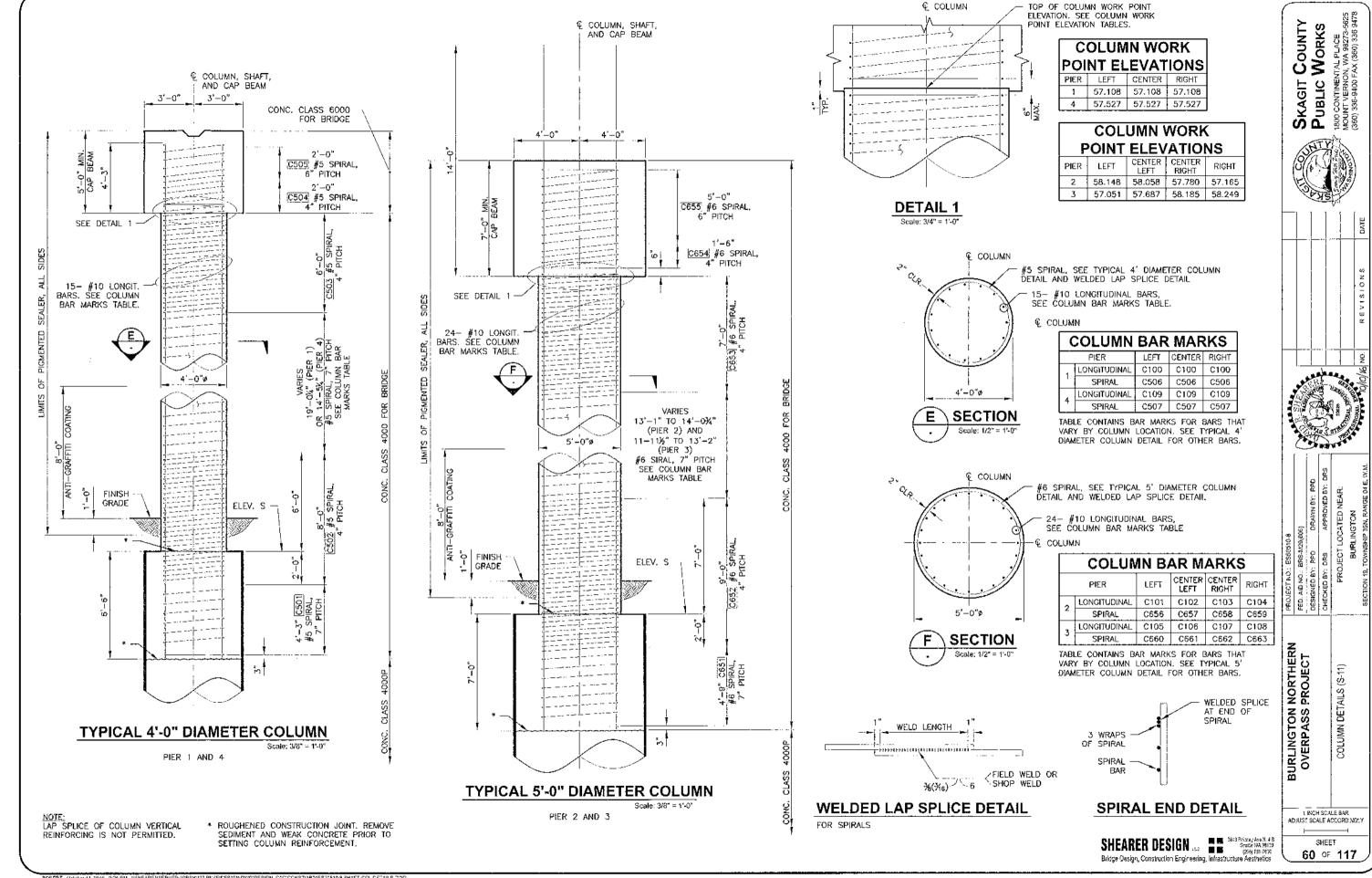
58 OF 117

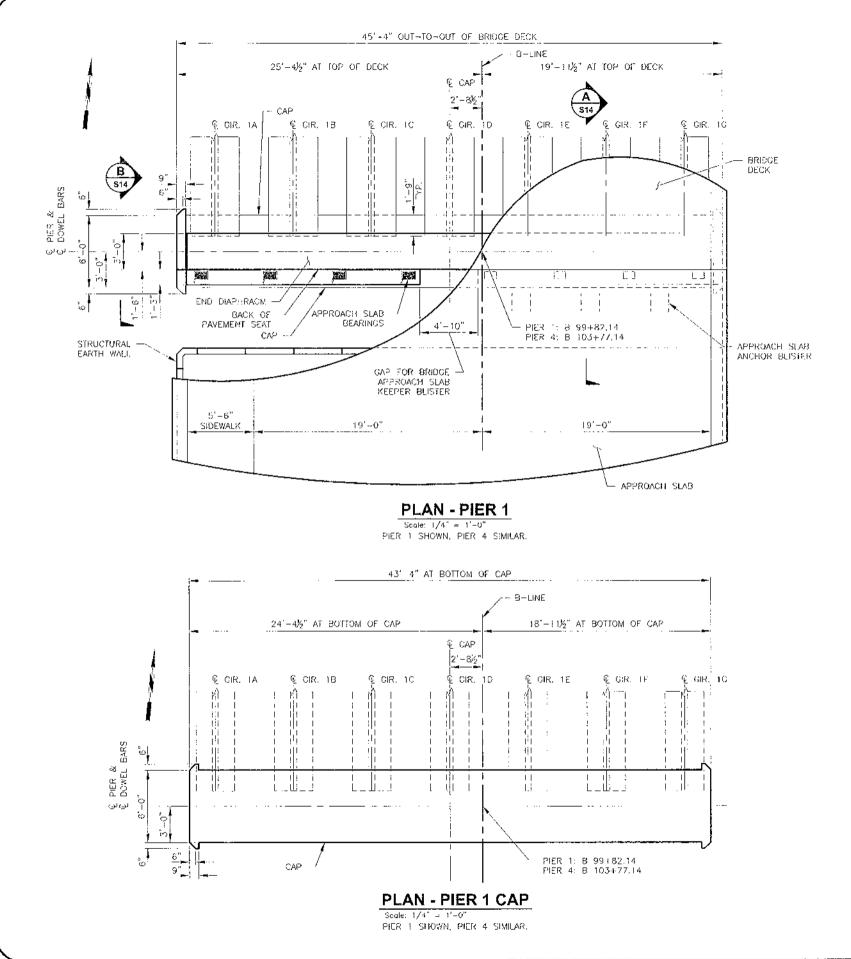
SKAGIT COUNTY
PUBLIC WORKS
1800 CONTINENTAL PLACE
MOUNT VERNON, WA 98273-5625
(380) 336-9400 FAX (380) 336 9478

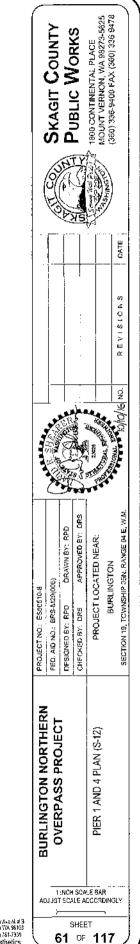
SHEARER DESIGN L.c. 3618 Phinaphae N. 23 Saude Ph. 2670 Saude Ph. 2670 Ph.

PIER 1 AND 4

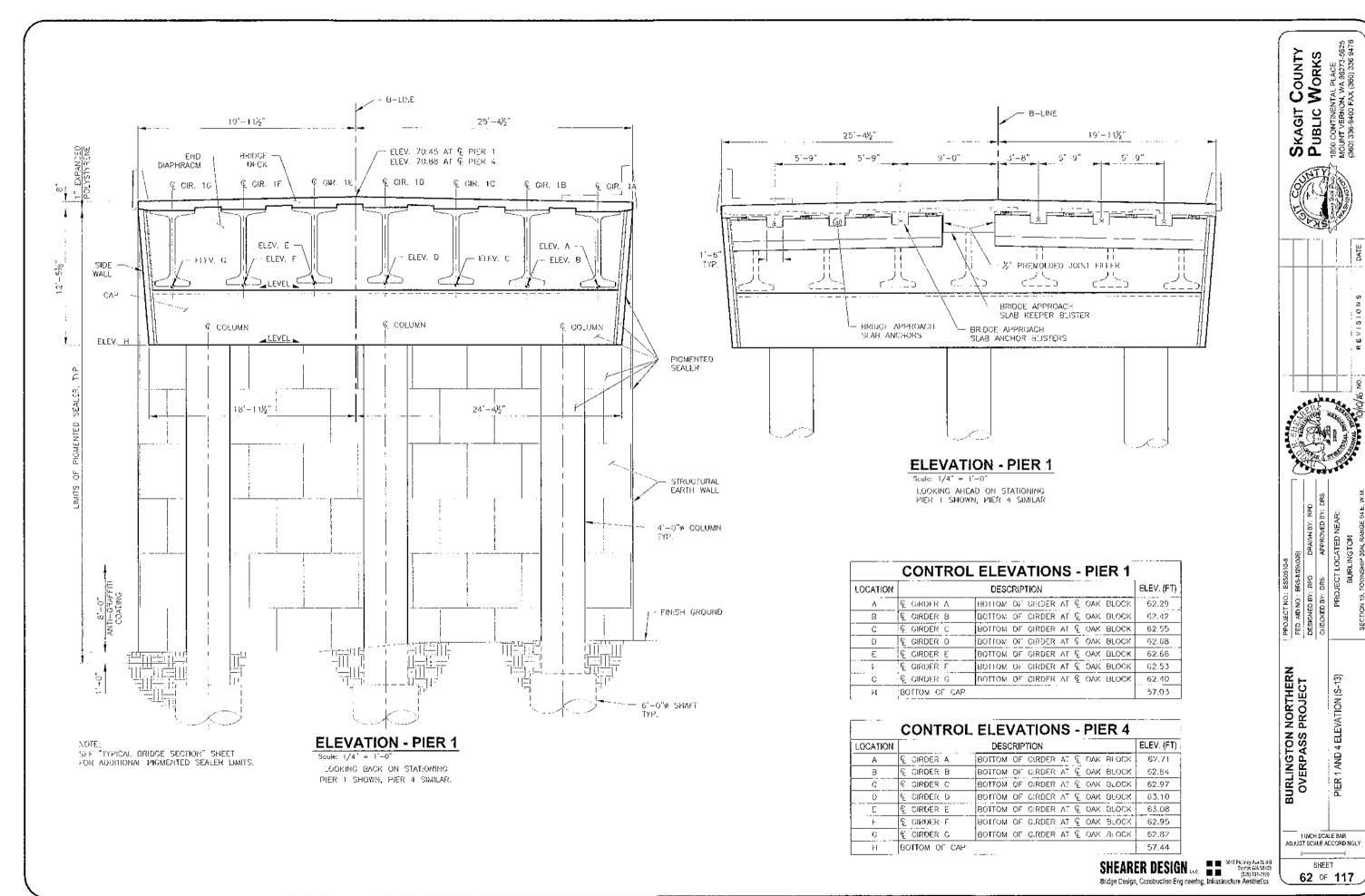


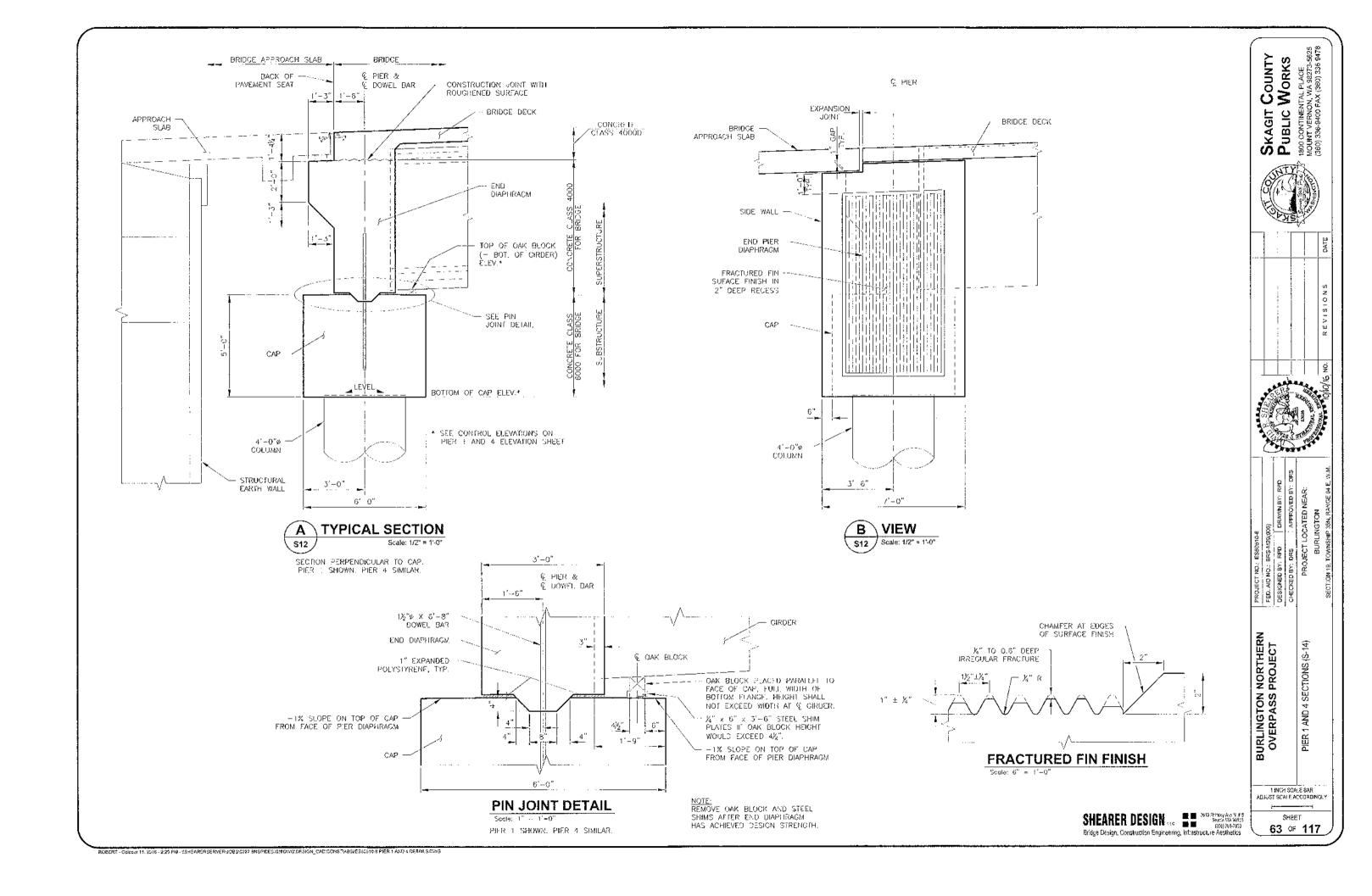


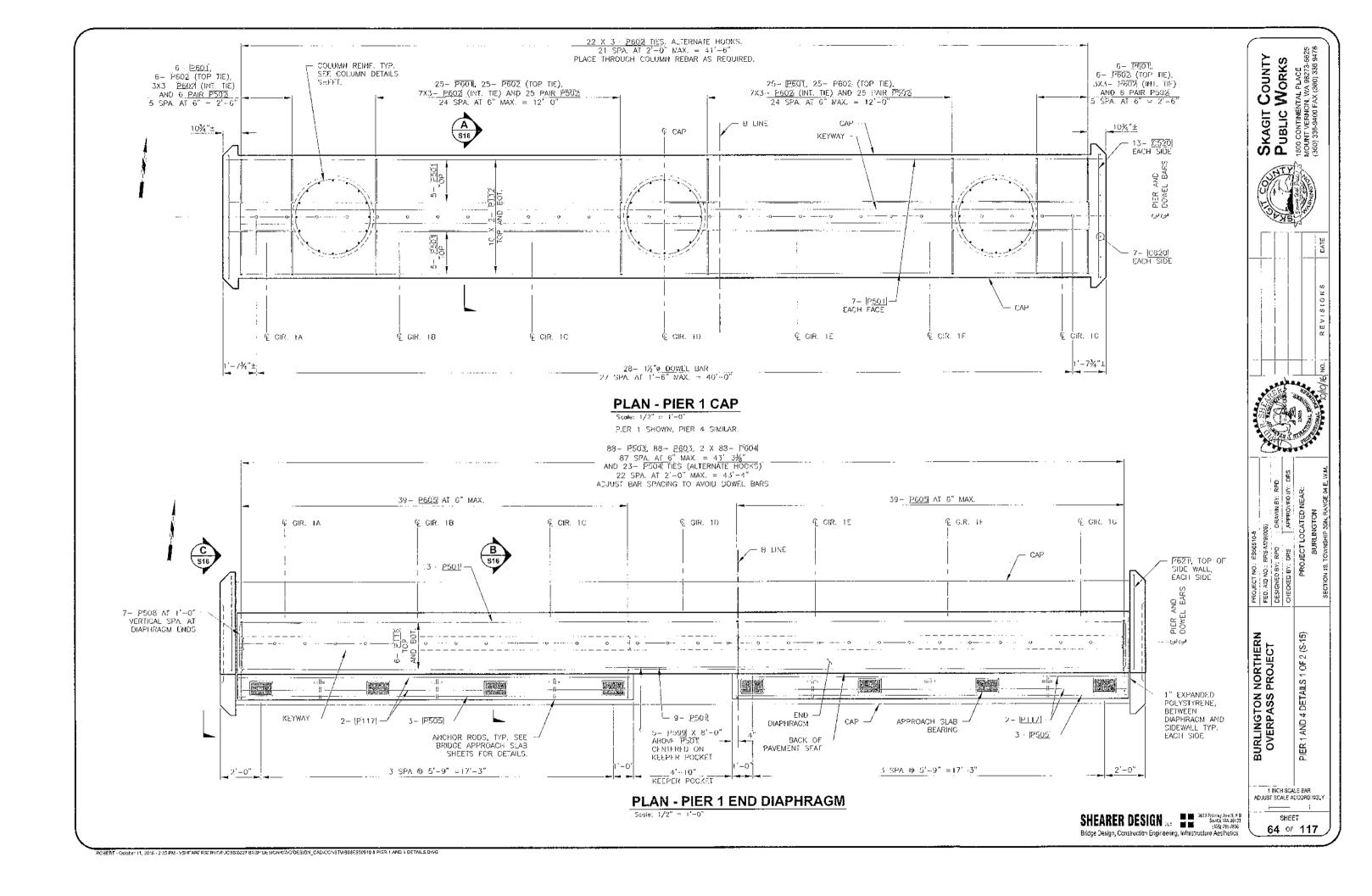


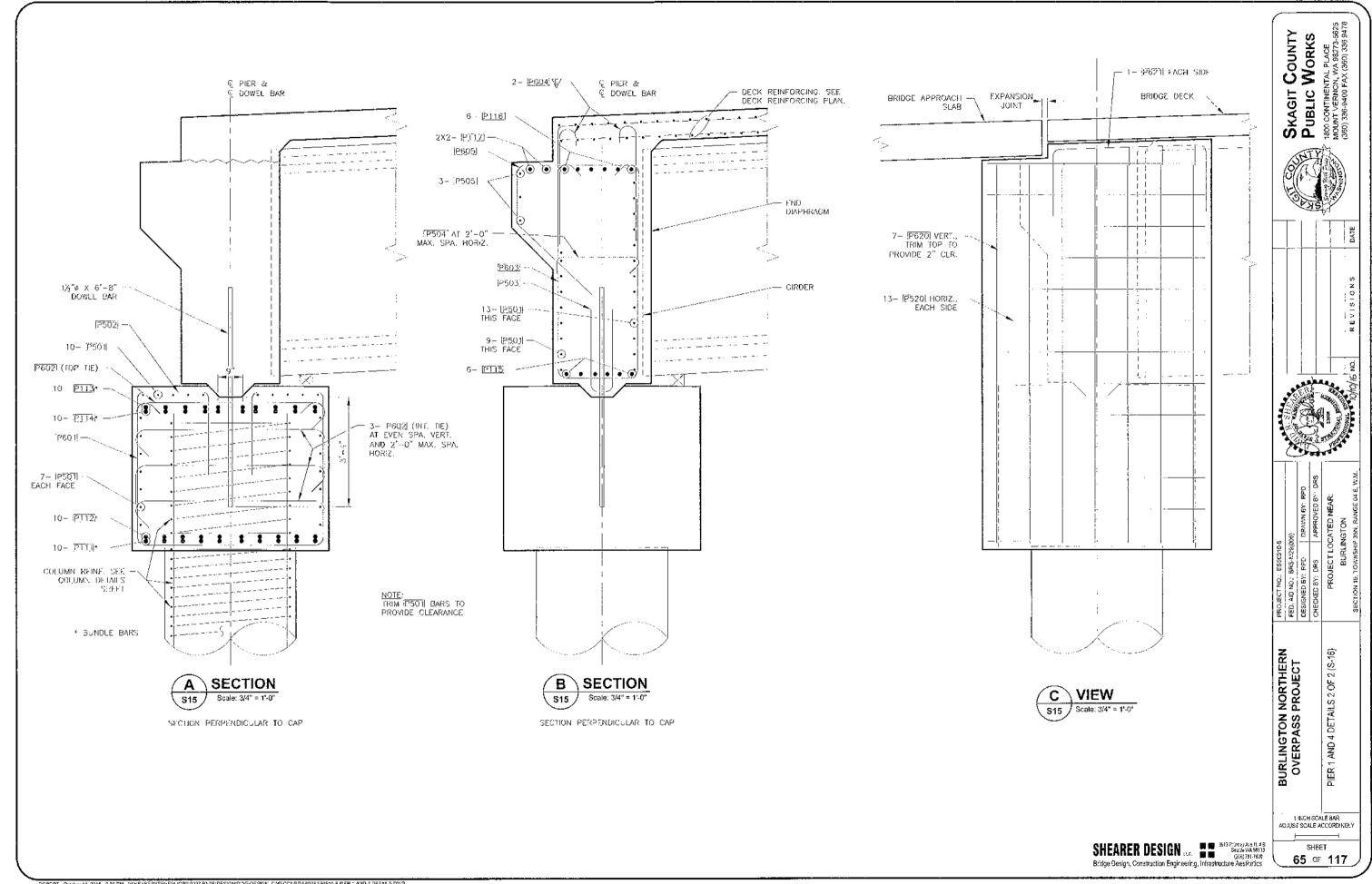


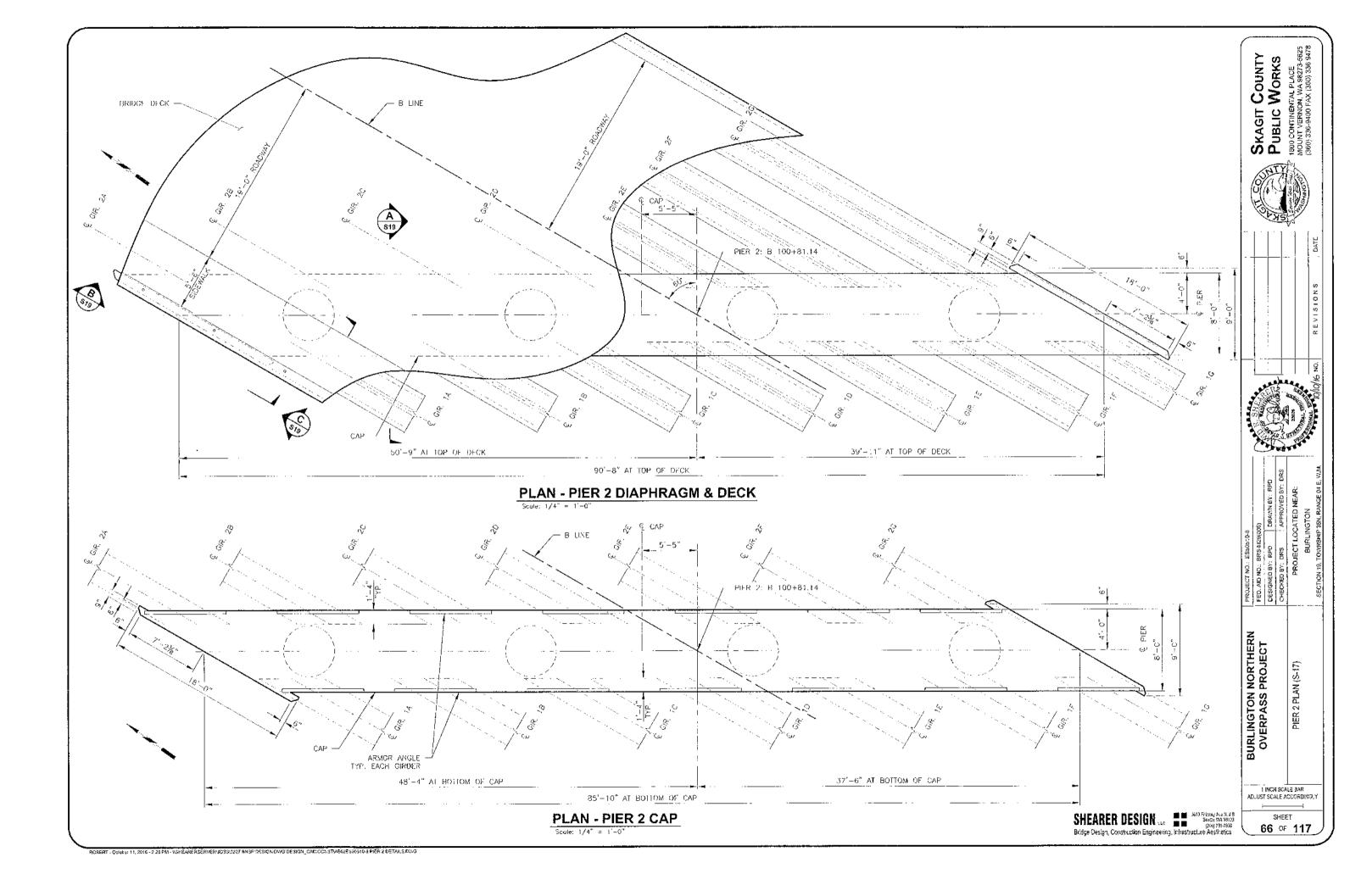
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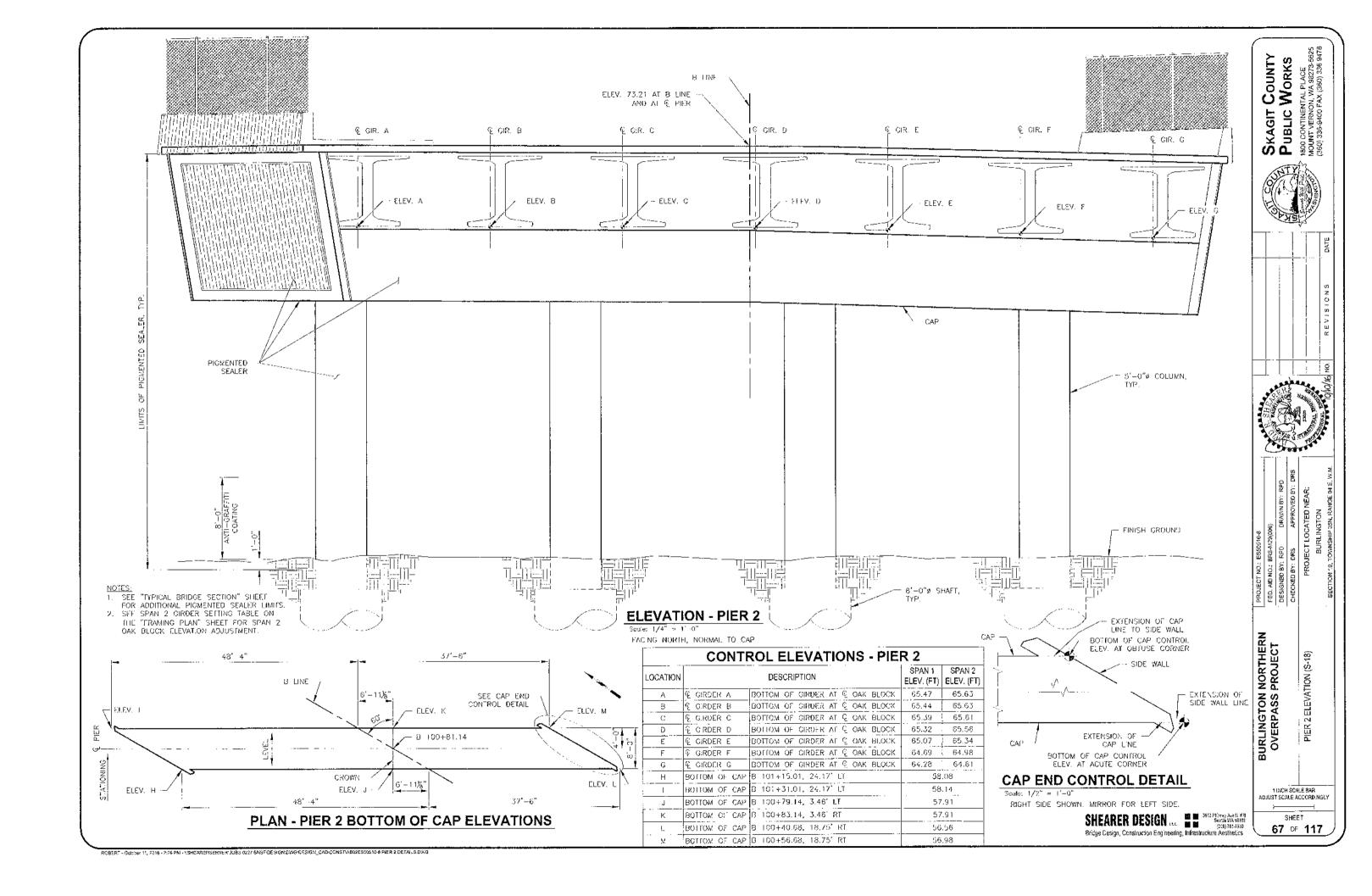


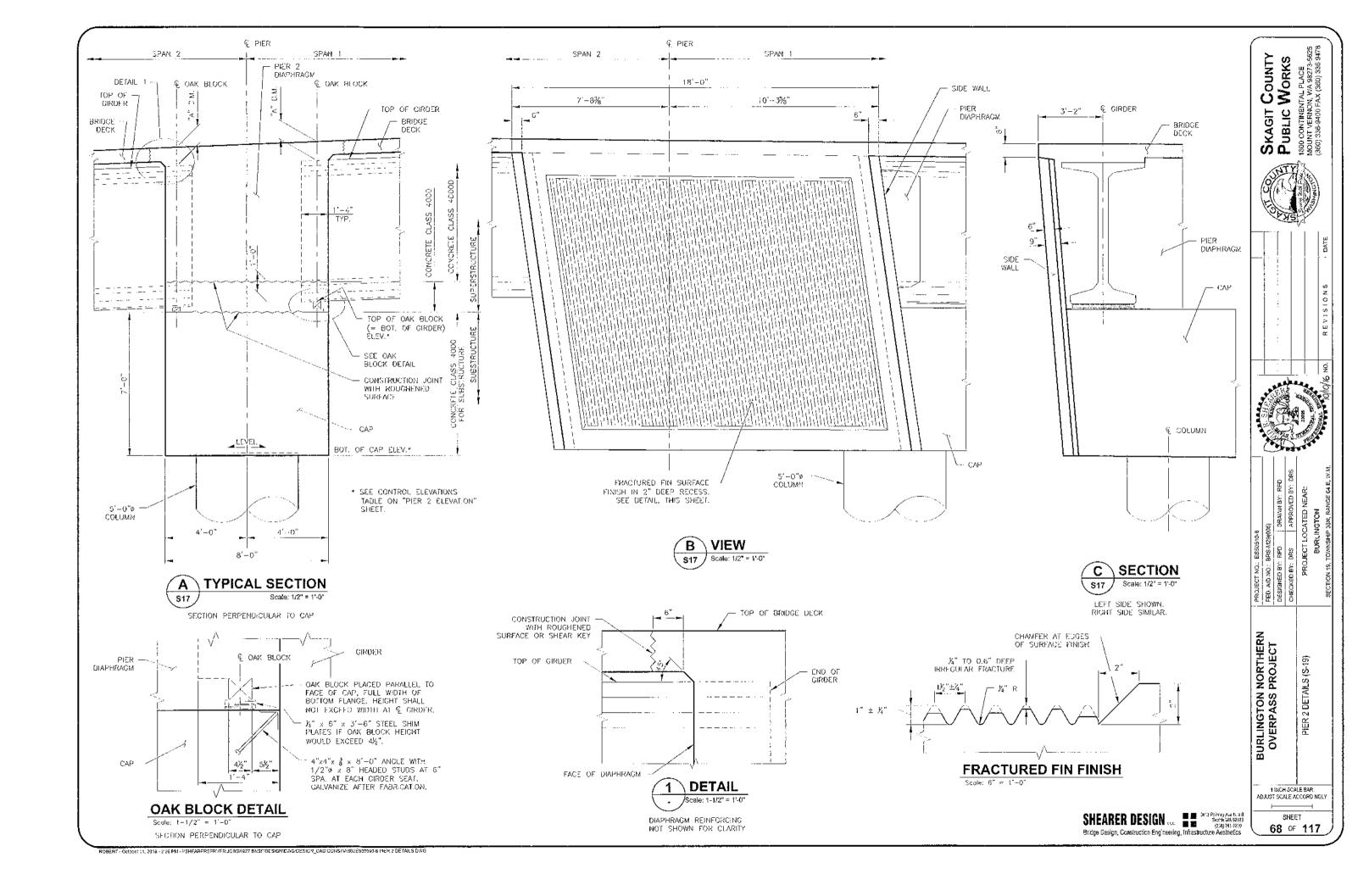


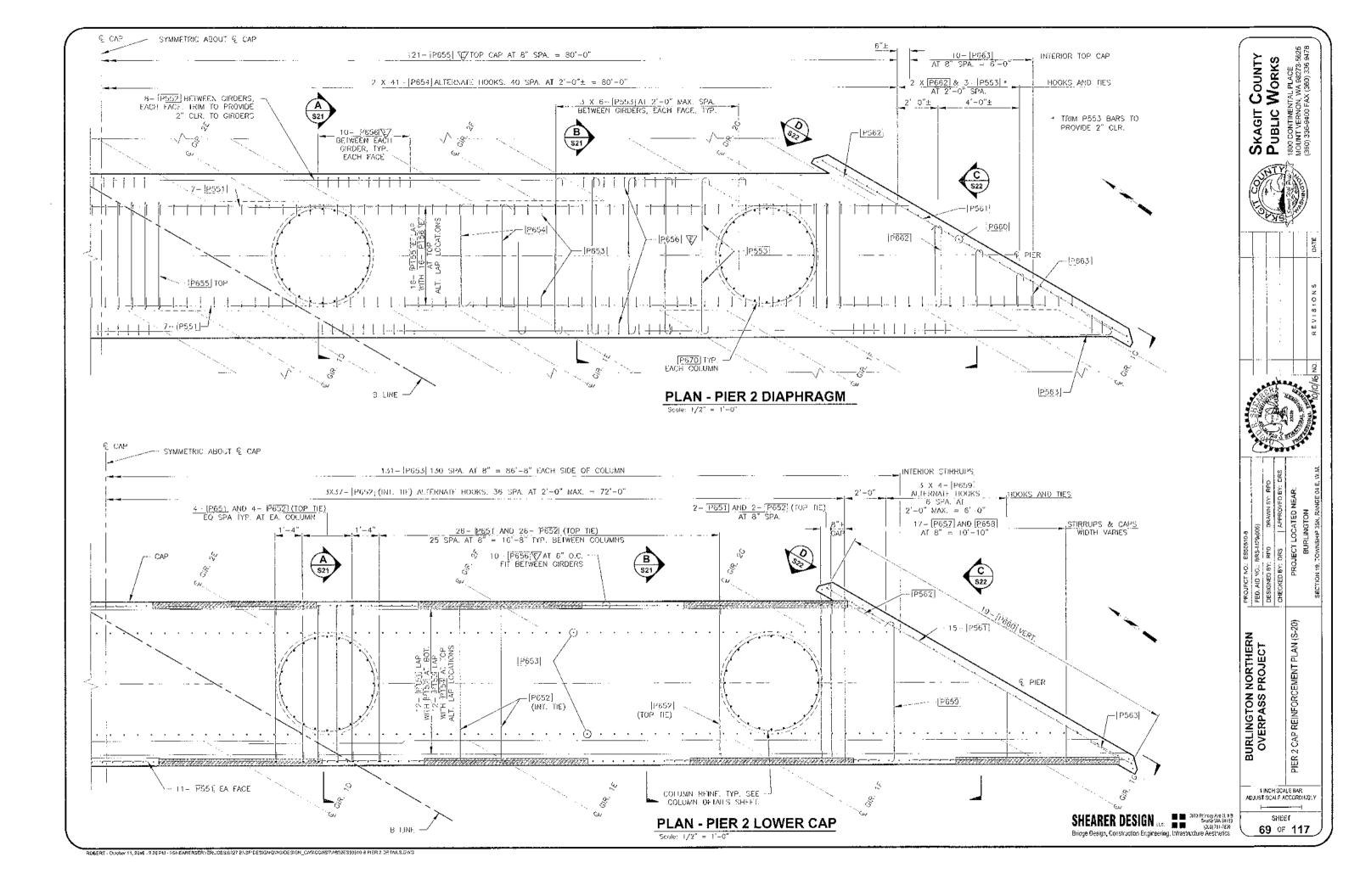


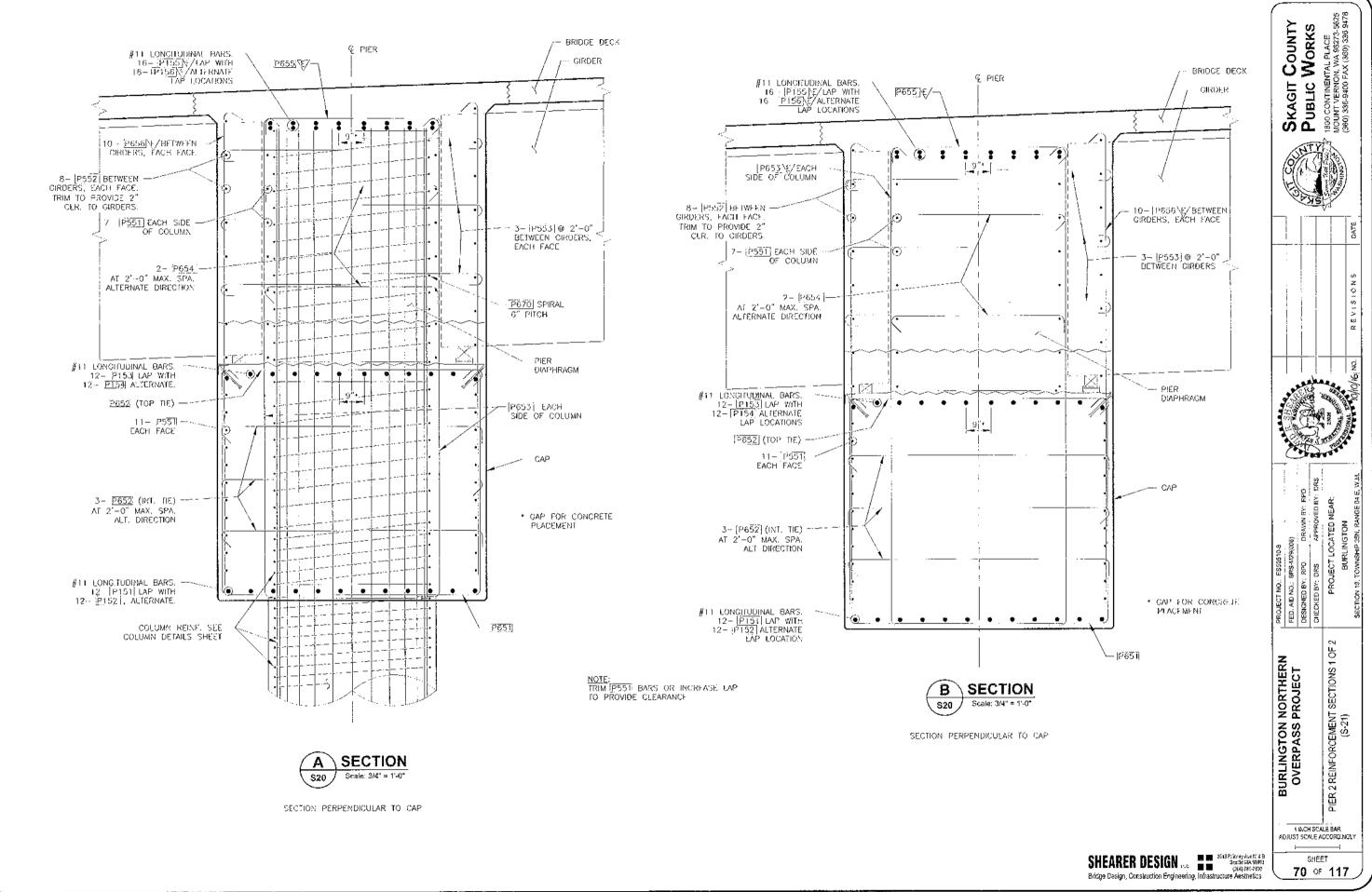


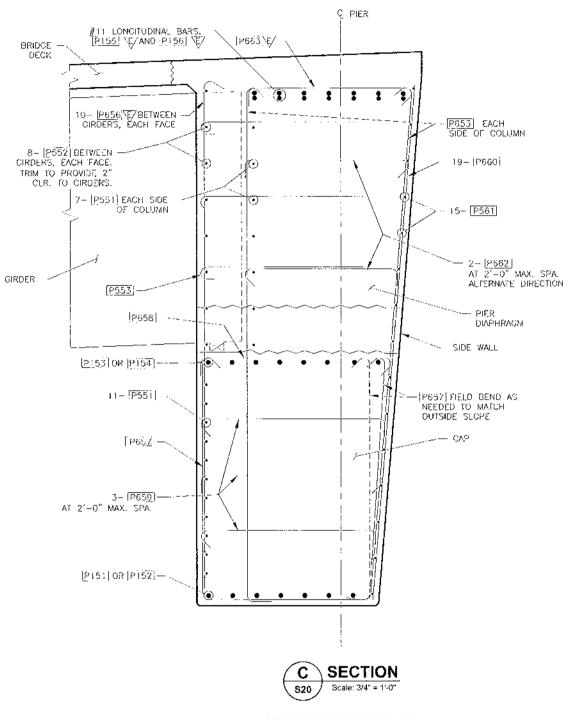




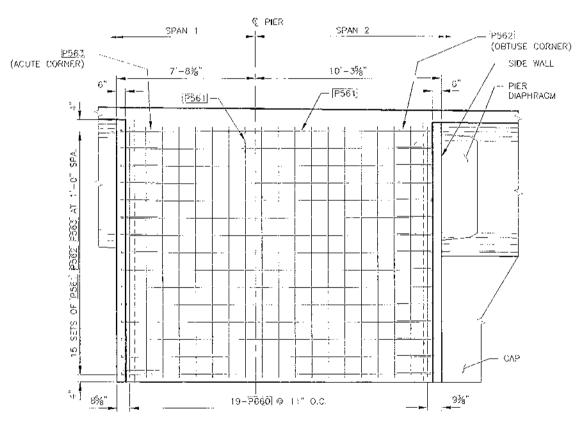








SECTION PERPENDICULAR TO CAP

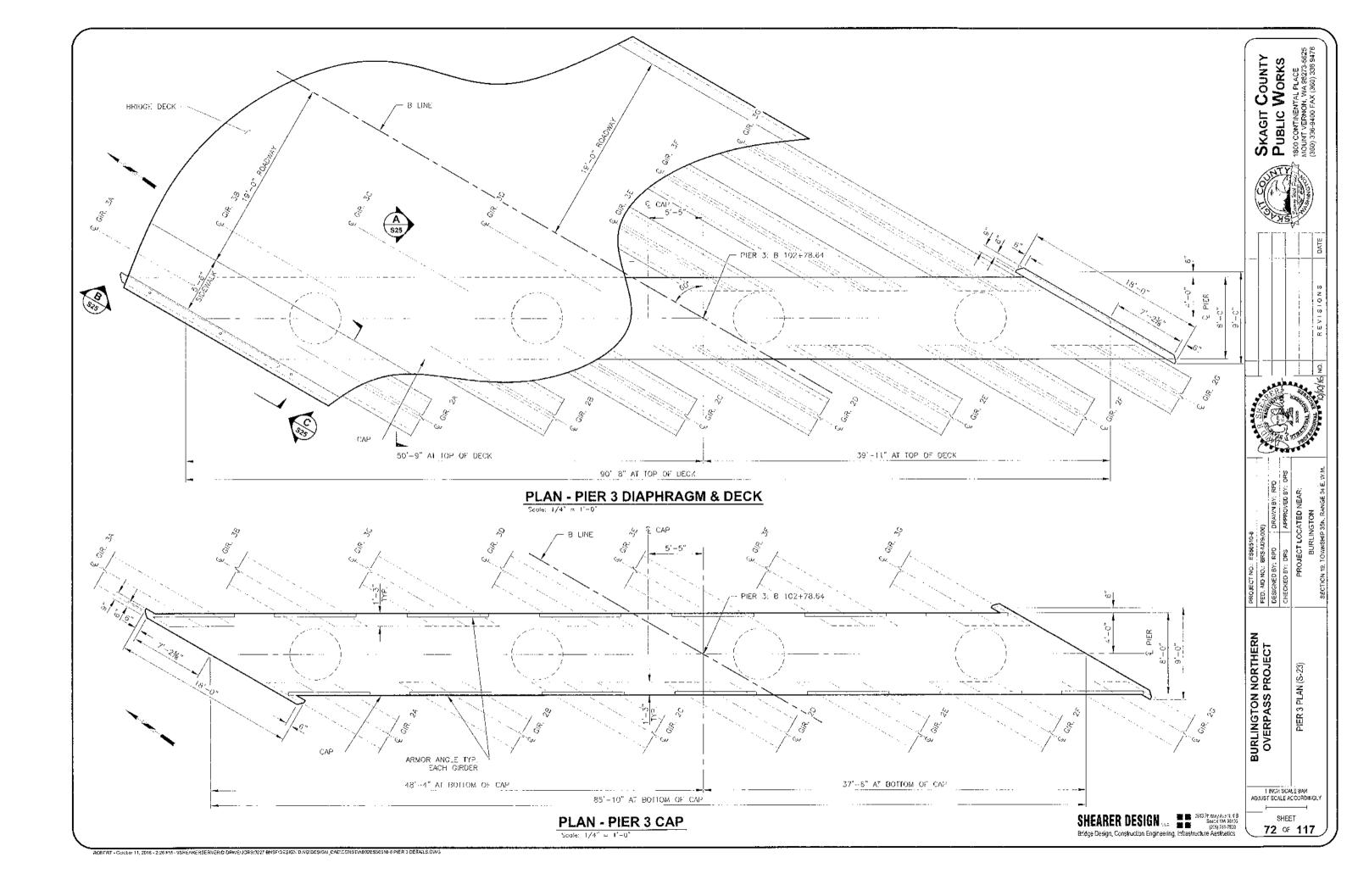


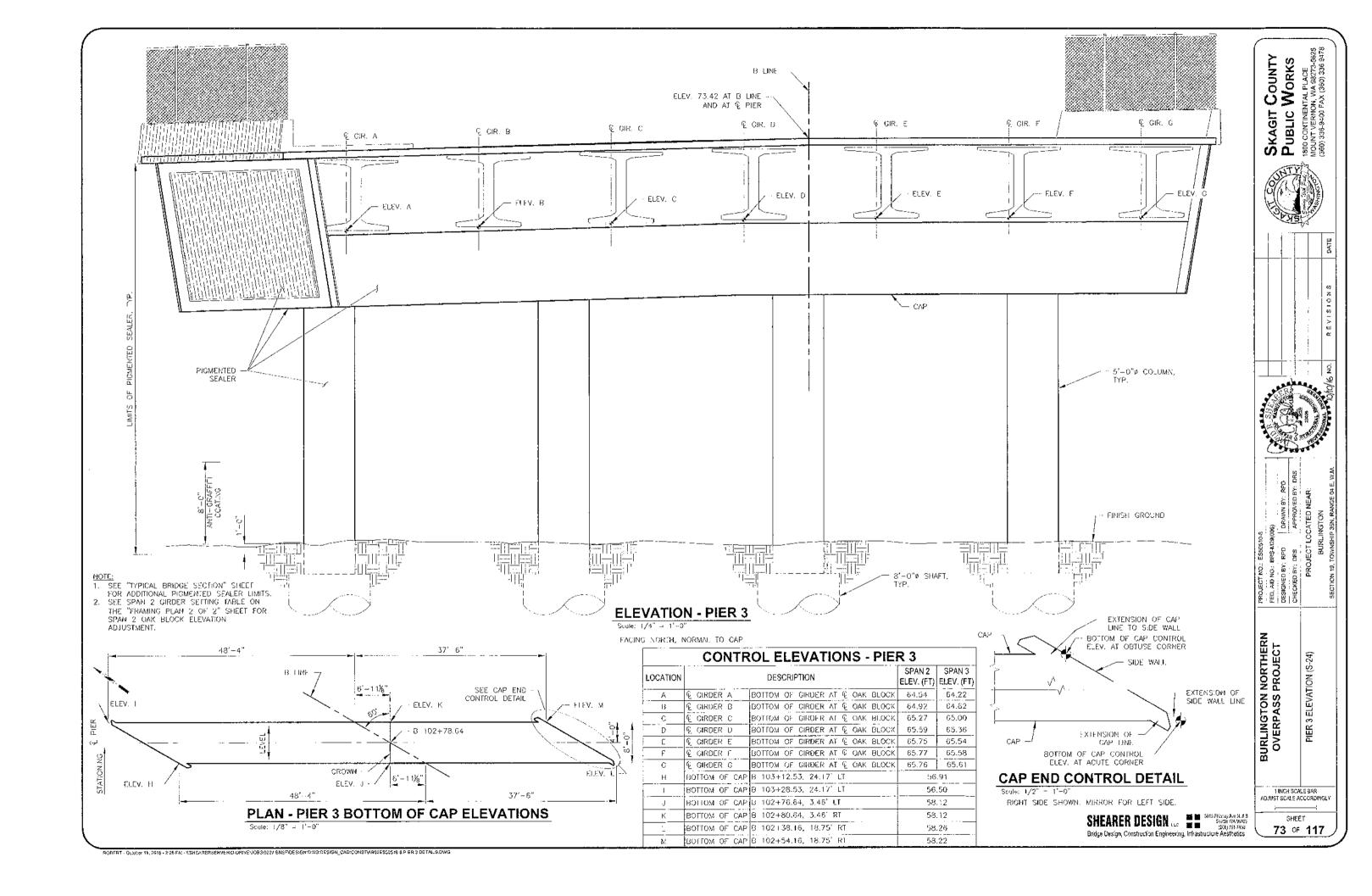
D VIEW
| S20 | Scale: 3/8" = 1'-0"

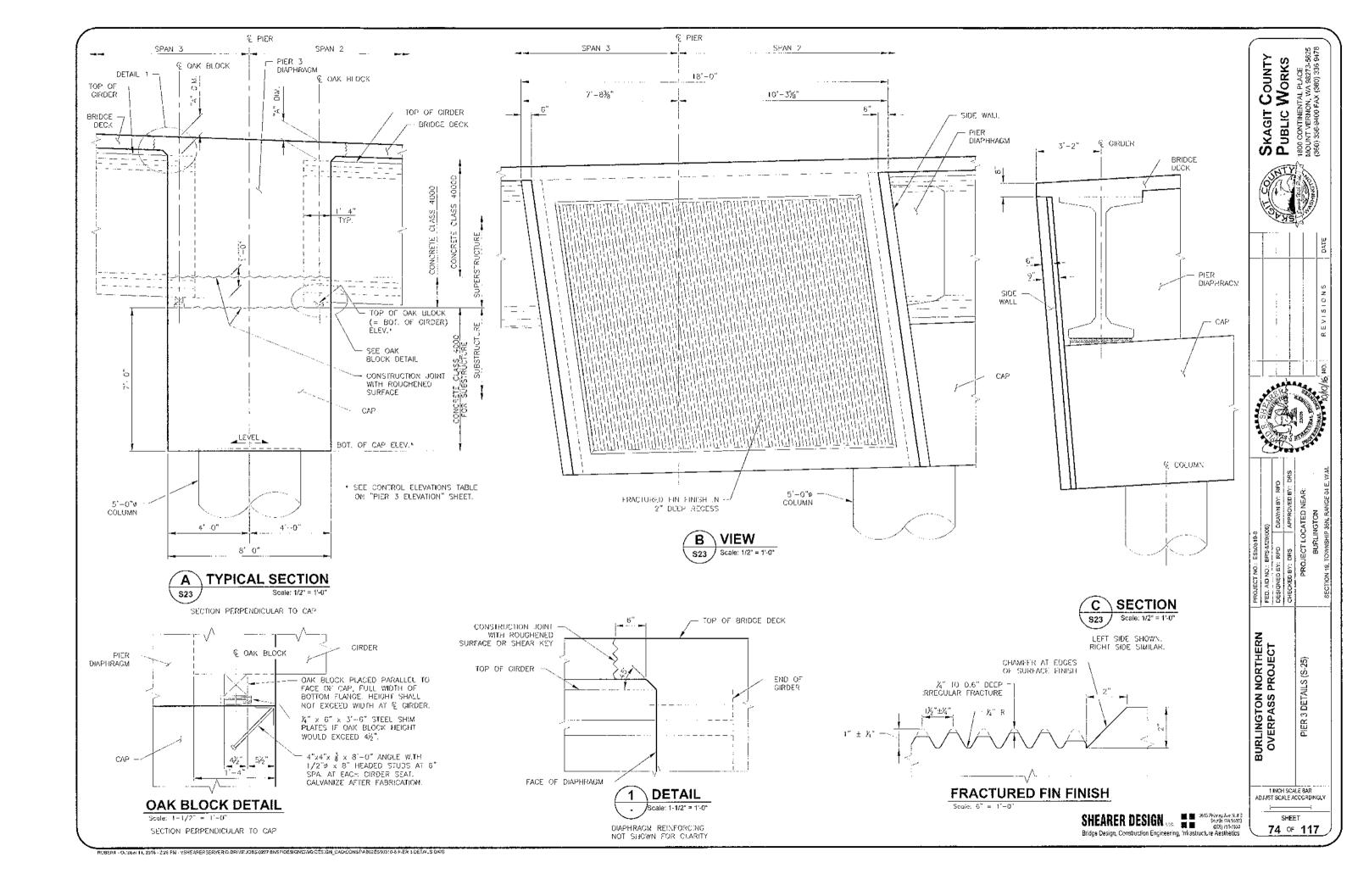
SHEARER DESIGN (1).2 Seaths (1): 88-138 (2

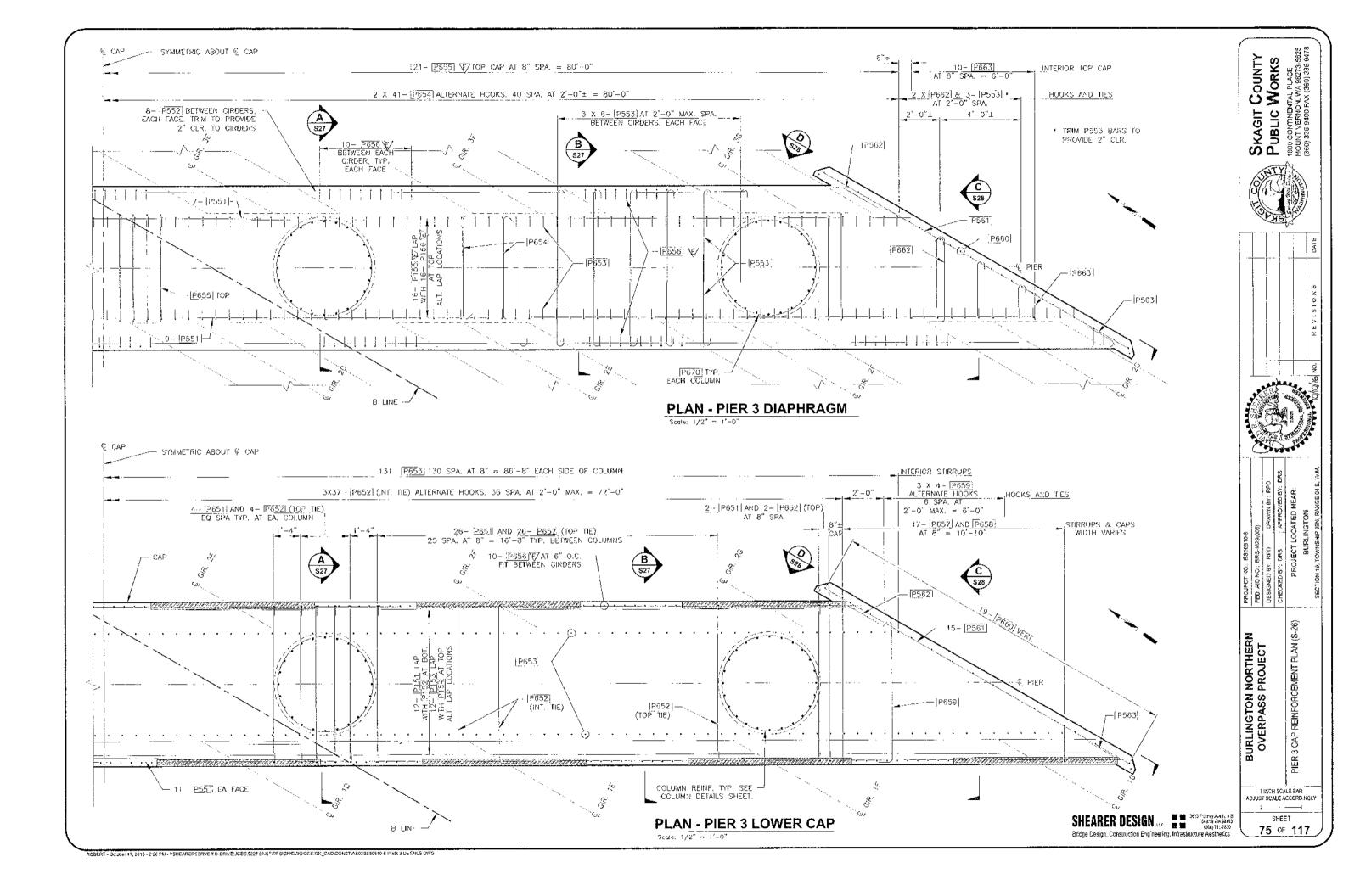
SKAGIT COUNTY
PUBLIC WORKS
1900 CONTINENTAL PLACE
MOUNT VERNON, WA 98273-5625
(350) 336-9400 FAX (360) 336 9478 PIER 2 REINFORCEMENT SECTIONS 2 OF (S-22) BURLINGTON NORTHERN OVERPASS PROJECT 1 INCH SCALE BAR ADJUST SCALE ACCORDINGLY

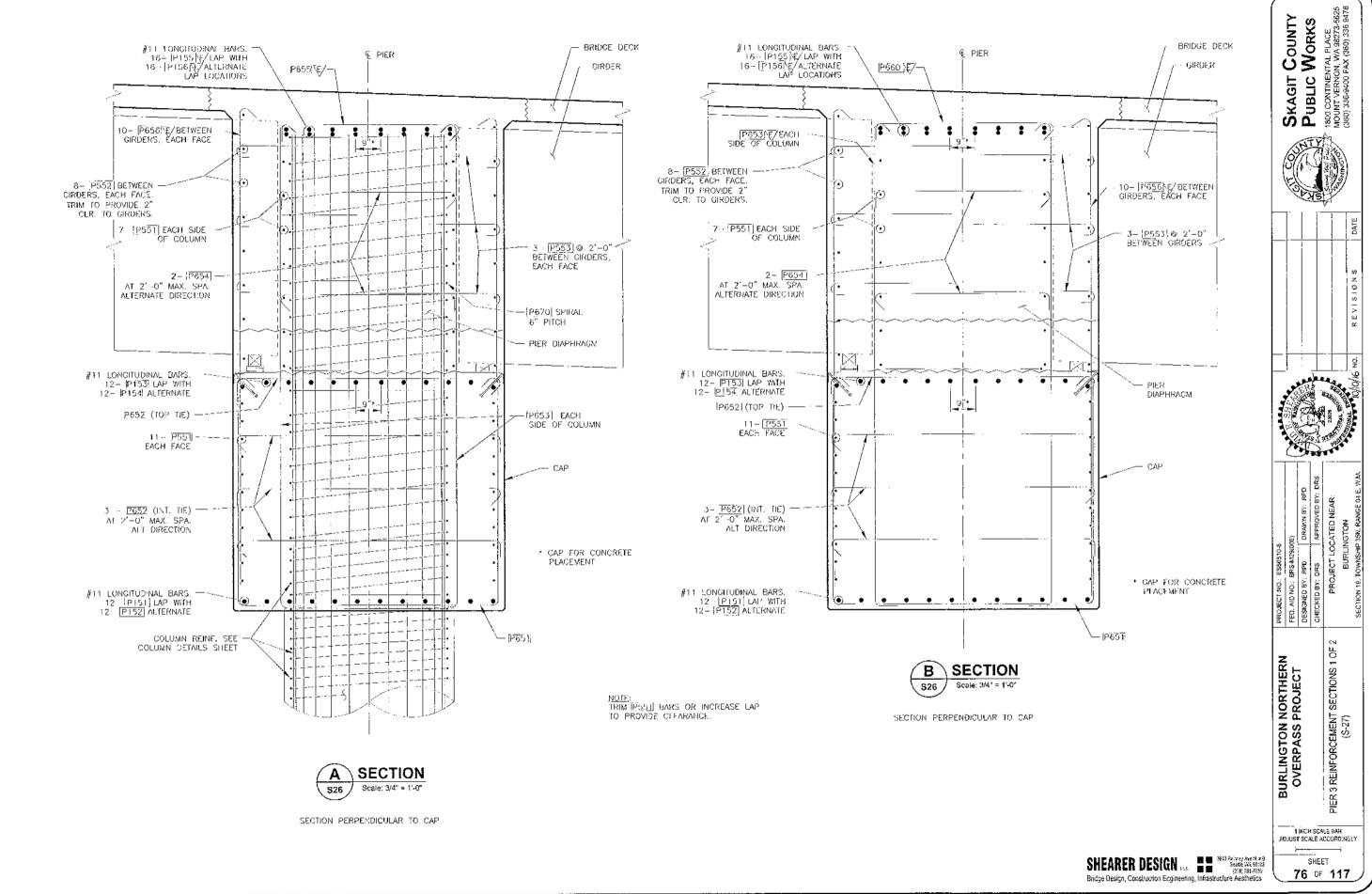
SHEET 71 OF 117

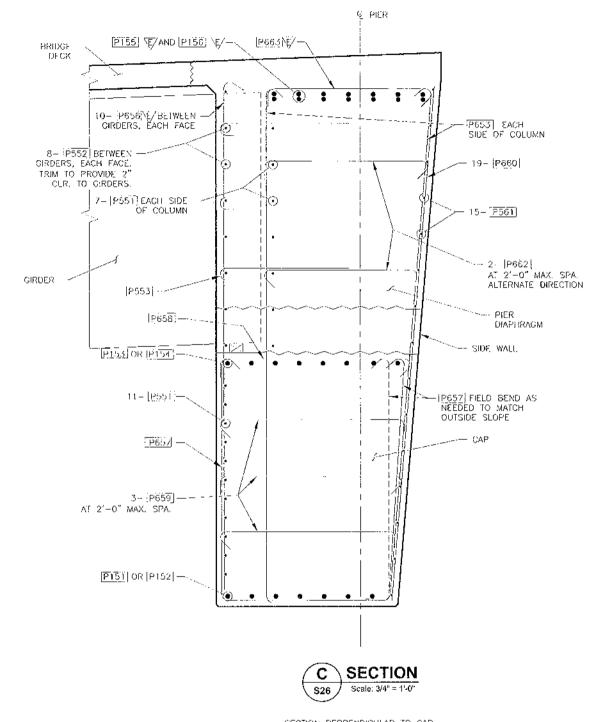




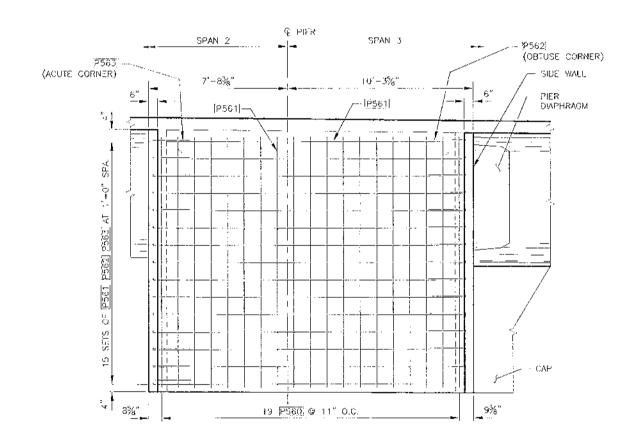








SECTION PERPENDICULAR TO CAP





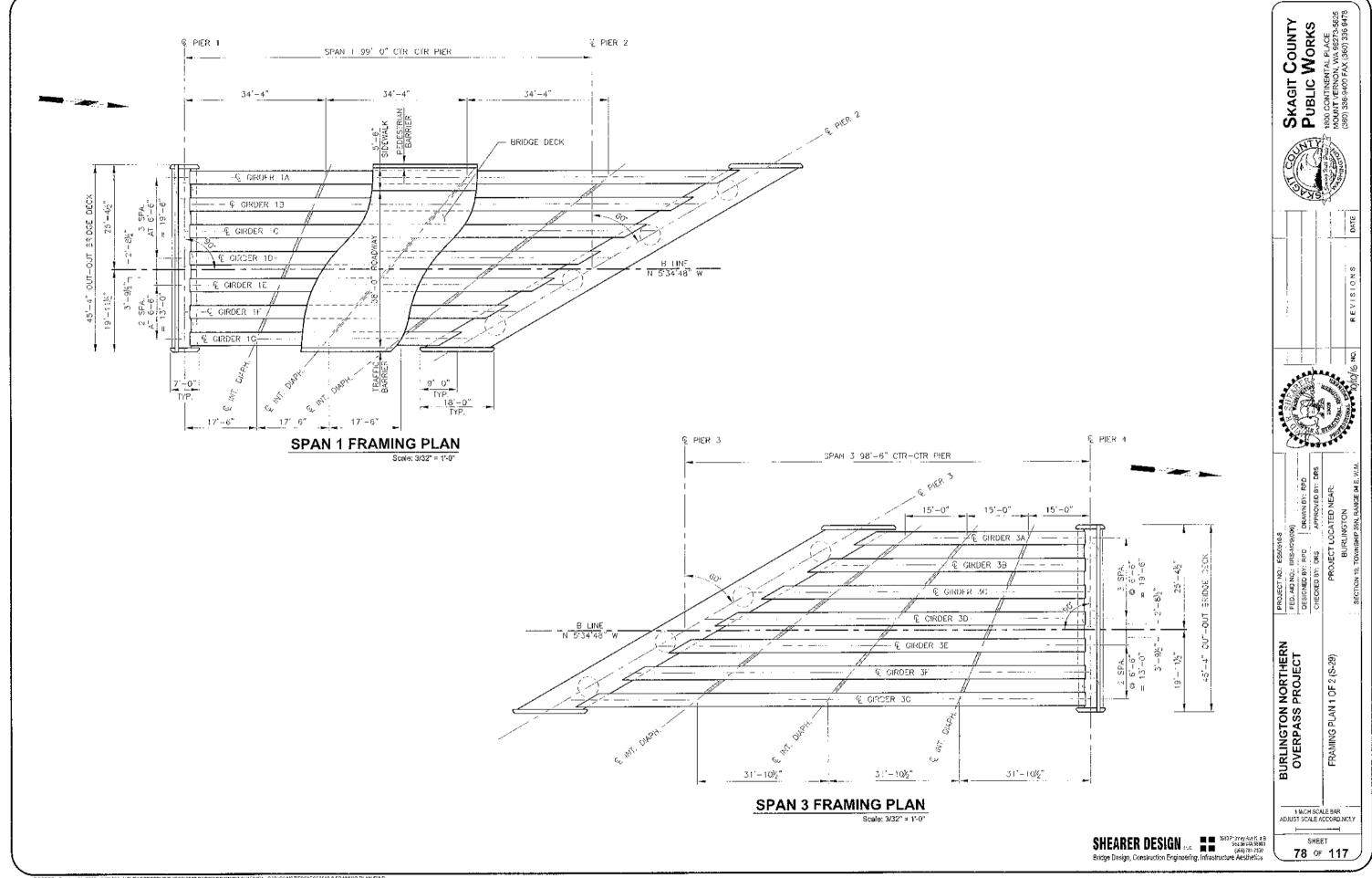
SHEARER DESIGN LLC. \$\frac{2548 \text{Points} \text{As Biggs}}{2643 \text{Points} \text{As Biggs}}\$
Sridge Design, Construction Engineering, Infrastructure Aesthetics

BURLINGTON NORTHERN OVERPASS PROJECT PIER 3 REINFORCEMENT S (S-28)

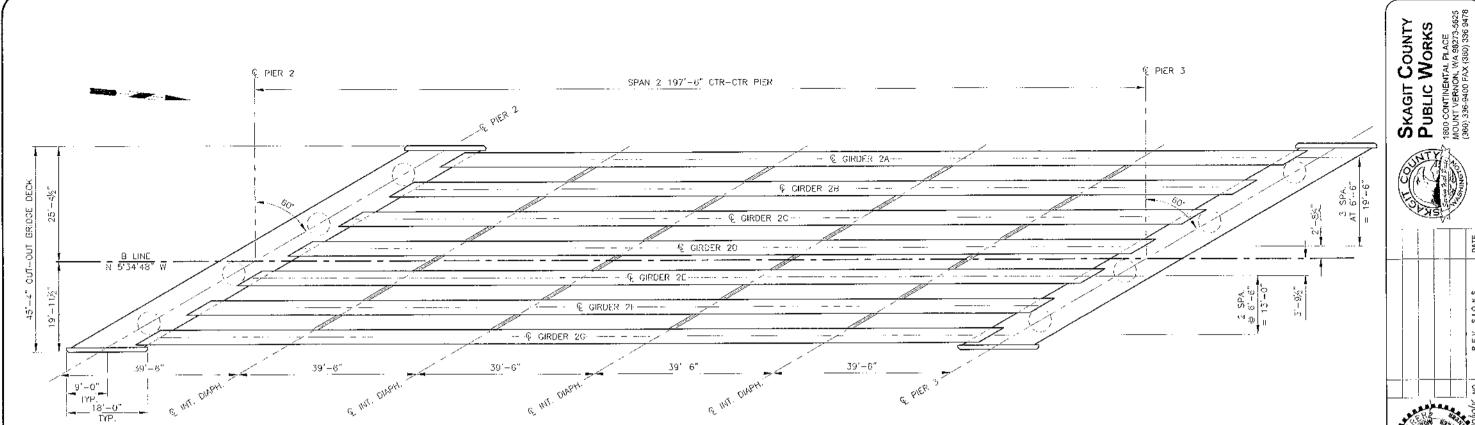
1 INCH SCALE BAR ADJUST SCALE ACCORDINGLY

SHEET 77 OF 117

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ROBERT - October 11, 2016 - 2127 PM - PSHEAR FRS FRVER QUISIO227 BN5P/DES/OND/MG/DES/ONST/FPG18540510 & FRAMING PLAN.DWG



SPAN 2 FRAMING PLAN

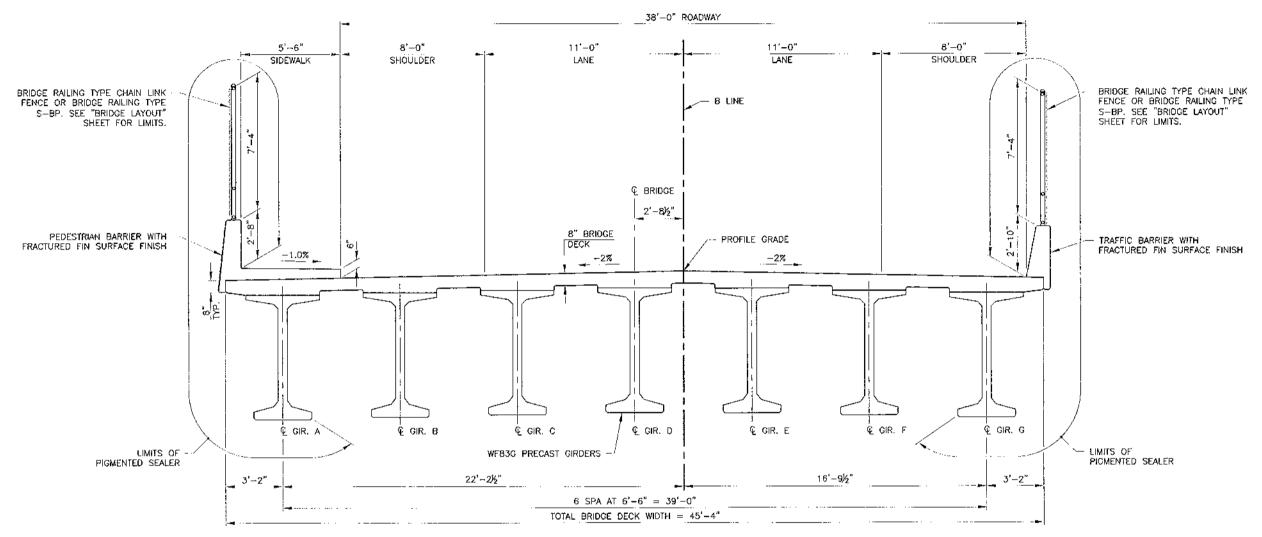
Scale: 3/32" = 1'-0"

SPAN 2 GIRDER SETTING									
GIRDER TOTAL CAMBER (IN)	GIRDER "A" DIMENSION (IN)	ADJUST OAK BLOCK ELEVATION (IN)							
17,5	9.25	1.75							
18.5	10.00	1.00							
19.5	11.00	0.00							
20.5	12.00	-1.00							
21.5	13.00	-2.00							
22.5	14.00	-3.00							
23.5	15.00	4.00							
24.5	16.00	5.00							

NOTE:
THE CONTRACTOR SHALL MEASURE THE ACTUAL TOTAL CAMBER IN THE CIRCLES AND USE THE TABLE ABOVE TO ADJUST OAK BLOCK ELEVATIONS FOR SETTING HE GIRDERS. THE CAMBER SHALL BE MEASURED NO MORE THAN 14 DAYS PRIOR TO SETTING GIRDERS IN THEIR FINAL POSITION.
INTERMEDIATE VALUES IN THE TABLE MAY BE LINEARLY INTERPOLATED.

BURLINGTON NORTHERN OVERPASS PROJECT 1 INCH SCALE BAR ADJUST SCALE ACCORDINGLY SHEET 79 of 117

SHEARER DESIGN (1.0 Secret WA) 2013 Phinory Are N = 9
Secret WA) 2013
Bridge Oesign, Construction Engineering, Infrastructure Aesthetics



TYPICAL BRIDGE SECTION

Scate: 3/8" = 1'-0"

BURLINGTON NORTHERN FED. AID NO.: ESSOG108

CHECKED BY: DRS

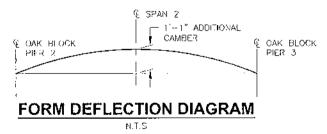
APPROVED BY

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				(IN)								CONC.	SO,	83	S		CATIO STR	N OF ANDS	STRAICI TO EX	HI SIR. KIEND			D		TAL MBER			EINFORG	CEMENT	DETAIL	3	
SPAN	CIRDER	GIRDER SFRIES	END 1 TYPE END 2 TYPE	ENSICE	 (FT-IN)	 (FT-:N)		(DEG.	92) (DEG.)	PLAN LENGTH (ALONG GIRDER GRADE) (FI-IN)	AT FNAL F'c (kSI)	Al RELEASE F'c'(ks)	NUMBER OF STRAIGHT STRANDS	NUMBER OF HARPED STRANDS	NUMBER OF TEMP, STRANDS	≝ (ŧN)	F@ (N)	Fo (III)	ENU 1	END 2	DECK SCREED CAMBER C (IN)	LOWER BOUND AT 40 DAYS	UPPER SOUND AT 120 DAYS	LOWER BOUND AT 40 DAYS	UPPER BOUND AT 120 CAYS	۷ì	V2 (.N)	V3	V4 (IN)	V5	V6 (!N)	HI1 (FT-JN)
1	Α	WF83G	c ə	11,00	3'-0"	9'-0"	3, -0,,	90	60	130'11%"	7.0	5.0	30	iO	2	2%	4	8	3706	(3) 00 (8) (9) 01 (9)	1½	15%	5%	_	77	6	3½	24	ô	5	9	8'-11/2"
1	8	WF83G	СЭ	11.00	3'-0"	: : 7'-0"	7'-0"	90	80	119'-8"	7.0	5.0	26	8	2	2¾	4	. 11	③10€	(3) TO (8)	. 1	12%	2¾		_	6	31/2	24	6	5	9	8'-11/2"
1	С	WF83G	c o	11.00	3'-0"	/'-0"	/'-0"	90	60	108'-4%"	7.0	5.0	22	7	0	2½	4	17%	3106	(3) or (3)		1	2½	_		6	3½	24	6	5	Э	8'-1½"
1	υ	WF83G	C D	11.00	3'-0"	7'-0"	7'-0"	90	60	97'-1¾"	7.0	5.0	14	7	o	2%	4	26%	3006	3006 (3006)		1 1/2	1点			6	3½	24	6	5	9	8'-1½"
1	Г.	W/783G	C D	11.00	3'0"	7'-0"	7'0"	90	60	85'-10%"	7.0	5.0	12	6	0	2%	4	30	(3) TO (6)	் (இன இனே இ		%	1/8	-	-	6	51/2	24	6	5	9	8'-1½"
1	F	wF836	C D	11.00	3'-0"	7'-0"	7'-C"	90	60	74*-7½"	7.0	5.0	10	6	0	2¾	4	36	@0T©	® எ இ இ எ இ		1/4	% 	- 		б	3½	24	6	5	9	8'-1%"
1	c	WF 8.3G	С	11,00	3'0"	7'0"	7'-0"	90	60	63'-4 ³ 6"	7.0	5.0	8	4	О	3	4	42	: : (3) to (6)	(1) or (2) (3) or (3)	:	%	*		_	6	3½	24	В.	5	9	8'-1%"
2	A-G	WF83G	D D	11.00	19'-0'	° 24'-5	"32"-0	60	60	186'-10"	9,5	7.5	46	21	6	4%	5%	13½	(3) or (3)	இஎடு இ எ இ	8%	5¼	11½	1814	24½	9	214	24	6	5	9	8' 1%"
3	А	WF830	D C	11.00	3'-0"	7'-0"	7'-0"	60	90	53'-5¾"	7.0	5.0	6	4	0	2 %	4	48	ें ③το⑥ :1§το (β)	: - (3) to (6)		1/8	k.	-	_	6	31/2	24	6	5	9	8'1½"
3	В	WF830	рс	11.00	 3'-0" 	7'-0"	7'-0"	60	90	64′8%″	7.0	5.0	8	6	0	3	4	42	(3) to (6)	3106	1/8	У,	<i>h</i>	_	_	6	31/2	24	6	5	9	8'-12"
3	c	WF830	D C	11.00	3'-0"	7'-0"	7'-0"	60	90	/6′-0"	7.0	5.0	12	4	0	2%	1	35	(3) or (3)	(3) TO(6)		1/4	3/4	-	_	6	51/2	24	6	5	9	8'-1½"
3	D	WF830	рс	11.00	3' -0"	7' -0"	7'-0"	60	90	87'-3%"	7.0	5.0	12	6	0	2%	4	30	(3) or (3)		У.	%	1/8	_	-	б	3½	24	6	5	9	8' ፣ ኢ"
3	E		:	11.00	3'-0"	1,-0,,	. 7'~0" -	60	90	98'-64"	7.0	5.0	16	8	σ	2½	4	22	(3) or (3)	113770167	%	5%	1¾			6	31/2	24	6	5	9	8'-1½"
3	F	WF83G	D C	11.00	3'-0"	7'-0"	7'-0"	60	90	109'-9¼"	7.0	5.0	18	8	0	2½	4	16	(3) 10 (6) (8) 07 (6)	இரை	5%	¾,	1%	_	-	6	3½	24	6	5	9	8'-1½"
3	G	WF83G] u c	11,00	3'0"	7'~0"	7'-0"	60	90	121' 0%"	7.0	5.0	22	10	2	2½	4	12	() 10(6) (1) 70(8)	இறை	 %	1	2½			6	3½	24	6	5	9	8, 178

- 1. CONCRETE IN SPAN 2 GIRDERS (2A-2G) SHALL BE LIGHTWEIGHT CONCRETE. ALL OTHER GIRDERS SHALL BE NORMAL WEIGHT CONCRETE. SEE GIRDER NOTE 9.
- DEFLECT GIRDER FORMS FOR SPAN 2 GIRDERS (2A-2G) TO PROVIDE ADDITIONAL CAMBER AS SHOWN IN THE FORM DEFLECTION DIAGRAM, SEE GIRDER NOTE 10.
- 3. "C" IS DECK SCREED CAMBER AT MID-SPAN (TO ACCOUNT FOR DEFLECTION DUE TO SLAB PLACEMENT AND SUPERIMPOSED DEAD LOADS). SEE SCREED SETTING DIMENSIONS DETAIL ON THE "DECK SECTIONS" SHEET.
- 4. SPAN 2 ONLY: THE "A" DIMENSION VALUES SHOWN IN THE GIRDER SCHEDULE ARE LISTED FOR REFERENCE. THE CONTRACTOR SHALL MEASURE THE ACTUAL TOTAL CAMBER AND USE THE SPAN 2 GIRDER SETTING TABLE SHOWN ON THE "FRAMING PLAN" SHEET TO ADJUST OAK BLOCK ELEVATIONS FOR SETTING THE GIRDERS. THE CAMBER SHALL BE MEASURED NO MORE THAN 14 DAYS PRIOR TO SETTING GIRDERS IN THEIR FINAL POSITION.

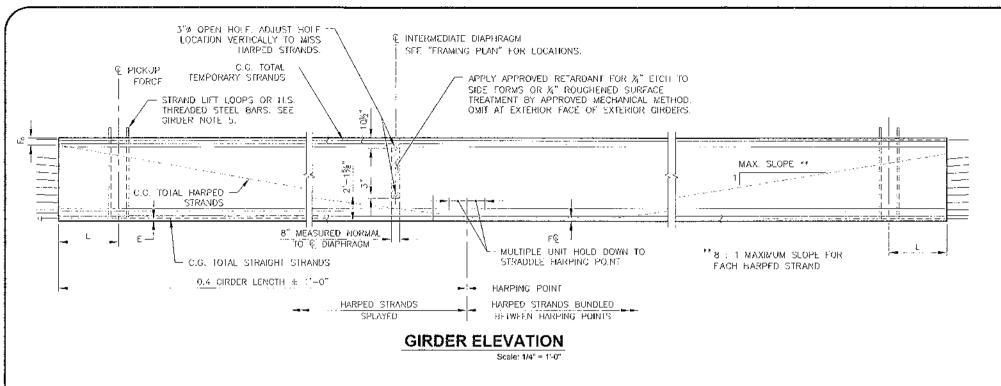
GIRDER SCHEDULE

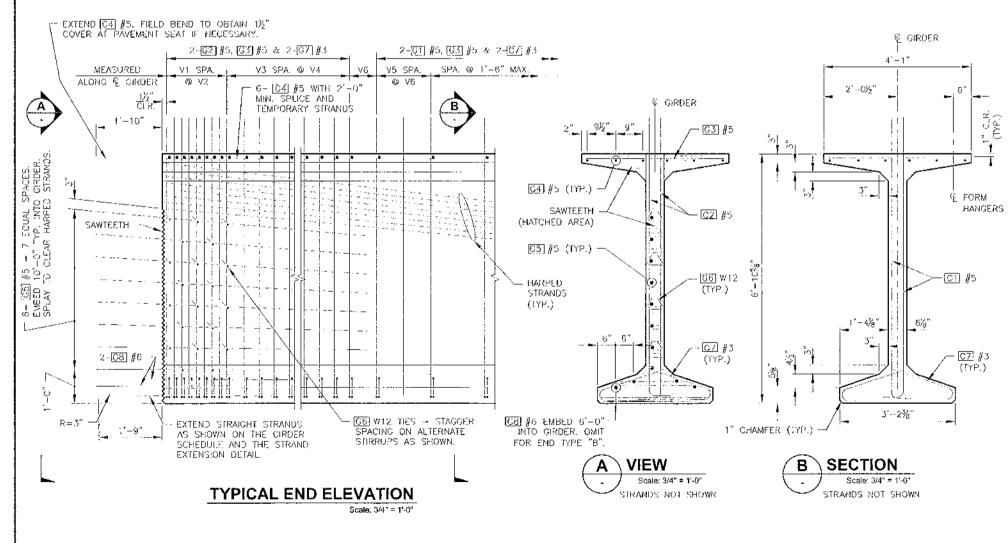


SPAN 2 ONLY

TOTAL CAMBER = ADDITIONAL CAMBER + ESTIMATED CAMBERS
GIVEN IN THE GIRDER SCHEDULE, SEE GIRDER NOTE 10.

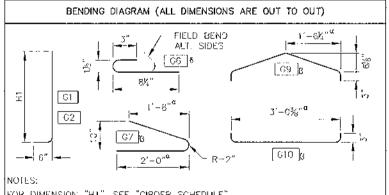
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GIRDER NOTES

- PLAN LENGTH SHALL BE INCREASED AS NECESSARY TO COMPENSATE FOR SHORTENING DUE TO PRESTRESS AND SHRINKAGE
- 2. ALL PRETENSIONED AND TEMPORARY STRANDS SHALL BE 0.60 AASHTO M203 GRADE 270 LOW RELAXATION STRANDS, JACKED TO 202.5 KSI,
- CUT ALL STRANDS FLUSH WITH THE GIRDER ENDS AND PAINT WITH AN APPROVED EPOXY RESIN, EXCEPT FOR EXTENDED STRANDS AS SHOWN.
- 4. TEXTURE TOP OF CIRDER IN ACCORDANCE WITH STANDARD SPECIFICATION
- 5. INSTALL LIFTING EMBEDMENT IN ACCORDANCE WITH STANDARD SPECIFICATION 6-02.3(25)L. REMOVE TO TOP OF GIRDER AFTER
- CAUTION SHALL BE EXERCISED IN HANDLING AND PLACING GROBERS, ALL GROBERS SHALL BE CHECKED BY THE CONTRACTOR TO ENSURE THAT THEY ARE BRACED AGEOUATELY TO PREVENT TIPPING AND TO CONTROL LATERAL BENDING DURING SHIPPING. ONCE ERECTED, ALL GIRDERS SHALL BE BRACED LATERALLY TO PREVENT TIPPING UNTIL THE DIAPHRAGMS ARE CAST AND CURED.
- 7. TEMPORARY TOP STRANDS SHALL BE EITHER PRETENSIONED OR POST-TENSIONED IN ACCORDANCE WITH SECTION 6-02.3(25)L OF THE STANDARD SPECIFICATIONS AND THE GIRDER DETAILS SHEETS. THE LETING LOCATION "L" AND CONCRETE RELEASE STRENGTH "F'G!" SHOWN IN THE GIRDER SCHEDULE ASSUME THAT THE TEMPORARY TOP STRANGS ARE PRETENSIONED. ALTERNATIVELY, POST—TENSIONED TEMPORARY TOP STRANDS MAY BE USED IF THE LETING POINTS IN THE GIRDER SCHEDULE ARE MAINTAINED AND THE STRANDS ARE STRESSED PRIOR TO LIFTING THE CIRDER FROM THE FORM.
- 8. FOR INTERMEDIATE DIAPHRAGMS, OMIT HOLES AND PLACE INSERTS ON THE INTERIOR FACE OF EXTERIOR CIRDERS. PLACE HOLES AND INSERTS. PARALLEL TO DIAPHRAGM CENTERLINE. INSERTS SHALL BE : "# MEADOWBURKE MX-3 HI-TENSILE, 1"# x 5½" WILLIAMS F22 OPEN FERRULE INSERT, 1"# x 4%" DAYTON-SUPERIOR F-62 FLARED TRIN SLAB FERRULE INSERT, OR APPROVED EQUAL
- CONCRETE FOR GIRDERS IN SPAN 2 (2A-2G) SHALL BE LIGHTWEIGHT CONCRETE. THE CIRDER DESIGN IS BASED ON AN ASSUMED UNIT WEIGHT OF LIGHTWEIGHT CONCRETE OF 0.140 KCF FOR CALCULATION OF DEAD LOADS AND 0.123 KCF FOR CALCULATION OF MODULUS OF ELASTICITY. ALL OTHER GIRDERS SHALL BE FABRICATED USING REGULAR WEIGHT CONCRETE. SEE THE CIRDER SCHEDULE FOR REGUIRED CONCRETE STRENGTHS
- DEFLECT GIRDER FORMS FOR GIRDERS IN SPAN 2 (2A-2G) TO PROVIDE ADDITIONAL CAMBER AS SHOWN IN THE FORM DEFLECTION DIAGRAM, FINAL DEFLECTED SHAPE SHALL BE PARABOLIC AND SHALL APPROXIMATE ROADWAY PROFILE. THE MAXIMUM FORM DEFLECTION VALUE IS BASED ON THE ESTIMATED CAMBERS GIVEN IN THE GIRDER SCHEDULE WITH ADDITIONAL CAMBER NEEDED TO MATCH THE ROADWAY PROFILE THE CONTRACTOR SHALL INDEPENDENTLY VERIET GIRDER CAMBERS SHOWN IN THE GIRDER SCHEDULE AND ADJUST FORM DEFLECTION VALUES AS REQUIRED AND UPON THE ENGINEER'S APPROVAL.

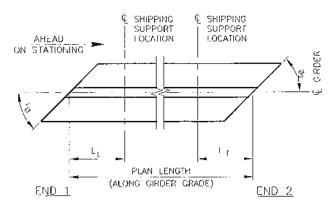


FOR DIMENSION "HI", SEE "GIRDER SCHEDULE"

- $\alpha = VARIES FOR SKEWED ENDS.$
- δ #3 OR #4 MAY BE SUBSTRUTED, FIELD BENDING IS OPTIONAL.
- B PAIRS OF G7 BARS, OR G9 AND G10 BARS, MAY BE USED INTERCHANGEABLY AS BOTTOM FLANCE HES

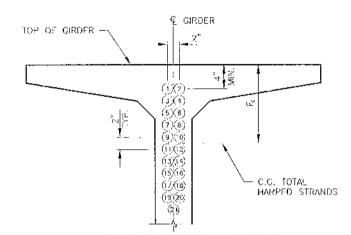
SHEARER DESIGN Loc. ### 3813 Paux et al 4 2820 (2008) P41-75-00 Bridge Design, Construction Engineering, Infrastructure Aesthetics

82 of 117



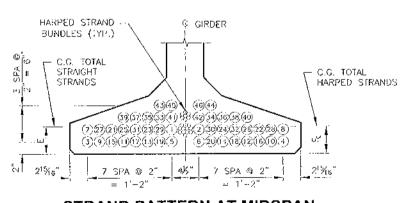
GIRDER SCHEDULE LEGEND

LEAND LEARE SHIPPING SUPPORT LOCATIONS AT LEADING AND ERAILING ENDS, RESPECTIVELY.



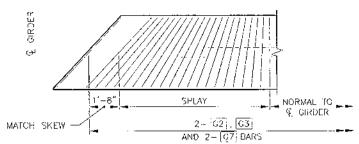
STRAND PATTERN AT END

HARPED STRAND LOCATION SEQUENCE SHALL BE AS SHOWN (1), (2) LTC.



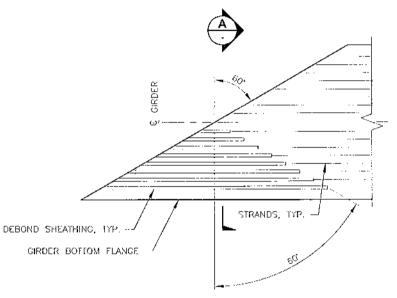
STRAND PATTERN AT MIDSPAN Scalar 1-1/2" = 12-0"

STRAIGHT STRAND LOCATION SEQUENCE SHALL BE AS SHOWN (i), (2) ETC.



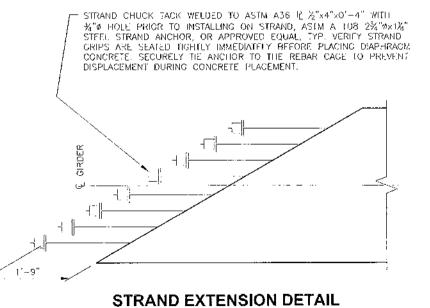
TRANSVERSE REINFORCING SKEWED ENDS

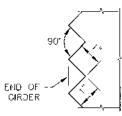
Scale: 3/8" = 1'-0"



BOTTOM FLANGE DEBONDED STRANDS

DEBOND ALL BOTTOM FLANCE STRAND ON SIDE OF GIRDER WITH ACUTE CORNER AS SHOWN, ALTERNATE CIRDER END DEBOND AND REINFORCEMENT ABOUT CIRDER & FOR OPPOSITE END.

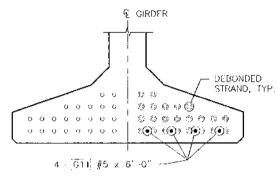




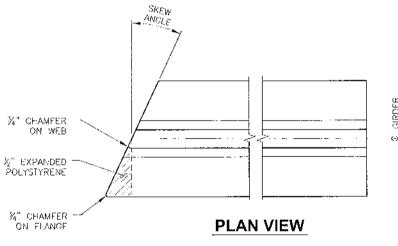
SAWTEETH ARE FULL WIDTH - USE SAWTOOTH KEYS FROM BOTTOM OF BOTTOM FLANGE TO BOTTOM OF LOWEST HARPED STRAND AS WEU. AS TOP FLANGE ADJACENT TO HARPED STRANDS.

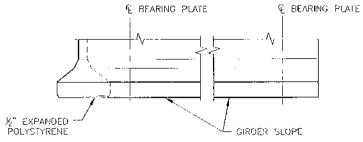
SAWTEETH DETAIL

Scale: 6" = 1'-0"









ELEVATION VIEW

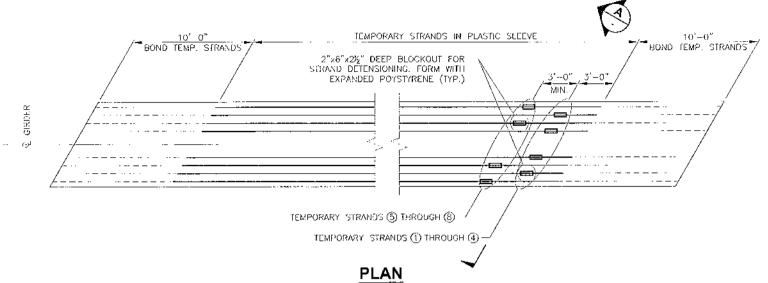
BOTTOM FLANGE SPALL PROTECTION DETAIL

Scale: 3/4" = 1'-

SHEARER DESIGN ... \$\frac{1}{2} \frac{3613 \text{Prices} \text{AwN #3 acons} \text{Awn with acons} \text{Awn acons} \text{Awn

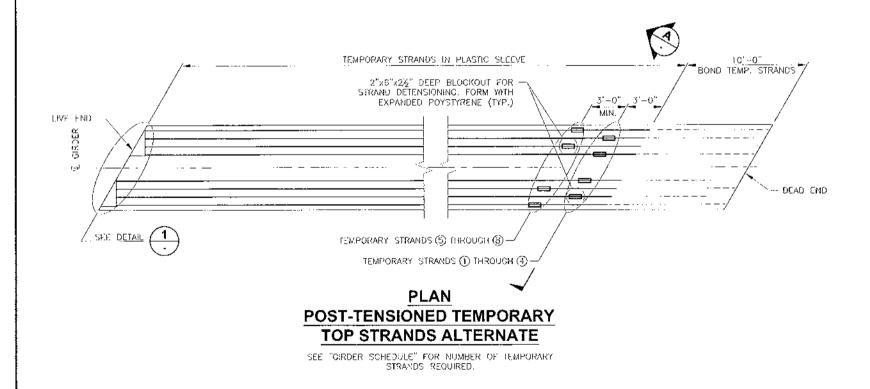
SKAGIT COUNTY
PUBLIC WORKS

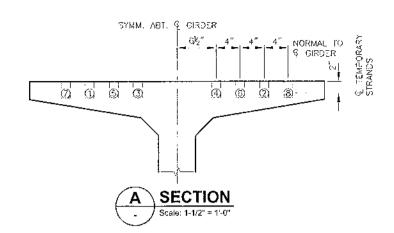
1.800 CONTINENTAL PLACE
MOUNT VERNON, WA 98273-5625
(360) 336-9400 FAX (360) 336 9478 BURLINGTON NORTHERN OVERPASS PROJECT GIRDERS 3 OF 4 (\$-34) 1 INCH SCALE BAR ADJUST SCALE ACCORDINGLY SHEET 83 OF 117



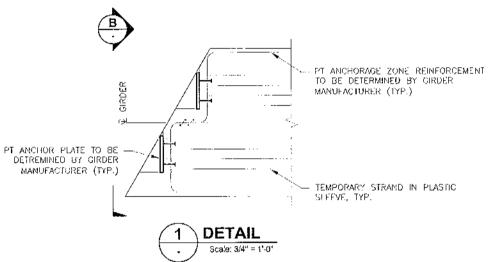
PRETENSIONED TEMPORARY TOP STRANDS ALTERNATE

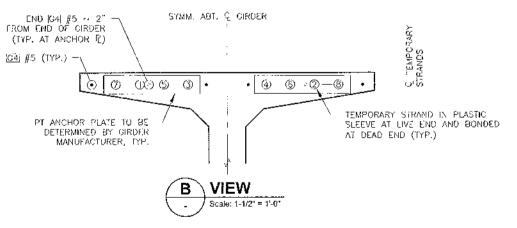
SEE "GIRDER SCHEDULE" FOR NUMBER OF TEMPORARY STRANDS REQUIRED.





NOEKS: 1. TEMPORARY STRAND LOCATION SEQUENCE SHALL BE AS SHOWN (한 선) ETC.

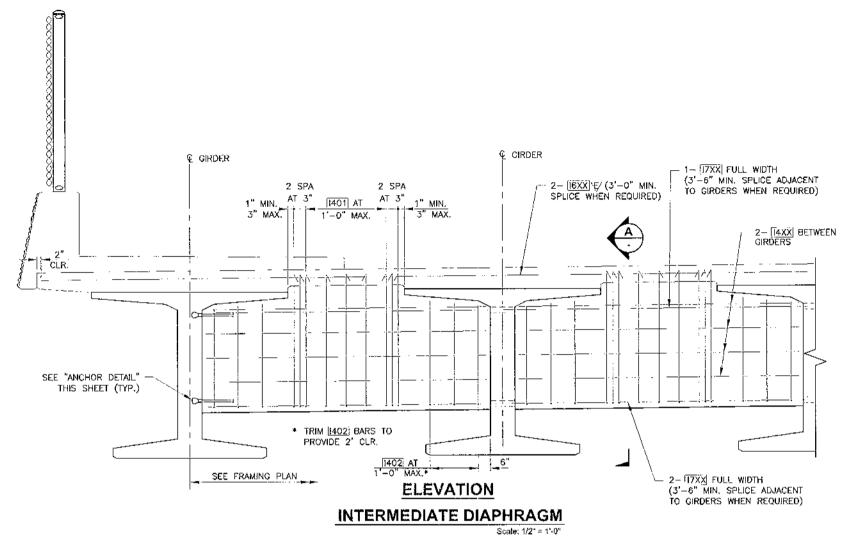




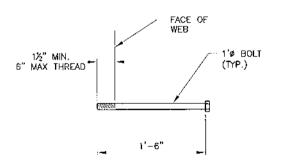
TEMPORARY STRAND LOCATION SEQUENCE SHALL BE AS SHOWN $\textcircled{\scriptsize{0}}$ $\textcircled{\scriptsize{0}}$ FIG.

SHEARER DESIGN 1.0. \$350 Philosylve N #3 Seeds (NA 50.00 (20) Bridge Design, Construction Engineering, Indrastructure Aesthetics

SKAGIT COUNTY PUBLIC WORKS 1800 CONTINENTAL PLACE MOUNT VERNON, WA 86273-5625 (360) 336-9400 FAX (360) 336 9478 FED, AID NO.: BY
DESIGNED BY: CHICKED 6Y: D BURLINGTON NORTHERN OVERPASS PROJECT WF83G GIRDERS 4 OF 4 (S-35) TINCH SCALE BASS ADJUST SCALE ACCORDINGLY SHEET 84 OF 117



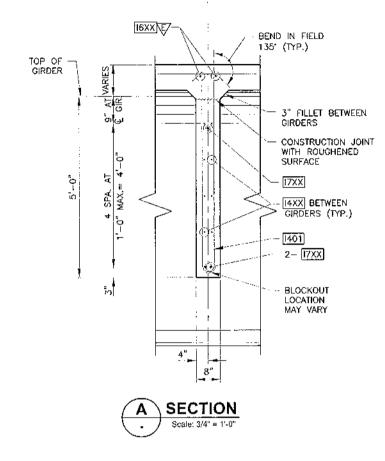
SPAN 2 SHOWN NORMAL TO DIAPHRAGM, OTHER LOCATIONS SIMILAR



ANCHOR DETAIL

ASTM A307

€ DIAPHRAGM



NOTES

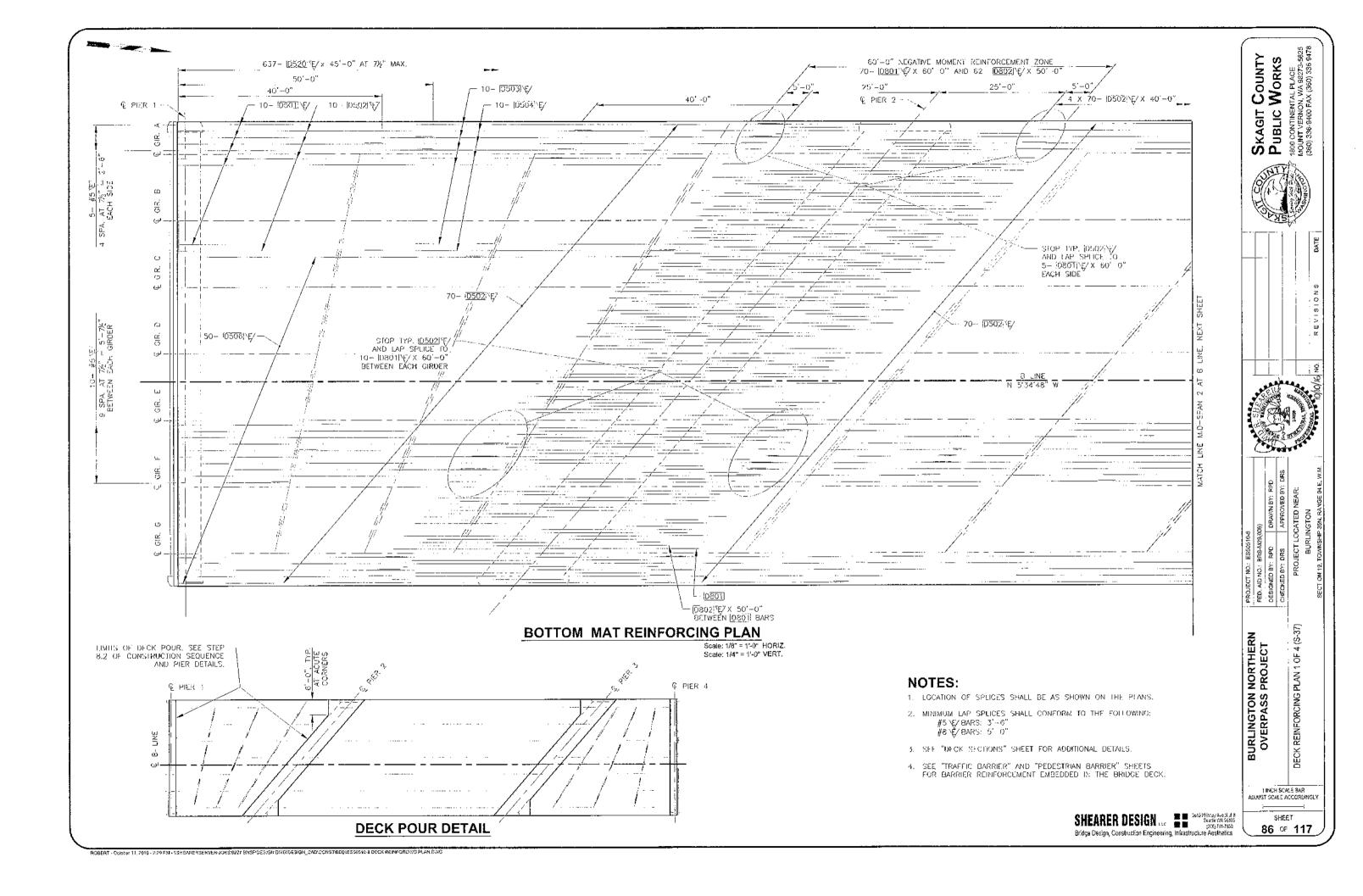
- 1. GIRDERS SHALL BE HELD RIGIDLY IN PLACE WHEN DIAPHRAGMS ARE PLACE.
- 2. IT MAY BE NECESSARY TO THREAD #7 REINFORCING BARS THROUGH HOLES IN CIRDERS PRIOR TO PLACING EXTERIOR GIRDERS.
- 3. CUT/RELEASE GIRDER TEMPORARY STRANDS BEFORE CASTING DIAPHRAGM, SEE TEMPORARY STRAND CUTTING SEQUENCE.
- 4. LONGITUDINAL DIMENSIONS ARE PARALLEL TO LINE OF INTERMEDIATE DIAPHRAGM. SEE "FRAMING PLAN" SHEET.
- 5. FOR CONCRETE PLACEMENT PROCEDURE SEE "SUPERSTRUCTURE CONSTRUCTION
- REINFORCING BAR MARK ENDING IN "XX" (14XX, 16XX AND 17XX) VARY BY LOCATION OF INTERMEDIATE DIAPHRAGM AS FOLLOWS. DIAPHRAGMS ARE NUMBERED (1ST, 2ND, 3RD, ETC.) INCREASING ALONG STATIONING. SEE "BARLISI" SHEETS FOR MORE DETAILS.
 - ARIST SHEELS FOR MORE DETAILS.

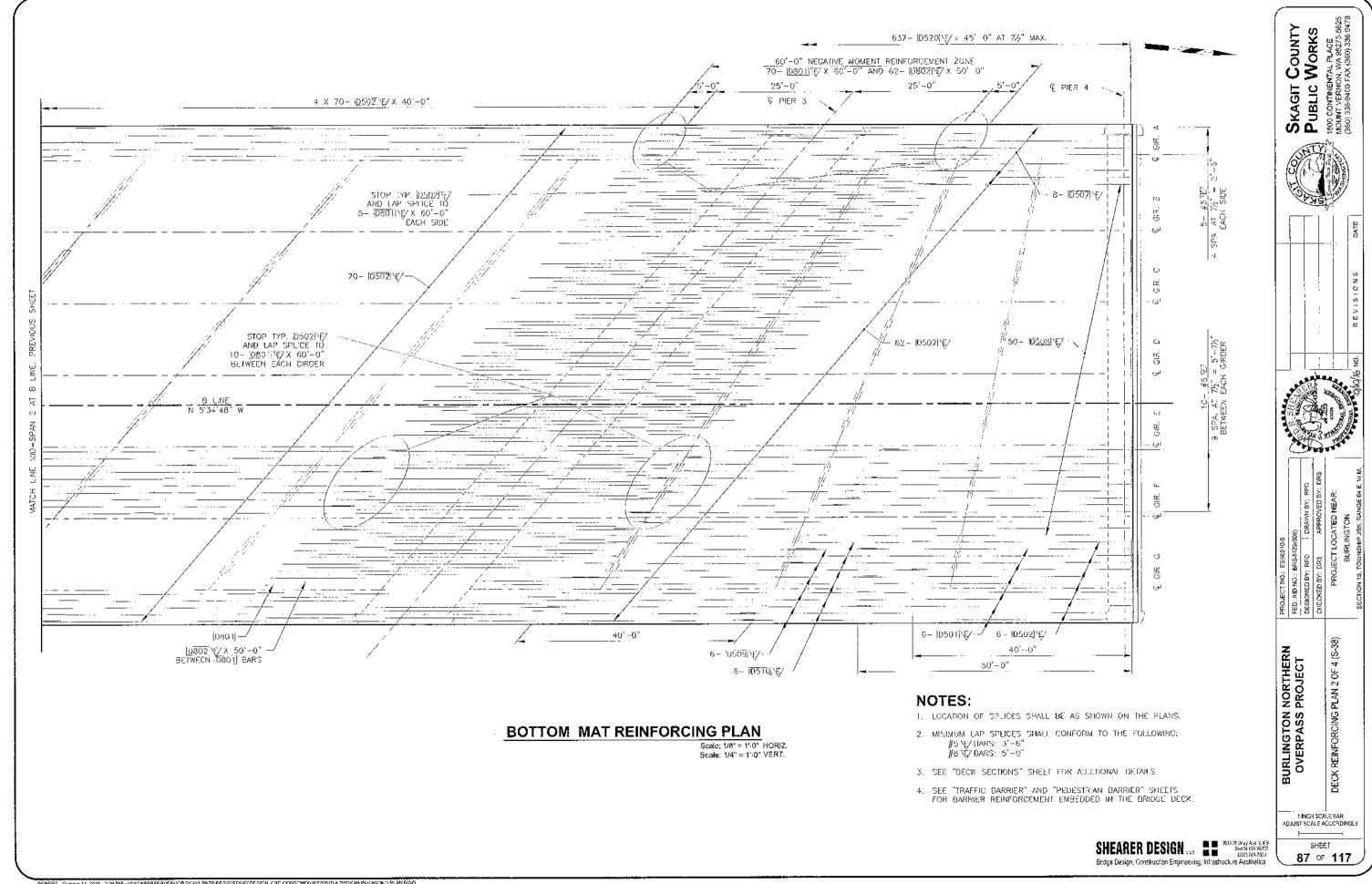
 11 = SPAN 1 1ST DIAPHRACM AND SPAN 3 3RD DIAPHRAGM
 12 = SPAN 1 2ND DIAPHRACM AND SPAN 3 2ND DIAPHRAGM
 13 = SPAN 1 3RD DIAPHRAGM AND SPAN 3 1ST DIAPRAGM
 21 = SPAN 2, ALL DIAPHRAGMS

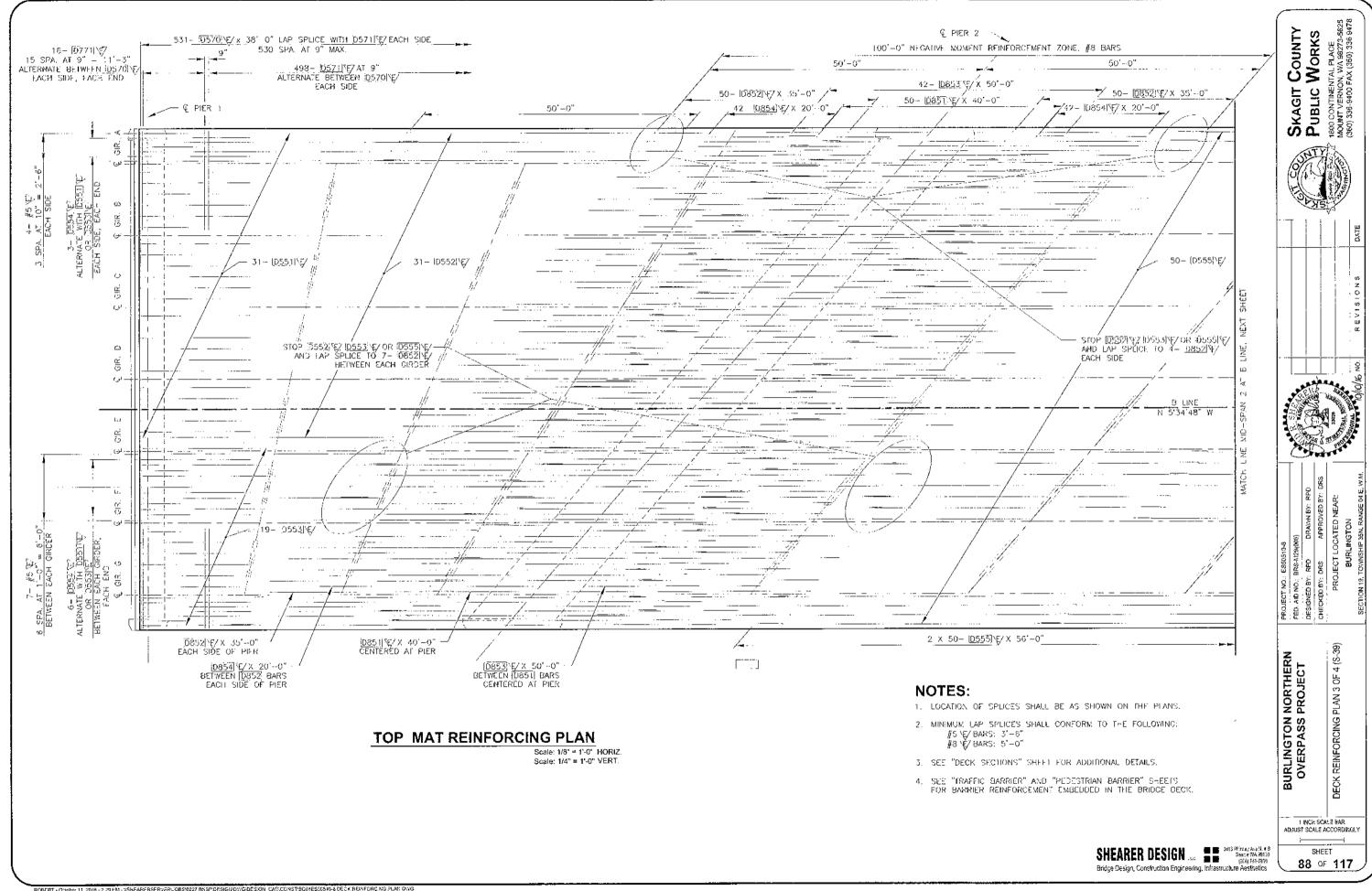
SKAGIT COUNTY PUBLIC WORKS 1800 CONTINENTAL PLACE MOUNT VERNON, WA 98273-5625 (360) 336-9400 FAX (360) 336-9478 INTERMEDIATE DIAPHRAGM DETAILS (S-36) BURLINGTON NORTHERN OVERPASS PROJECT 1 INCH SCALE BAR ADJUST SCALE ACCORDINGLY

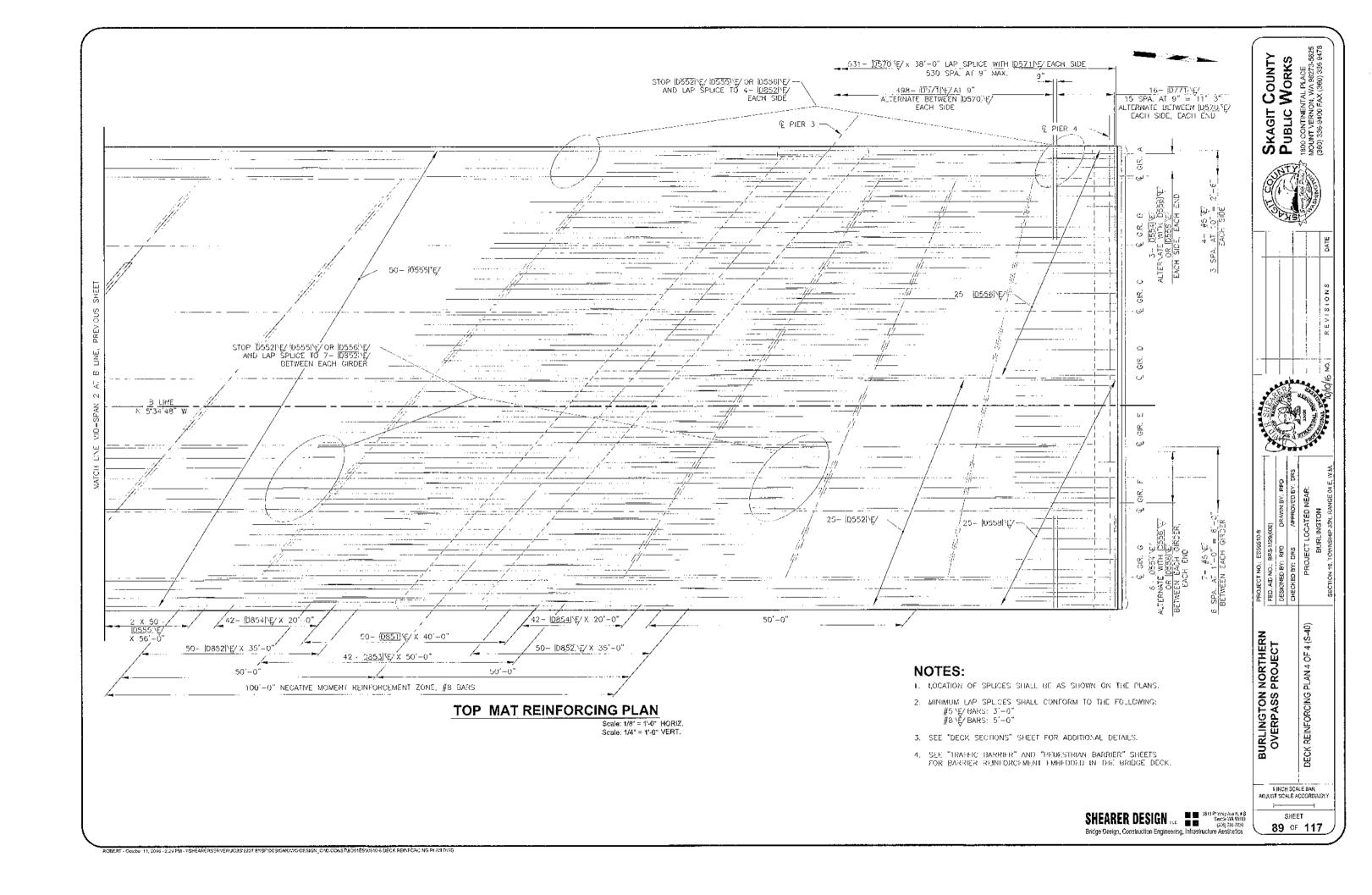
> SHEET 85 OF 117

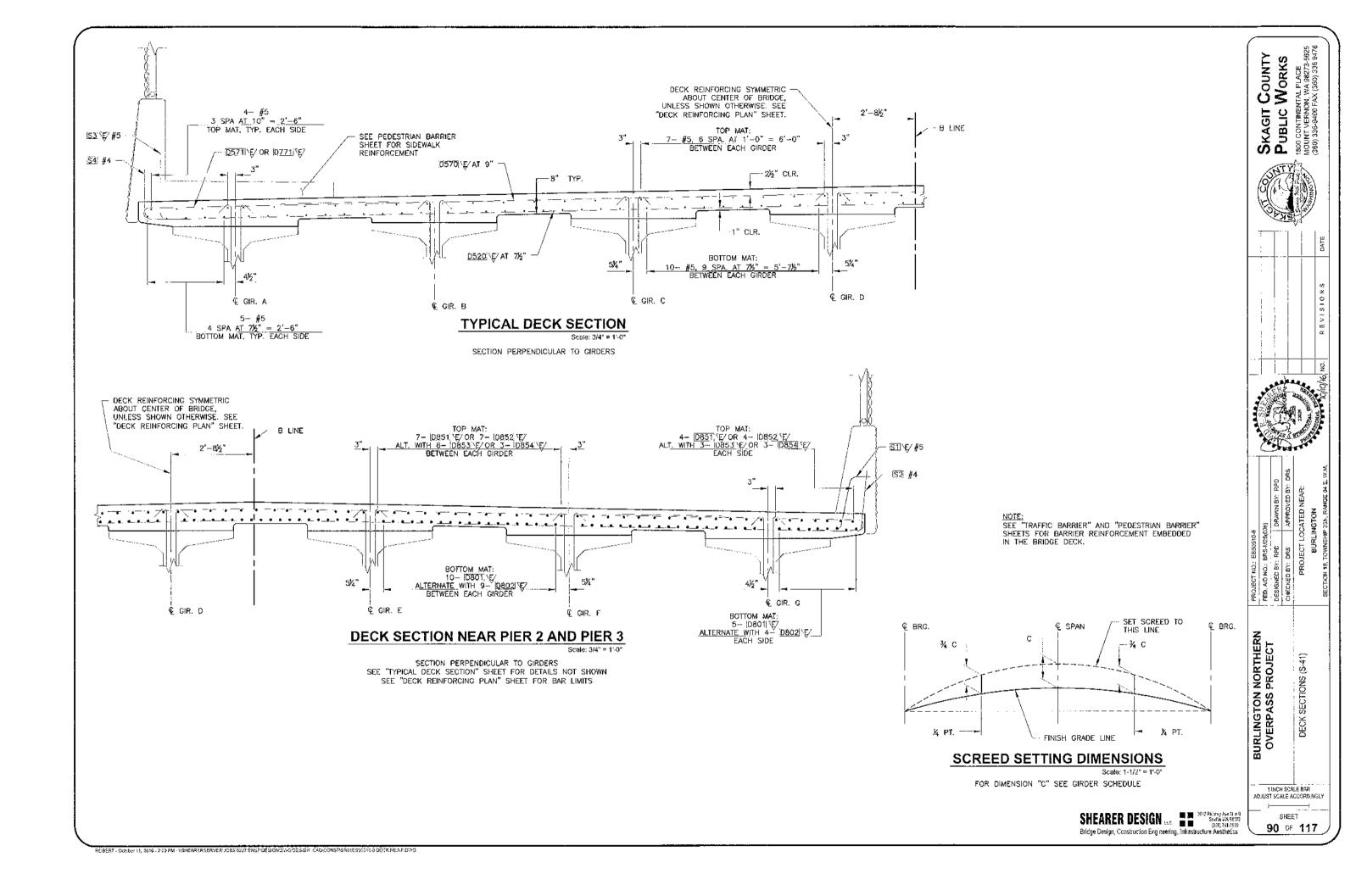
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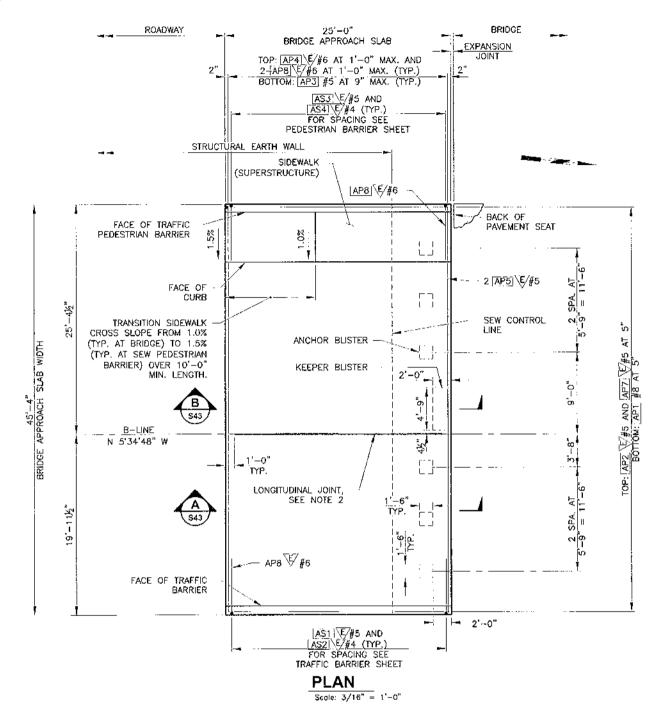








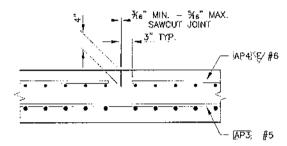




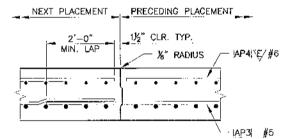
BRIDGE APPROACH SLAB AT PIER 1 SHOWN, PIER 4 SIMILAR

NOTES:

- 1. ALL EDGES OF BRIDGE APROACH SLAB SHALL HAVE 1/2" RADIUS EXCEPT AT LONGITUDINAL JOINTS AND ADJACENT TO
- 2. LONGITUDINAL JOINTS SHALL BE PLACED ON LANE LINES AND SHALL BE CONSTRUCTED AND SEALED IN ACCORDANCE WITH WSDOT STANDARD SPECIFICATION SECTION 5-05.3(8). JOINTS MAY BE EITHER A SAWCUT CRACK CONTROL JOINT OR A CONSTRUCTION JOINT, SAWCUT JOINTS SHALL TERMINATE 1'-0" BEFORE REACHING EDGE OF SLAB AND MUST BE SAWCUT AS SOON AS POSSIBLE AFTER PLACEMENT OF CONCRETE. SEE LONGITUDINAL JOINT DETAIL ON THIS SHEET.
- 3. THE MINIMUM LAP SPLICE OF #5 IS 2'-0", EPOXY COATED #5 IS 2'-6", EPOXY COATED #6 IS 3'-0", AND #8 IS 3'-3".
 ALL LAP SPLICES SHALL BE STAGGERED SO THAT NO MORE THAN 50% OF REBAR IS SPLICED AT THE SAME LOCATION. LAP SPLICES SHALL BE LOCATED WITHIN THE MIDDLE HALF OF THE BRIDGE APPROACH SLAB. OPTIONAL SPLICES ARE ALLOWED
- 4. SEE "TRAFFIC BARRIER" AND "PEDESTRIAN BARRIER" SHEETS FOR BARRIER INFORMATION NOT SHOWN.

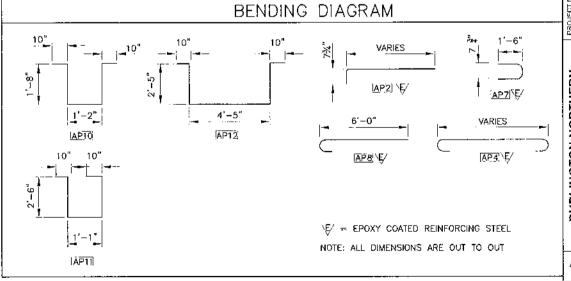


LONGITUDINAL JOINT DETAIL Scale: 1'' = 1'-0"



ALTERNATE LONGITUDINAL JOINT DETAIL

EDGE PRECEDING PLACEMENT ONLY WITH X" RADIUS.



SKAGIT COUNTY
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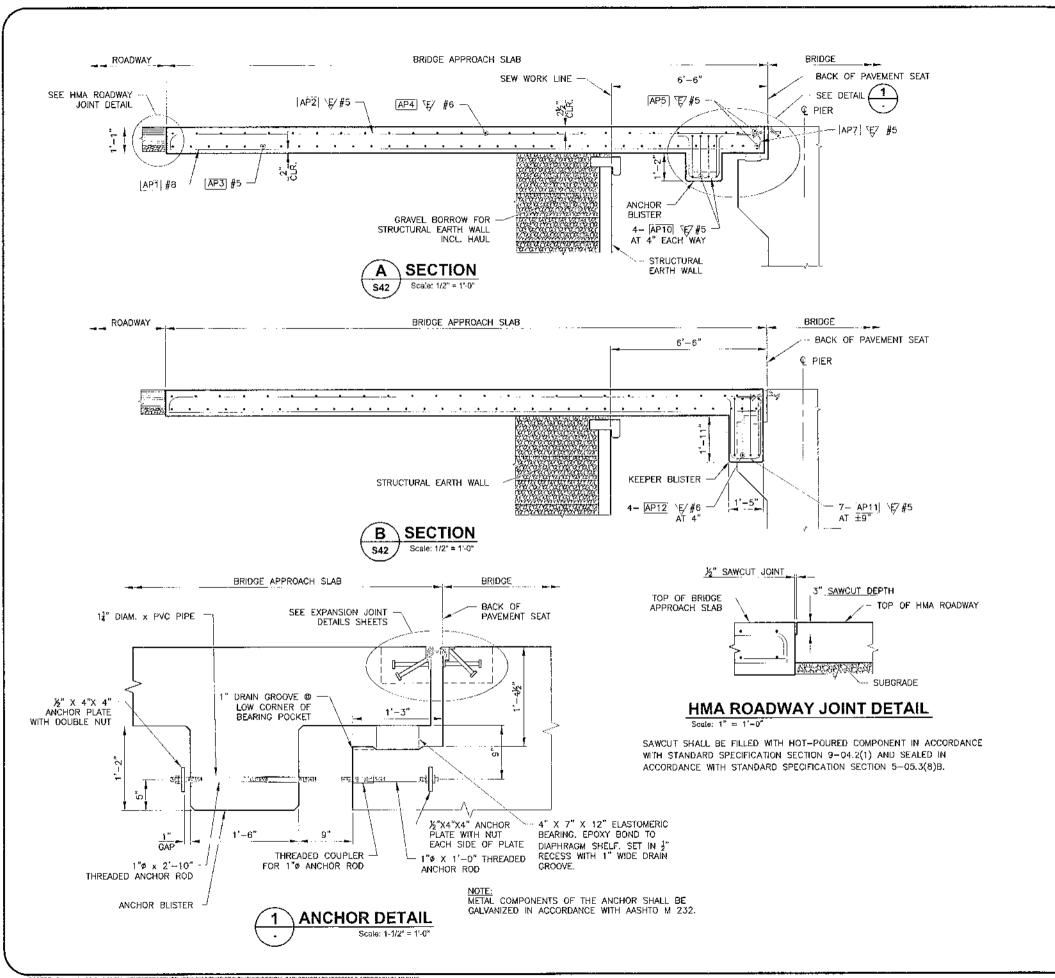
1800 CONTINENTAL PLACE
MOUNT VERNON, WA 96273-5625
(360) 336-9400 FAX (360) 336-9478

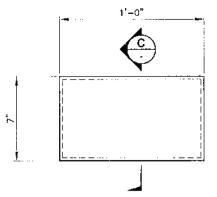
NORTHERN PROJECT

BRIDGE APROACH SLAB 1 OF 2 (S-42) BURLINGTON NOVERPASS P

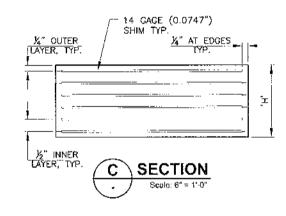
LINCH SCALE BAR ADJUST SCALE ACCORDINGLY

SHEET 91 OF 117





PLAN: ELASTOMERIC BEARING



BEARING DESIGN TABLE								
SERVICE I LIMIT STATE								
DEAD LOAD REACTION	18.6 KIPS							
LIVE LOAD REACTION (W/OUT IMPACT)	28.3 KIPS							
UNLOADED HEIGHT, 'H'	4.02 INCHES							
LOADED HEIGHT (DL)	3.98 INCHES							
SHEAR MODULUS AT 73' F	140 PSI							

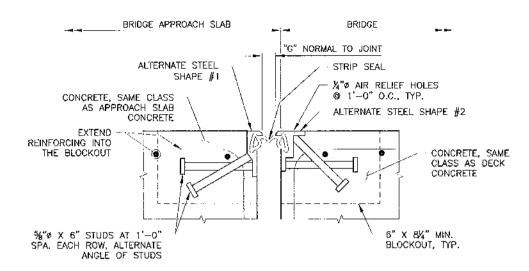
BEARING NOTES:

- 1. STRUCTURAL STEEL SHALL BE ASTM A36, GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH AASHTO M111
- 2. THE BEARING PADS SHALL BE BONDED TO THE DIAPHRAGM SHELF WITH AN APPROVED ADHESIVE.

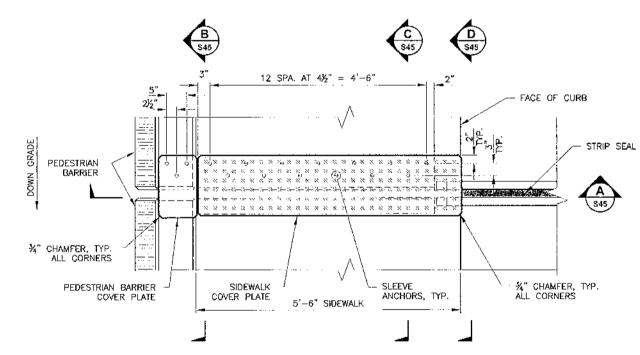


92 OF 117

SHEARER DESIGN 10.0 2013 Phirtre Ale II 43 Secula VIA 9001 2003 Phirtre Ale II 43 Secula VIA 9001 Phirtre Ale II 43 Phirtre Ale II 43



STRIP SEAL TYPICAL SECTION



PLAN - EXPANSION JOINT AT SIDEWALK

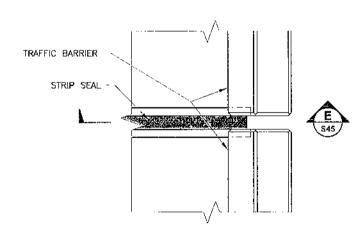
PIER 1 EXPANSION JOINT										
		OPENING "G" NORMAL TO JOINT								
MANUFACTURER	ITEM NAME	MIN.	MAX.	MIN. INSTALLATION WIDTH	@40°F	@64°F	@80°F			
D.S. BROWN	DSB STRIP SEAL A2R-400	½"	41/2"	1½"	2.12"	1.74"	1,49"			
WATSON BOWMAN	WABO STRIP SEAL SE-300	0"	3"	1½"	2.12"	1.74"	1.49"			
R. J. WATSON, INC	RJ STRIP SEAL 300	0"	3"	1½"	2.12"	1.74"	1.49"			
DYMAT CONSTRUCTION PRODUCTS INC.	DYMA STRIP SEAL J2000C	Х,"	4¾"	1½"	2.12"	1.74"	1.49"			

PIER 4 EXPANSION JOINT										
OPENING "G" NORMAL TO JOINT										
MANUFACTURER	ITEM NAME	MIN.	MAX.	MIN. INSTALLATION WIDTH	@40°F	⊕64°F	980°F			
D.S. BROWN	DSB STRIP SEAL A2R-400	½"	4½"	1½"	2.06"	1.67"	1.40"			
WATSON BOWMAN	WABO STRIP SEAL SE-300	0"	3"	1½"	2.06"	1.67"	1.40"			
R. J. WATSON, INC	RJ STRIP SEAL 300	O"	3"	1½"	2.06"	1,67"	1.40"			
DYMAT CONSTRUCTION PRODUCTS INC.	DYMA STRIP SEAL J2000C	Х,"	4¾"	1½"	2.06"	1.67"	1.40"			

STEEL SHAPE TYPES										
MANUFACTURER	ITEM NAME		$\frac{s}{1}$,	<u>~</u>		۲ کا ۲ کا ×		
		TYPE	S	Т	TYPE	٧	W	TYPE	X	Y
D.S. BROWN	DSB STRIP SEAL	SSCM2	1½"	3¾"	SSA2*	11/4"	2"	SSE2*	1"	11/2"
WATSON BOWMAN	WABO STRIP SEAL	M, R, P	2¾"	3¼"	Α	1¼"	2"	E	1½"	11/2"
R. J. WATSON, INC	RJ STRIP SEAL	RJM	2¾"	3½"	RJA	1¼"	2"	RJE	1¼"	11/2"
DYMAT CONSTRUCTION PRODUCTS INC.	DYMAT STRIP SEAL	С	2½"	3"	А	11/4"	2"	E	1%"	1½"

DO NOT USE STEEL SHAPES WITH HORIZONTAL LEGS IN CURB OR BARRIER REGION.

* TRIM VERTIAL LEG OF SSCM2 SHAPE FOR USE IN TRAFFIC BARRIER



PLAN - EXPANSION JOINT AT TRAFFIC BARRIER

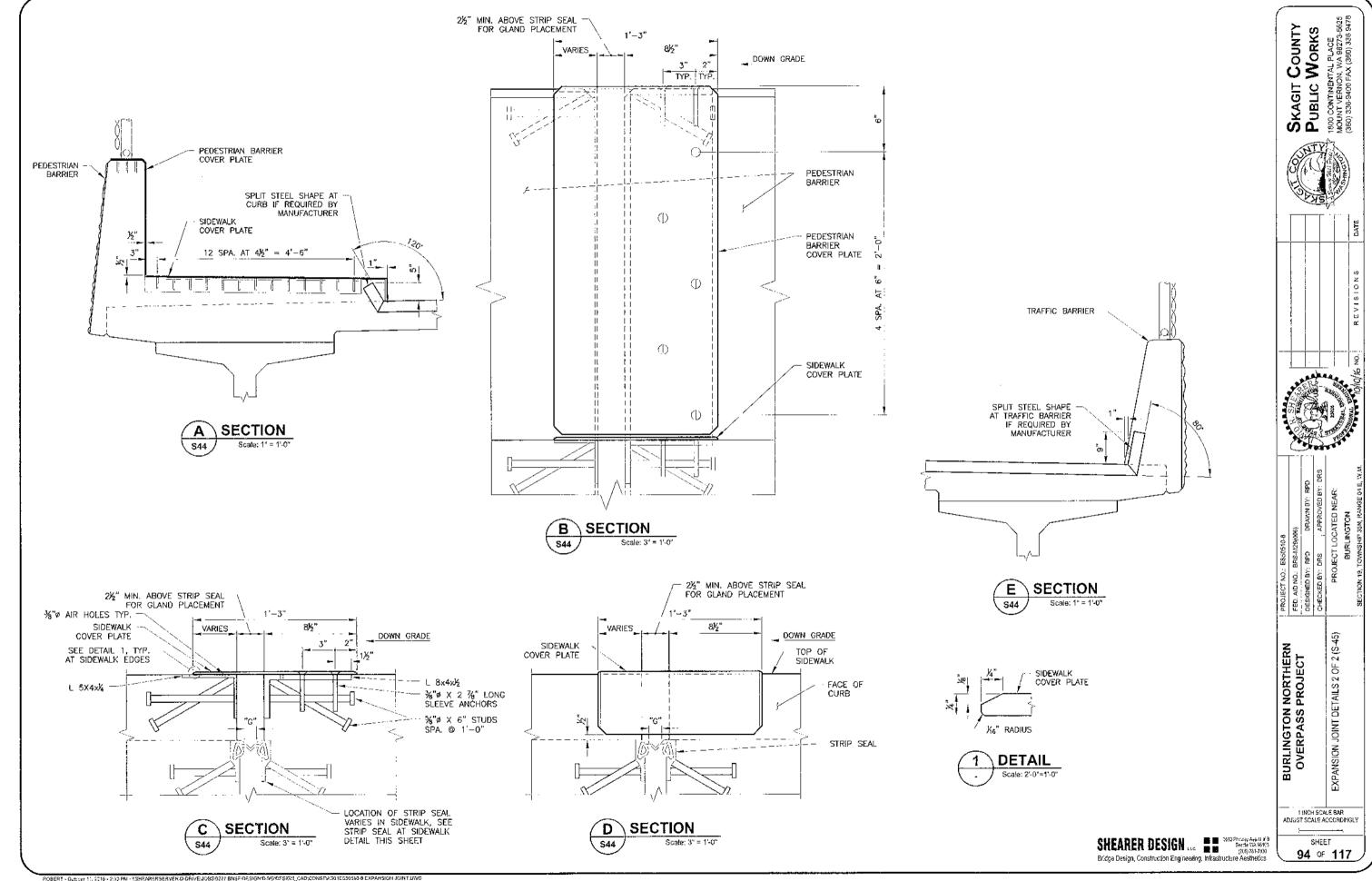
SHEARER DESIGN ... 1013 FAutoy As 91, 4 B Saute WA 93193 (25) 91,7438 Bridge Design, Construction Engineering, Infrastructure Aesthetics

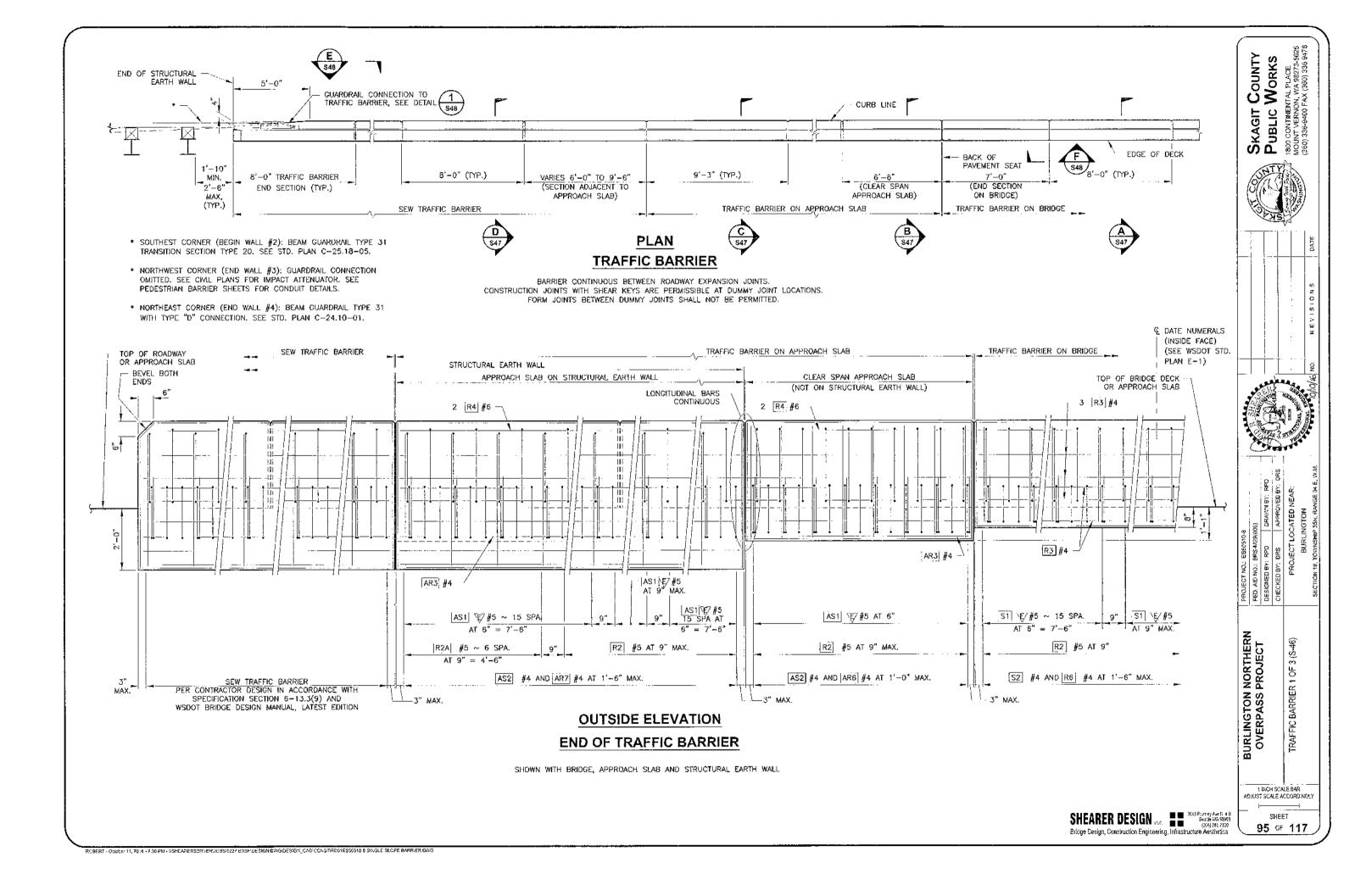
SKAGIT COUNTY
PUBLIC WORKS

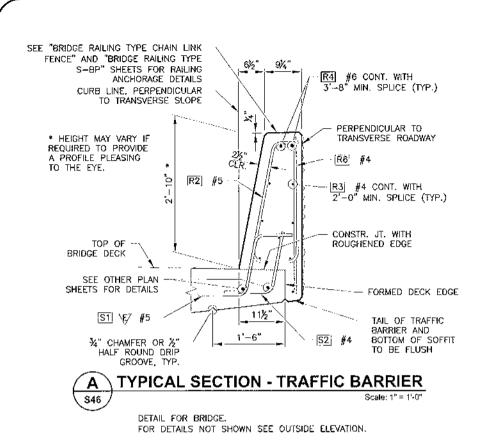
1800 CONTINENTAL PLACE
MOUNT VERNON, WA 98273-5625
(360) 336-9400 FAX (360) 336 9478 EXPANSION JOINT DETAILS 1 OF 2 (S-44) BURLINGTON NORTHERN OVERPASS PROJECT

> 1 INCH SCALE BAR ADJUST SCALE ACCORDINGLY

> > SHEET 93 OF 117







SKAGIT COUNTY
PUBLIC WORKS

1800 CONTINENTAL PLACE
MOUNT VERNON, WA 86273-5625
(360) 336-9409 FAX (360) 336-9479 BRIDGE _1'-0"_ S2 AND AS2 11% R9 **S1** 8¾" 1'-0" 8¾ | AS1 1114" VARIES 3¼" S1 FAND AS1 F R2A R2 i ALL DIMENSIONS ARE OUT TO OUT F = EPOXY COATED REINFORCING STEEL

* = DIMENSIONS TO POINTS OF INTERSECTION

BENDING DIAGRAM

CURB LINE

TOP OF —
ROADWAY AT CURB LINE
APPROACH SLAB

| SECTION | | Scale: 1" = 1'-0"

AS2 #4

DETAIL FOR CLEAR SPAN APPROACH SLAB.
FOR DETAILS NOT SHOWN SEE OUTSIDE ELEVATION
AND TYPICAL SECTION — TRAFFIC BARRIER DETAILS.

AR3

CURB LINE

TOP OF ROADWAY AT CURB LINE

APPROACH SLAB

AS2 #4

EXPANDED POLYSTYRENE

AS2 #4

STRUCTURAL EARTH WALL

Section | Scale: 1" = 1'.0"

DETAIL FOR APPROACH SLAB ON STRUCTURAL EARTH WALL. FOR DETAILS NOT SHOWN SEE OUTSIDE ELEVATION AND TYPICAL SECTION — TRAFFIC BARRIER DETAILS.

CURB LINE CONDUIT IN SECTION OF SEW TRAFFIC BARRIER NEAR END OF SEW #3. SEE PEDESTRIAN BARRIER SHEETS FOR DETAILS. ROADWAY TOP OF ROADWAY AT SECTION CURB LINE MOMENT SLAB MOMENTASLAB LENGTH EXPANDED POLYSTYRENE STRUCTURAL D SECTION

DETAIL FOR SEW TRAFFIC BARRIER.

BARRIER DIMENSIONS SHALL CONFORM TO TYPICAL SECTION —
TRAFFIC BARRIER DETAIL AND THOSE SHOWN HERE.
CONTRACTOR SHALL DESIGN SEW TRAFFIC BARRIER IN
ACCORDANCE WITH SPECIFICATION SECTION 6—13.3(9) AND
WSDOT BRIDGE DESIGN MANUAL, LATEST EDITION.

Scale: 1" = 1'-0"

\$46

BURLINGTON NORTHERN

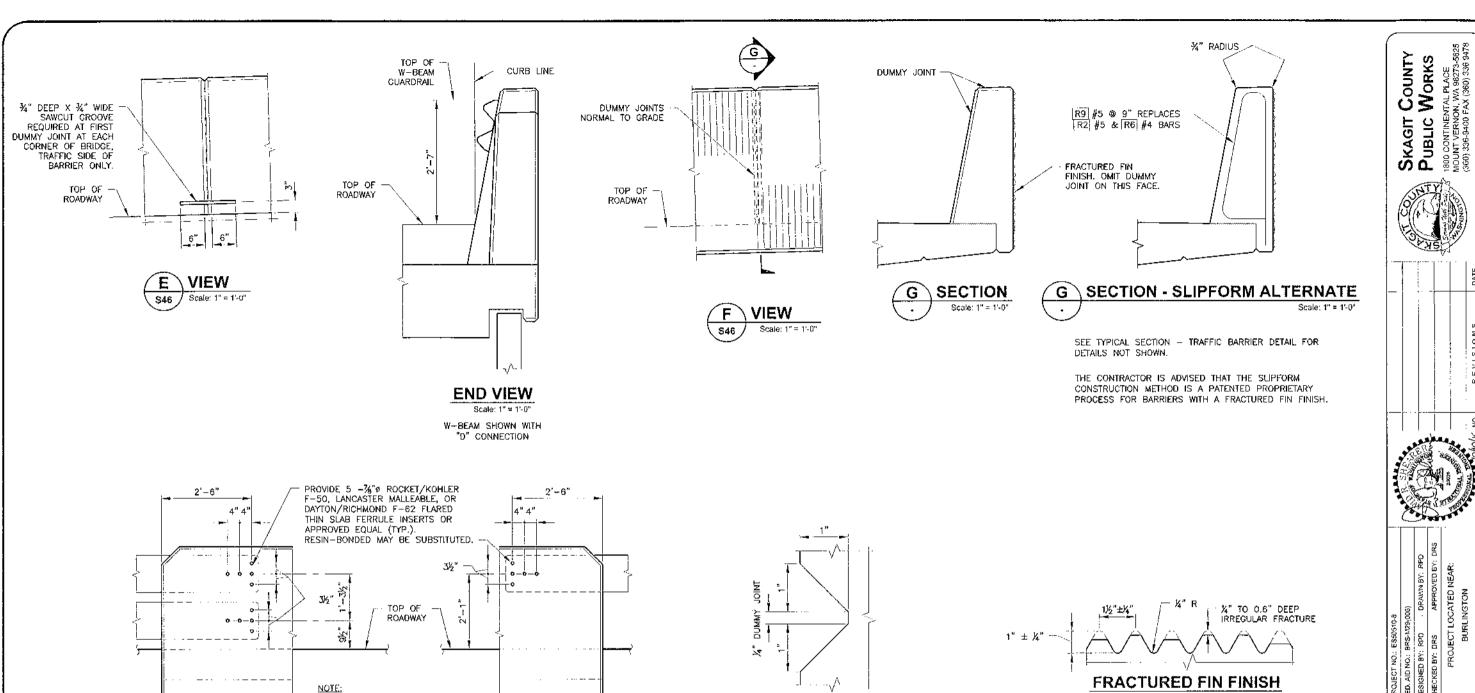
BURLINGTON NORTHERN

BURLINGTON NORTHERN

FED. Ald No.: Broad 2016/18

FED. Ald No

A\$1\E7#5



DUMMY JOINT

DETAIL

Scale: 1'-0" = 1'-0"

SOUTHEAST CORNER (BEGIN WALL #2) BEAM GUARDRAIL (TYPE 31) TRANSITION SECTION TYPE 20 (SEE STD. PLAN C-25.18-05)

NORTHEAST CORNER (END WALL #4) BEAM GUARDRAIL TYPE 31 TYPE "D" CONNECTION (SEE STD. PLAN C-24.10-01)

OUTSIDE ELEVATION

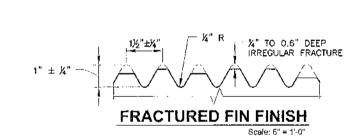
GUARDRAIL CONNECTION OMITTED AT

NW CORNER (END OF WALL #3).

TRAFFIC BARRIER - GUARDRAIL CONNECTION \$46

(GUARDRAIL CONNECTION TO SEW TRAFFIC BARRIER AT ENDS OF

STRUCTURAL EARTH WALLS, SEE CIVIL PLANS FOR GUARDRAIL TYPE.)

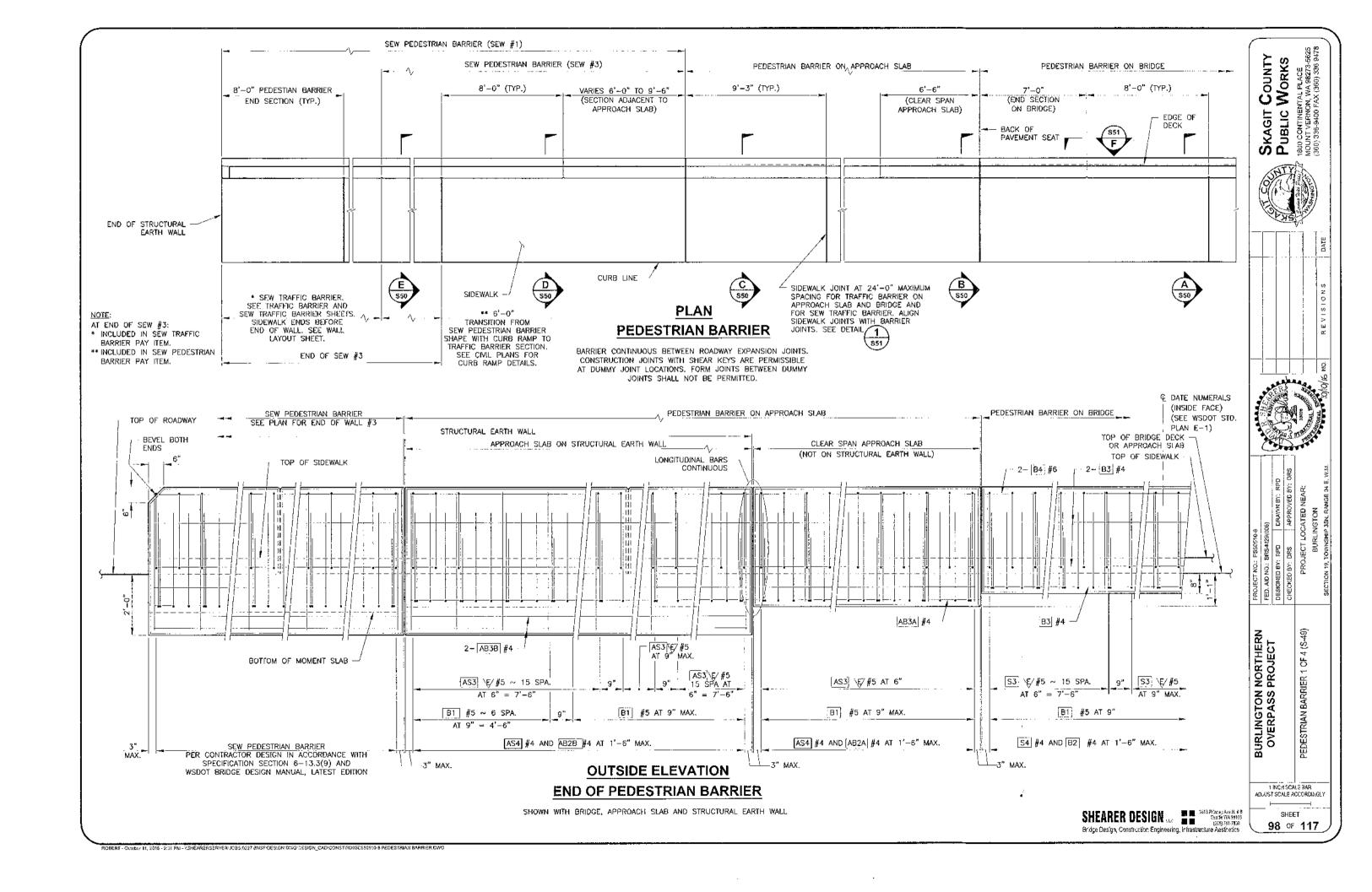


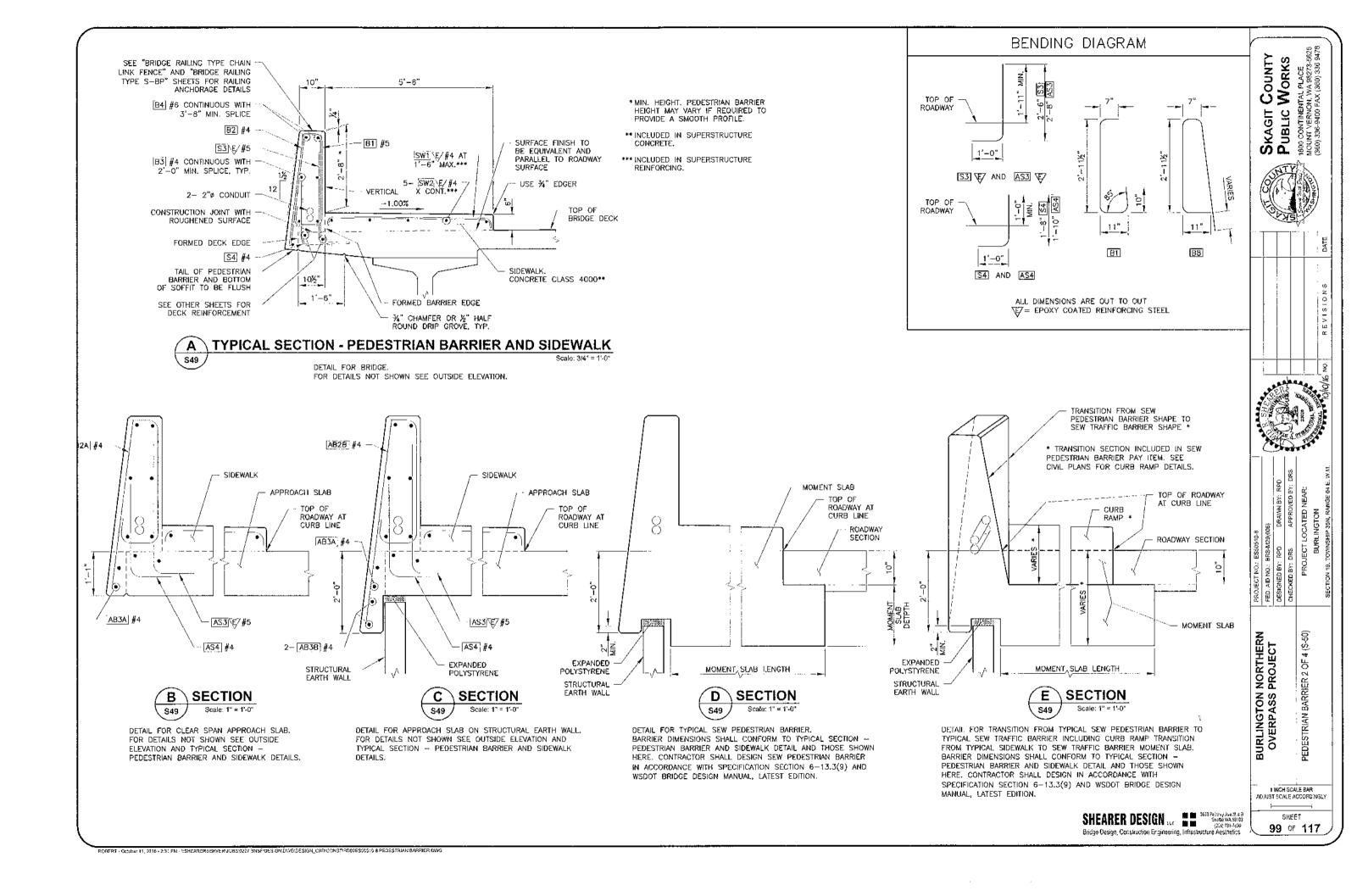
SHEARER DESIGN I.e. 3613 Frizzoy Au H. #8
Septie Wa 38193
(20) 370 Fridge Design, Construction Engineering, Infrastructure Aesthetics

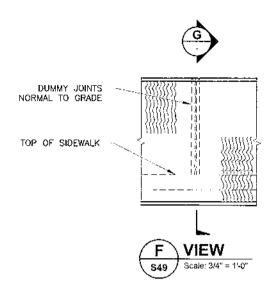
ADJUST SCALE ACCORDINGLY 97 of 117

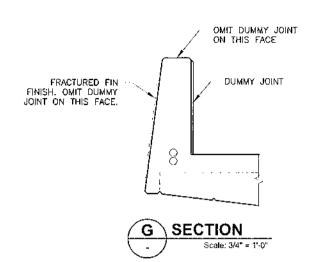
ROSERT - Calobe: 11, 2016 - 2 30 PM - VSHEARERSERVER/JO38/0227 BNSP/DESIGN/DWG/DESIGN_CAD/CCNS/TROD1ESS0510 8 SINGLE SLOPE BARRIER/GWG

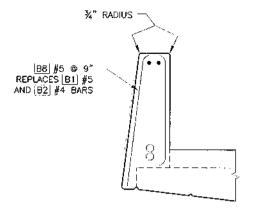
BURLINGTON NORTHERN OVERPASS PROJECT TRAFFIC BARRIER 3 OF 3 (S-48)







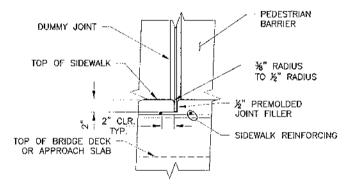


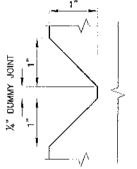




SEE TYPICAL SECTION — PEDESTRIAN BARRIER DETAIL FOR DETAILS NOT SHOWN.

THE CONTRACTOR IS ADVISED THAT THE SLIPFORM CONSTRUCTION METHOD IS A PATENTED PROPRIETARY PROCESS FOR BARRIERS WITH A FRACTURED FIN FINISH.

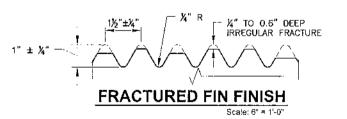




Scale: 1-1/2" = 1'-0"

SIDEWALK ON BRIDGE OR APPROACH SLAB SHOWN. CONTRACTOR SHALL INCORPORATE SIDEWALK JOINTS INTO SEW PEDESTRIAN BARRIER DESIGN.

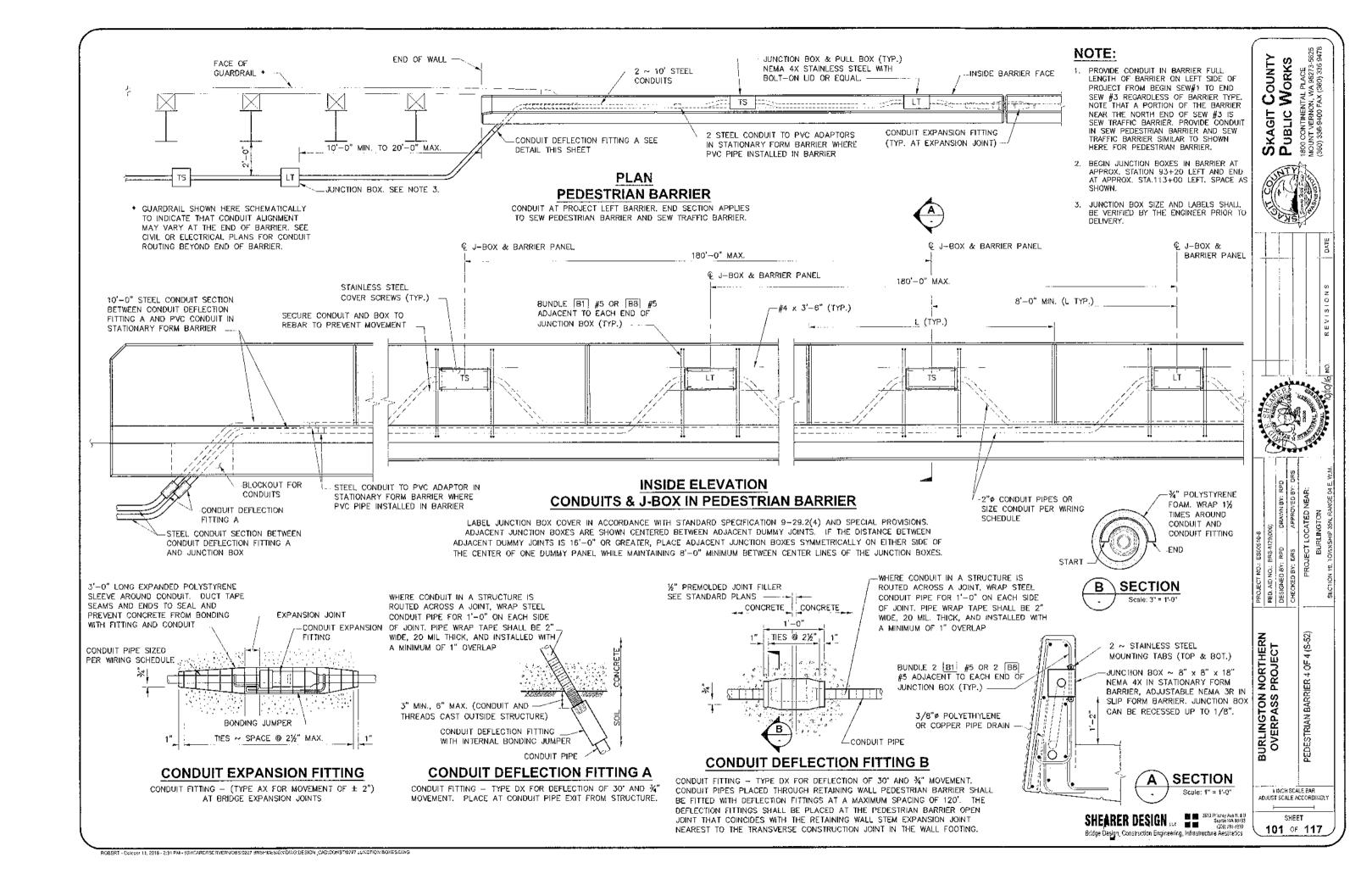


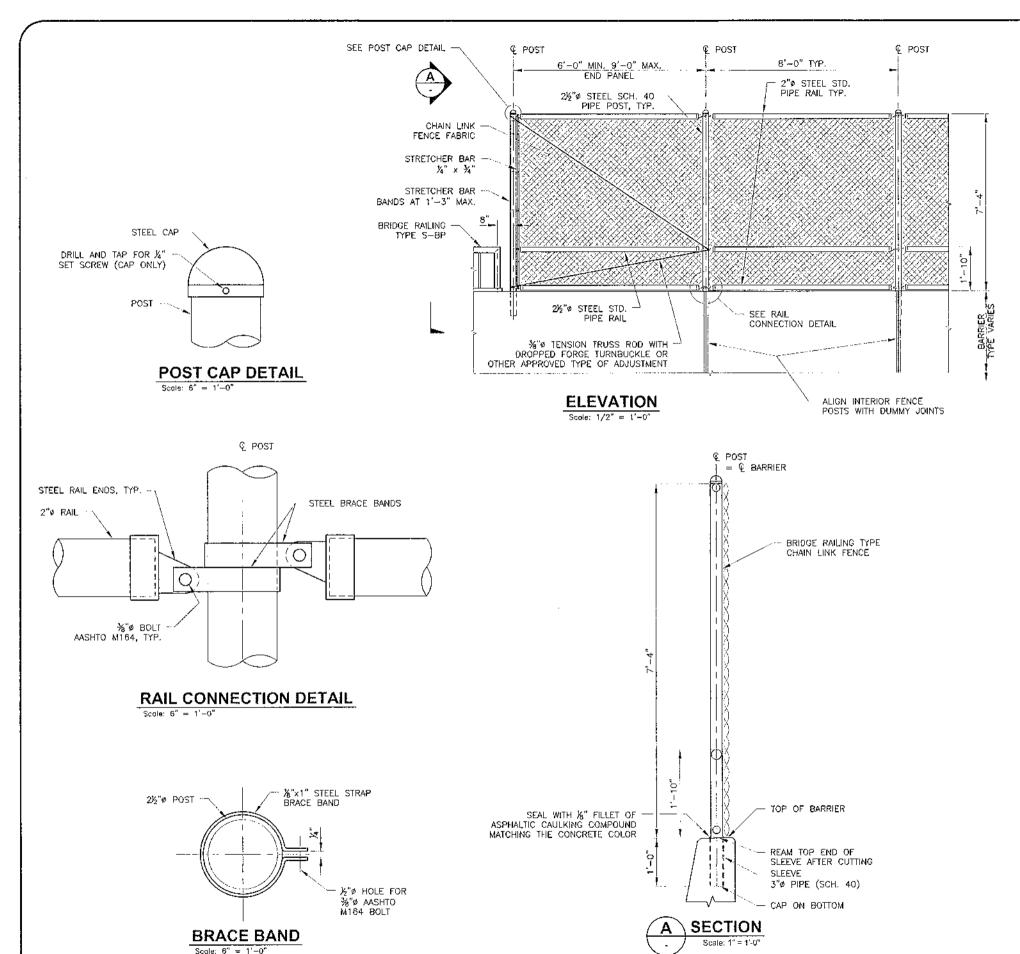


SHEARER DESIGN 1.10 2013 Frince, As Rt. 4 8 Sector WA 361/3 (20) Bridge Design, Construction Engineering, Infrastructure Aesthetics



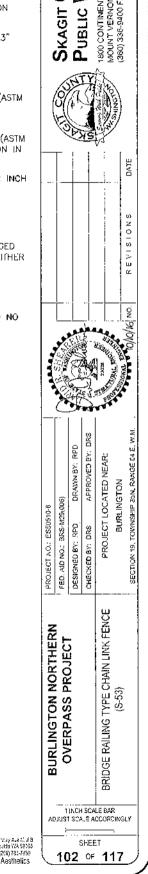
SHEET 100 OF 117





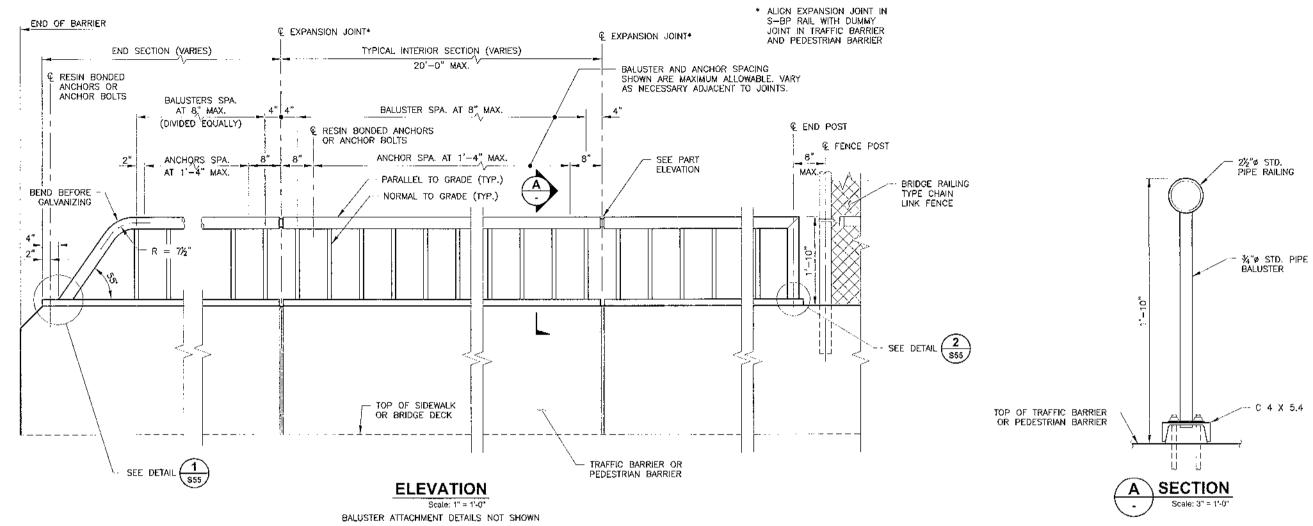
NOTES

- FENCING IS DESIGNED ACCORDING TO THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS 7TH EDITION WITH 2015 INTERIM REVISIONS.
- 2. APPLIED LOADING IS 0.015 KSF ACCORDING TO AASHTO LRFD SUBSECTION 13.8.2
- INSTALL ALL POSTS NORMAL TO GRADE IN THE LONGITUDINAL DIRECTION AND PLUMB IN THE TRANSVERSE DIRECTION.
- STRETCH CHAIN LINK FABRIC AND FASTEN TO VERTICAL POSTS AT 1'-3" MAX. CENTERS AND 2'-0" MAX. CENTERS TO HORIZONTAL RAILS.
- PROVIDE STEEL PIPE FOR POSTS AND RAILS CONFORMING TO ASTM SPECIFICATION A53 GRADE B, TYPE E OR S, GALVANIZED.
- PROVIDE STEEL PLATES AND SHAPES CONFORMING TO AASHTO M183 (ASTM SPECIFICATION A36) AND SHALL BE GALVANIZED IN ACCORDANCE WITH AASHTO M111...
- PROVIDE BOLTS, NUTS AND WASHERS CONFORMING TO AASHTO M164 (ASTM A325), WSDOT SPECIFICATION 9-06.5(3). GALVANIZE AFTER FABRICATION IN ACCORDANCE WITH AASHTO M232.
- 8. PROVIDE 9 GAGE WIRE FOR CHAIN LINK FENCE FABRIC WOVEN IN A 2 INCH DIAMOND MESH WITH KNUCKLE, TOP AND BOTTOM. FABRIC SHALL BE GALVANIZED CONFORMING TO AASHTO A392, CLASS 1.
- PROVIDE FITTINGS AND HARDWARE IN ACCORDANCE WITH WSDOT SPECIFICATION 9~16.1.
- 10. OMIT LONGITUDINAL RAILS AND ALLOW CHAIN LINK FABRIC TO BE BULGED 1½" OUT OF PLANE TO ALLOW FOR EXPANSION BETWEEN POSTS ON EITHER SIDE OF AN EXPANSION JOINT. PLACE POST CENTERLINE 1'-0" FROM CLOSEST EDGE OF EXPANSION JOINT UNLESS SHOWN OTHERWISE.
- 11. CHAIN LINK FABRIC SHALL BE WITHIN 14" OF CONCRETE SURFACE.
- 12. PROVIDE NO MORE THAN TWO FABRIC SPLICES PER RUN AND SPACED NO CLOSER THAN 50 FEET.



COUNTY

SHEARER DESIGN To send Princey Ave 11, 28 Section 14/18/2009 (2017) 2017



MAT'L	PART	MATERIAL SPECIFICATION						
	PIPES	ASTM A 53 GRADE B SCHEDULE 40 (STD. PIPE) GALVANIZED IN ACCORDANCE WITH AASHTO M 111.						
	WASHERS ASTM A 193 GRADE B7 GALVANIZED IN ACCORDANCE WITH AASHTO M 2							
STEEL	PLATES/BAR ASTM A 36 CHANNELS GALVANIZED IN ACCORDANCE WITH AASHTO M							
	DRIVE PI NS	ASTM A 276 TYPE 302 STAINLESS STEEL						
	ANCHOR BOLTS	ASTM F 593 TYPE 302 STAINLESS STEEL						
·	NUTS	TAMPER-PROOF TYPE OF EITHER CLEAR ANODIZED ALUMINUM OR ZINC ALLOY						

NOTES

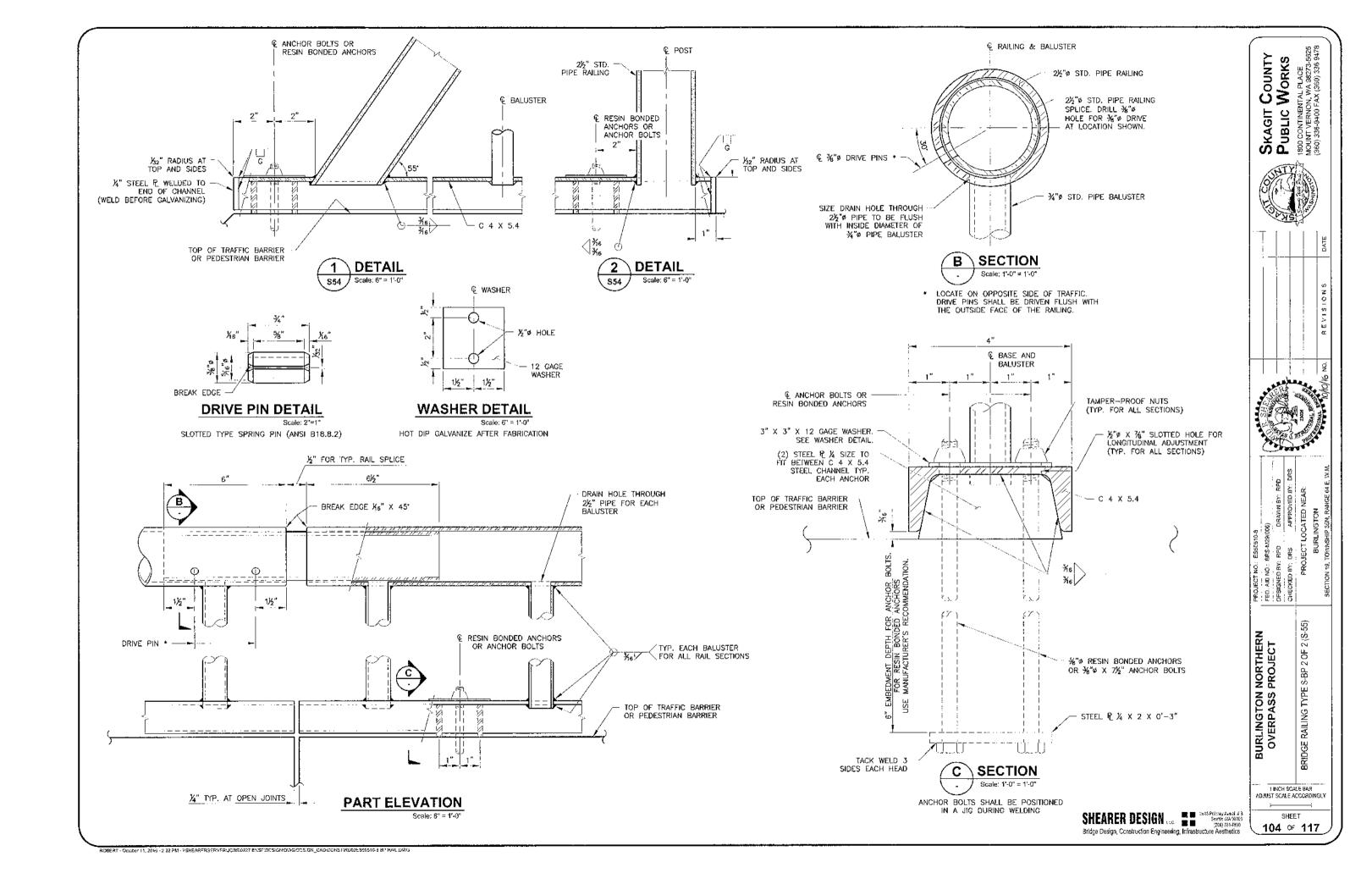
- SHOP DRAWINGS OF RAILING SHALL BE SUBMITTED FOR APPROVAL SHOWING COMPLETE DIMENSIONS AND DETAILS OF FABRICATION, GALVANIZING AND INCLUDING AN ERECTION DIAGRAM. MATERIAL SPECIFICATIONS SHALL BE PROVIDED IN THE SHOP DRAWINGS FOR ALL COMPONENTS.
- 2. CUTTING SHALL BE DONE BY SAWING OR MILLING AND ALL CUTS SHALL BE TRUE AND SMOOTH.
- PIPE RAILING, PIPE BALUSTERS, RAILING SPLICE AND CHANNELS SHALL BE ADEQUATELY WRAPPED TO ENSURE SURFACE PROTECTION DURING HANDLING AND TRANSPORTATION TO THE JOB SITE.
- 4. WELDING SHALL CONFORM TO AWS D1.1.
- 5. PIPE RAILING AND SPLICES MAY BE HEATED TO FACILITATE FORMING OR BENDING
- BRIDGE RAILING SHALL BE HOT DIP GALVANIZED AFTER FABRICATION PER AASHTO M111. ALL COMPONENTS OF RAIL PANEL, POSTS, AND SPLICE SLEEVES SHALL HAVE ALL SURFACES HOT DIP GALVANIZED PRIOR TO ASSEMBLY.
- REPAIR ALL GALVANIZED STEEL SURFACES DAMAGED BY FIELD OPERATIONS BY PAINTING THE DAMAGED AREAS WITH TWO COATS OF PAINT CONFORMING TO SPECIFICATION 9-08.1(2)B.

SHEARER DESIGN 11.0. Shall Principly Ase Bit a Bit Security Via a Bit

SKAGIT COUNTY PUBLIC WORKS PRO, BURLINGTON NORTHERN OVERPASS PROJECT RAILING TYPE S-BP 1 OF 2 BRIDGE 1 INCH SCALE BAR ADJUST SCALE ACCORDINGLY SHEET

103 OF 117

ROBERT - Coloder 11, 2016 - 2:31 PM - #SHFARERSFRVFRYORS/2227 BNSPGESIGN DWG/DESIGN_CAD/CONSTRAOZESSC576-6 811 PAL/DWG



S = BAR IS INCLUDED IN - E = BAR IS TO EPOXY COATED S = BAR IS INCLUDED IN E = BAR IS TO EPOXY COATED SUBSTRUCTURE QUANTITIES SUBSTRUCTURE QUANTITIES COUNTY V = BAR DIMENSIONS VARY BETWEEN DIMENSIONS V = BAR DIMENSIONS VARY BETWEEN DIMENSIONS Works L = LUMP SUM QUANTITY SHOWN ON THIS LINE AND THE FOLLOWING LINE L = LUMP SUM QUANTITY SHOWN ON THIS LINE AND THE FOLLOWING LINE T = TRANSVERSE OR T = TRANSVERSE OR E = EARTHQUAKEE = EARTHQUAKE DIMENSIONS (OUT TO OUT) DIMENSIONS (OUT TO OUT) KAGIT UBLIC LENGTH LENGTH MASS MASS υ Z e1 e2 Z LOCATION LOCATION FT IN LΘ FT FT LB FT IN FT IN LET FT IN IN FT IN INI IN IN FT N $\bar{\Omega}$ TER 1 SHAFTS PIER 4 COLUMNS TC9 UNGTO 7197 14 <u>.ONG14</u> 36479 0 8.0 60 0.0 1258 CSOT SPTRAL 0 | 8, 0 83 9. 1 PIRAL HIP To ! 9 of 275 1273 | 8 6 | 5739 862 0 8.0 50 0.0 SSOZ SPIRAL BUJ 57 L 0. 0. 0502|SPTR<u>AL</u> 503 SPTRAI 504 SPTRAL 68 !1. 0 S142 LONGLT 60 Q. O 4406 DDG5 SPIRAL 3 [8.0] 2 [0.0] 0 | 5. 45 [1.6 144 CSC/|SPTRAL 284 6.1 890 S143 LUNGLI 79 7. 11688. <u>0 | 8, 6 | 60 | 0.</u> - 0 | 8, 0 | 60 | 6, 0 395 | 9. 1 237 /2 | [6, 0 | 0 6. 0 0 8. 0 60 fc. 0 3188 5.4 19154 PIER I CAP AND END DIAPH. SSO4 SPIRAL BUT -46 ± 3.7 2460 PRII CAP LINGII 43 ± 0.0 PIER 3 SHAFT 46 | 0.7 2447 113 CAP LINGLE \$142 LONGLE 2480 60 | 0. 4406 <u> 2114 CAP UIMGEL</u> 2461 S143 LONGIT 14 116883 43]. 46 SGÖ3 SPIRAL TOP 395 9. PILS END BLAPH | CNGIL 47 1503 237 47 <u> P116 END DIAPH, LONGII</u> 1584 SSO4 SPIRAL BOT 0 6.0 - 0 | 8, 0 | 60 | C, 0 3188 ∤ 5. <u> 19154</u> <u>6 3</u> <u>PILZ CORBOL TOP LONGLE</u> 22 3 7 474 45<u>0.0</u> PEER 4 SHAFES <u>PSOT CAP LONGET</u> <u>| 45 | 0. 0.</u> \$144 LUEGTE PSOT END DIAPH LONGIN 45 0 0 1032 2 2.0 800 279 PSOS <u>CAP TRANSV.</u> <u>scot spiral</u> top 1258 SSOS SPIRAL BOT 53 | 0.0 | 0 [2, 0] 0 | 8 0 | 60 | 6.0 1184 14.0 5336 PSOB EMD DIA<u>PH KEY TRANS</u>Y. 88 7 3 8 0 2 6 0 l <u> 500</u> P504 EMD CEAPER FILE - Office and office an 119 19 ²505 EMD DEAPH LUNGIT PIER I COLUMNS P508 DIAPE, END CAP areali ascir 41 [9, 0 8085 2 4.0 | 3 | 0.0 118 262 C56: SPIRA. 60 0. 83 9. P599 CAP LUNGIT. 8 | 0, 0 | 862 647 60 | O. C T CAP STERRUP :502 SPIRAL PSO2 CAP TIE 1320 60 li 5031 SP [ŔA. 205 9. 1 C504 SPIRAL 68 11.0 PÁGS ENÚ DIAPH. TRANSÝ. 0.0 60 0.0 CSOS SPIRA: 5 0.0 60 0.0 45 [1, 6] 14 9.0 £173 PSOMENU DIAPH, FRANSY. C596 SPTRAE PAGE FAIR DIAPAL CORBEL TRANSY. 3 8.1 9.0 0 [3 [6.0] 430 PIER PICCIONNS STORWALL HORIZ. 6 8.1 49 0.8 DOENALE TICKEZ. 111 5050 102 LUNGI I. LUREL I IDEVAL: VERT 14 11 10. 249 [0.0]CLOS LONGIT, CTR RT 48 8.3 5028 STDEWALL TOP 4966 0104[LBNG1T. -48 | L.D. 551 SPËRAL /16 3652 SPTRAL 60 2373 0 8.6 60 0 0 1845 653 SPTRAL 307 2.4 60 654 SPIRAL -65 | 2, 9 395 879 55 ESPTRAL SNEE SNEE 656 SPIRAL 4 8.0 14 Î O. Ŝ 60 0.0 352 10. 530 0.18.0 3657 SPIRAL CIR LI 350 2.8 526 516 493 1 E 8 C658 SPIRAL CIR-RI 343 6.5 0 8 0 60 0 328 3.0 C659 SPIRAL RT 0 /. 0 | 8, 0 | 60 | TES 3 CDL DAMS 4846 ONGLE, LL ONG HE CIR-L 4914 SNSIT, CIR-RI 48 1.0 4966 TYPE 74 (NO HOOKS) BENDING DIAGRAMS OF 3 (S-U --- ---4975 NOTES: CINGLE 48 | 2 48 2 (TYPE 72 518VL 710 TYPE 50 REINFORCING FOR CONCRETE BARRIERS, BRIDGE BURLINGTON OVERPASS 3 8.0 50 0.0 394 11.6 2373 PTRAL 9 0.3 0 4 APPROACH SLABS, AND GIRDERS NOT SHOWN IN TYPE 73 PIRAL 0 4. 9 8.0 60 0.0 307 2.4 1840 TYPE 51 IST 0 8.0 CO 0.0 0 8.0 G3 0.0 395 879 PIRAL 2. THE FINAL NUMBER AND LENGTH OF BARS SHALL BE DETERMINED BY THE CONTRACTOR FROM THE PLANS. PIRAL 146 (4.4 TYPE 54 U 12 /E PTRAL LT 1.5 0 7.0 0 8.0 60 0.0 300 - 0 451 Ų_ TYPE 56 475 316 1 3 PIRAL CTR-LT 3 L 0 L7.0 BEND FOR TRANSVERSE BARS DUE TO ROADWAY ADD Y FEET FOR EVERY Z PIRAL CTR-RT 328 (10.3 494 CROWN CONDITIONS HAVE NOT BEEN SHOWN. THESE BARS SHALL BE BENT AS REQUIRED TO CONFORM FEET OF SPIRAL LENGTH TO TYPE 90 TYPE 57 496 ACCOUNT FOR SPLICES 0 8.0 60 0.0 330 4. 0663 PIRAL RI -0.1TYPE 58 TO THE CONFIGURATION OF THE STRUCTURE. TYPE 67 -135* FOR W=O, MECHANICAL COUPLER IS REQUIRED LINCH SCALE BAR ADJUST SCALE ACCORDINGLY TYPE 80 02 2 TYPE 89 SHEARER DESIGN 📲 61 TYPE 84 SHEET TYPE 79 105 of 117 Bridge Design, Construction Engineering, Infrastructure Aesthetics ROBERT - October 11, 2016 - 2,32 PM - NSHEARERSERVERUCGS 0227 BNSF/DESIGN DWG/GESIGN_CAD/CONST/BL04E550510-8 BARLIST/DWG

S = BAR IS INCLUDED IN SUBSTRUCTURE QUANTITIES E = BAR IS TO EPOXY COATED S = BAR IS INCLUDED IN E = BAR IS TO EPOXY COATED SUBSTRUCTURE QUANTITIES V = BAR DIMENSIONS VARY BETWEEN DIMENSIONS V = BAR DIMENSIONS VARY BETWEEN DIMENSIONS SHOWN ON THIS LINE AND THE FOLLOWING LINE SHOWN ON THIS LINE AND THE FOLLOWING LINE L = LUMP SUM QUANTITY L = LUMP SUM QUANTITY T = TRANSVERSE OR T = TRANSVERSE OR E = EARTHQUAKE E == EARTHQUAKE DIMENSIONS (OUT TO OUT) DIMENSIONS (OUT TO OUT) LENGTH MASS LENGTH MASS e1 e2 SIZE U Z | ⊕1 | € IJ Z LOCATION LOCATION E FI IN FT IN SEG. ≥ [Fr | IN FT IN FT IN FT L8 FΓ LΒ IN FT IN FT | IN FΤ IN PIER 2 DAY AND PIER DIAPH 665 DIVLH LI AP LUMGE 3616 38 1.8 2433 2663 DIAPH TEP THE 3 ! 3, 9 isa cap luwati 10.0 [57 | 7, 8 | 3676 53 CAP LUNGII. 2496 P670 | SPTRA . 199 [11.4 54 CAP LUNGIT 3803 DIAPH, LUMGIT 59 7.8 PIER 4 CAP AND END BLAPH. DIAPH. LONGIT. (LIP 2592 CAP LUNGIT 2447 2218 45 0. 2112 CAP 1.0NG15 -22 [89] 96 8.0 12 0.0 2480 DIAPAL LUNGUL 90 8.0 1411 PI13 CAP LUNGI 45 .1 9<u>6 [5</u> 46 2467 552 DIAPH, JUNGILI. 1201 P114 CAP LONGIT. 47 L. PETS <u>END DIAPH LON</u>GEL. 1503 5 | 0.9 1143 43 | 10. 0 CORVALL PORTA Î7 | 4. 0 253 289 PI16 END DIAPH LONGIT 47 9. 1524 22 474 17 4.0 SIDEWALL HORIZ <u>2117 CORBERT THE CUNGLE</u> 4.56 <u>P50: CAP LUNGE</u>T. UBIOUSE CORNERS 3 11.8 <u>45 0.0</u> <u>P501 END DIAPH. L</u>IINGIT, 45 0.0 1932 899 TIB FUSE: CHRNERS 0 | 0. 3 11.8 63 AQUIR CURNERS 0 + 0. 0 | 4, 5 | 0 | 0, 0 | 3 P502 CAP TRANSY. 563 ACUTE CURNERS 0.0 P503 END DIAPH, KEY FRANSY, 88 74 1 <u>: [6,0]</u> 500 5 L - 16 L 80 CAP STERRUP <u>2504 END DIAPK, TIE</u> 23 | 58 | 3 3 9 2501 PSOSTEND DIAPH, EUNGIL 19 | 0.0 119 CA2 IDE ID2 18 14 2 0 |.o.q|<u>3</u>|o.q[119 CAP YERT. 258 ..6160 P508 <u>C1APEL END CA</u>P DIAPH TIE 809 599 CAP LENGIT 8 | 0.0 2601 CAP STERROP 2602 CAP TEE 1425 15 3.6 655 DIAPH TOP FIE 1179 62 72 l 8. 0 556 CAP VERT 2759 1326 88 72 P603 END DIAPH TRANSV. -15 bő 2166 P657 CAP STERRUP 15 11. 977 76 | 8. O | 16 [10.] 34 327 ND DIAPH TRANSV. 2638 CA2 TIE TO? YD DIAPH, CORBIT TRANSY. 3 6.0 430 78 99 TOEWALL HORTZ 235 6 | 8.0 DEVALL MORTZ. 818 TOPWALL VERT -lig. (IDEWALL VERT. 2 : 0. 8 4. 8 STOEWALL TOP 662 DIAPH TIE 1 CAP LUNCIT 46 3.7 <u> 13P TI</u> 3 | 3, 5 NTERMEDIATE DIAPH. 199 [1] 3926 2670 SPIRAL 6 10.0 6 6.0 0 8.0 1201 V2RAST TUAT 0 5.0 12 2.1 4 | 8.0 <u>2183</u> 395 IRANSV. 4 | 5.0 | 4 | 5 9 | 0.3 6 2.0 7 7.0 PIER 3 CAP AND PIER DEAPH. DNGIT, SIDE 1:1 & 3-3 3612 483 CAP LONGIT. UNGEL. SIDE 1 2 & 3-8 1413 LUNGIT SIDE 1-3 & 3-1 9 5. 604 38 2433 ÍCÁP LEMGTÓ 12 54 -36 | 6.0 <u>3676</u> 2496 11 7 148 53 CAP LUNGER. 1421 LUNGII. S<u>PAN 2. ALL</u> L<u>UMGI</u>I, INP 1-1 & 3-3 49 ; 0, 294 PAP LUNGI I IUP 1-2 & 3-2 357 - 59 | . 3803 1612]L<u>DXGFF</u> DIAPH, LUNGII. 58 | 0.4 2592 1613 LOMOIT. TOP : 3 % 3-1 DIASH, LONGÜT, TUP 1621 LUNGII, TUP ŞPAN 2, ALL 93 ! 4, 9 CAP L<u>ONGET.</u> 20.8 100 96 | 8.1 2218 90 | 4. 0 9.01 90 J. 8. C 141 17<u>11 LUNG(T. TARU 1-1 & 3-3</u> 41 7.0 41 DIAPH. LONGIT 1201 1712 LONGIT. THRU 1 2 & 3-2 50 <u>6. 0</u> 5. 0 50 | 6, 552 DTAPOL LONGET <u>96 |5</u>0 <u> 1713 UNIGET. TH</u>RU 1-<u>3-2-3-1</u> 6.0 341f 1143 66 <u>[1, 9</u> 253 72) LUNCHE. THRU SPAN 2, ALL 6, 0 80 1964 1.0 IDEWALL HURLA SIDEWALL MORIZ 16 50 589 HROUSE COSNERS 3 11.8 TYPE 74 (NO HOOKS) BENDING DIAGRAMS Ų NOTES: DETUSE CHRNERS 0.3 3 [1].8 TYPE 72 560 ACUTE CORNERS TYPE 50 1. REINFORCING FOR CONCRETE BARRIERS, BRIDGE 2 0.0 2363 ACUTE CORNERS 0 0.0 APPROACH SLABS AND GIRDERS NOT SHOWN IN 651 CAP STERRUP 3663 TYPE 51 THIS BARLIST. 25<u>11</u> 6160 652 CAP THE TUR 9 2. 14 2.0 2. THE FINAL NUMBER AND LENGTH OF BARS SHALL U_ _ 653 CAP VERT. TYPE 54 809 W..... 6 6 BE DETERMINED BY THE CONTRACTOR FROM THE اساين 654 D.(APH. III) TYPE 56 <u>755</u>5 [B][APH | TUP | ⊊[-1179 ADD Y FEET FOR EVERY Z 656 CAP VERT 2759 FEET OF SPIRAL LENGTH TO BEND FOR TRANSVERSE BARS DUE TO ROADWAY TYPE 57 TYPE 90 ACCOUNT FOR SPLICES P697 CAP STORRUA 13 lu 9// CROWN CONDITIONS HAVE NOT BEEN SHOWN. TYPE 58 TYPE 67 22 | 3 THESE BARS SHALL BE BENT AS REQUIRED TO 135 FOR W=0, MECHANICAL U 327 P658 CAP (115 TU) CONFORM TO THE CONFIGURATION OF THE COUPLER IS REQUIRED STRUCTURE. TYPE 80 e2 TYPE 89 235 el TYPE 84 TYPE 79 -14 | 4.0 660 SIDEWALL VERT.

COUNTY WORKS

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Section 1

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PROJ DESIC CHEC

NORTHER PROJECT

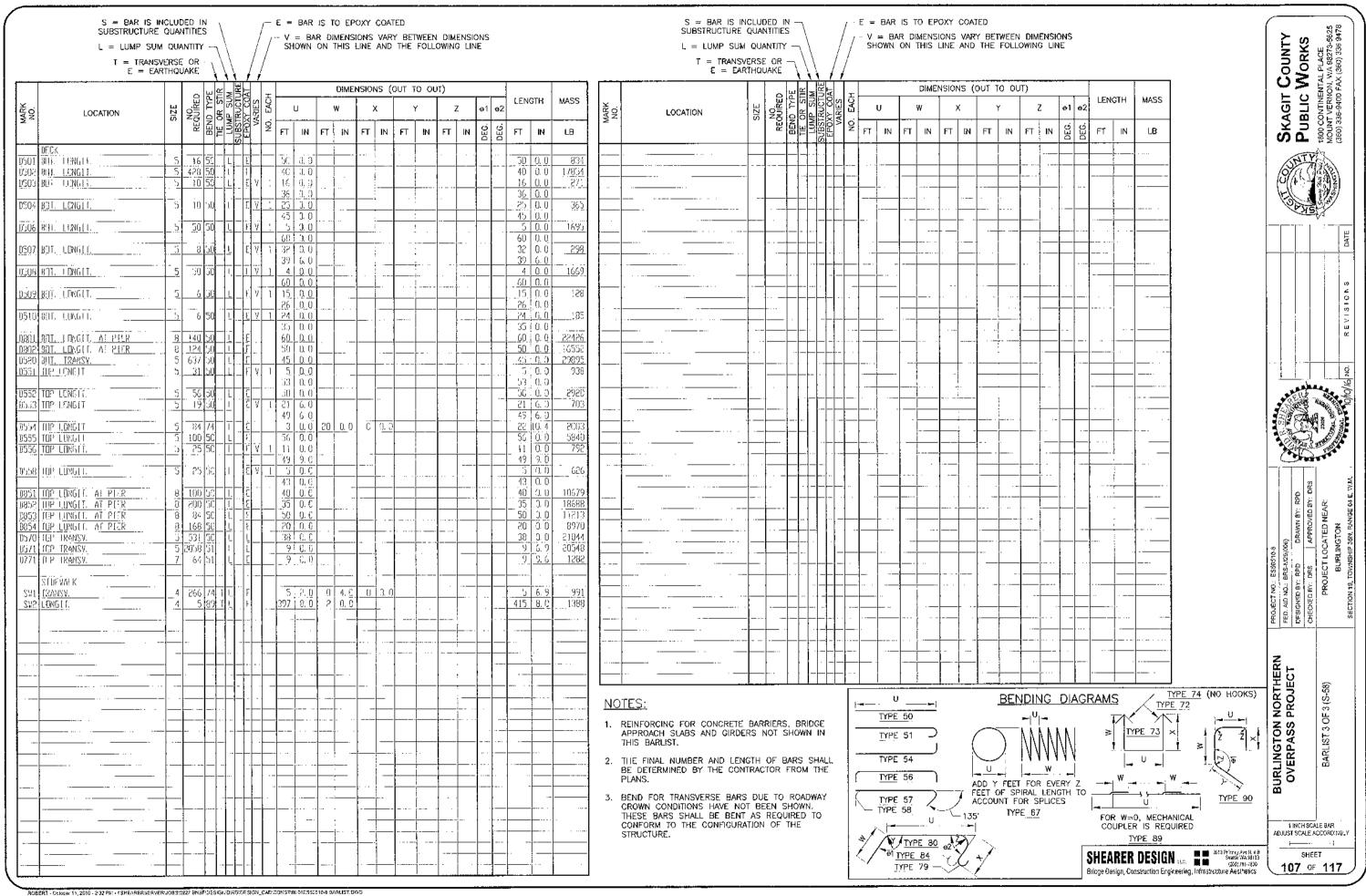
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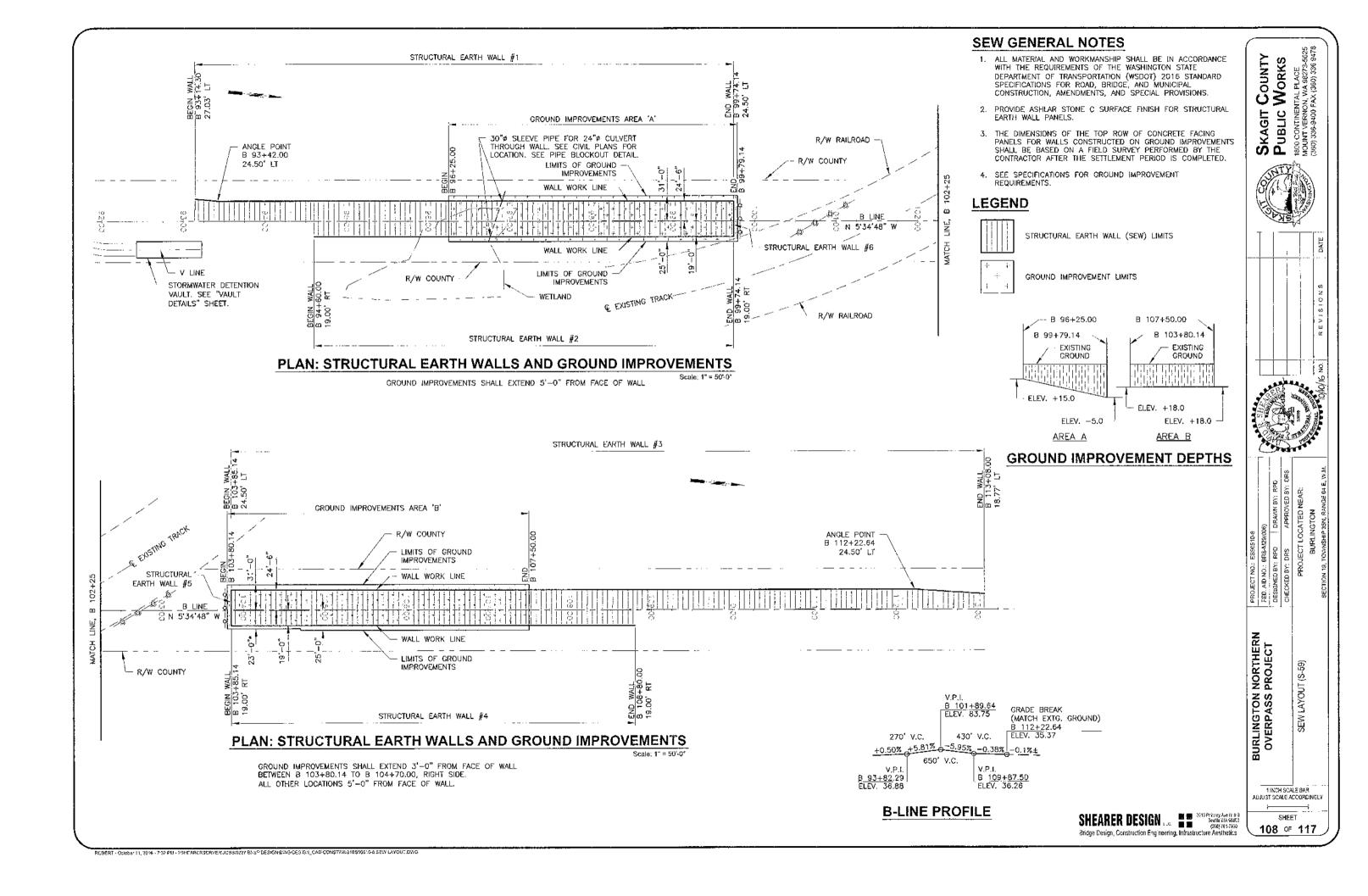
2

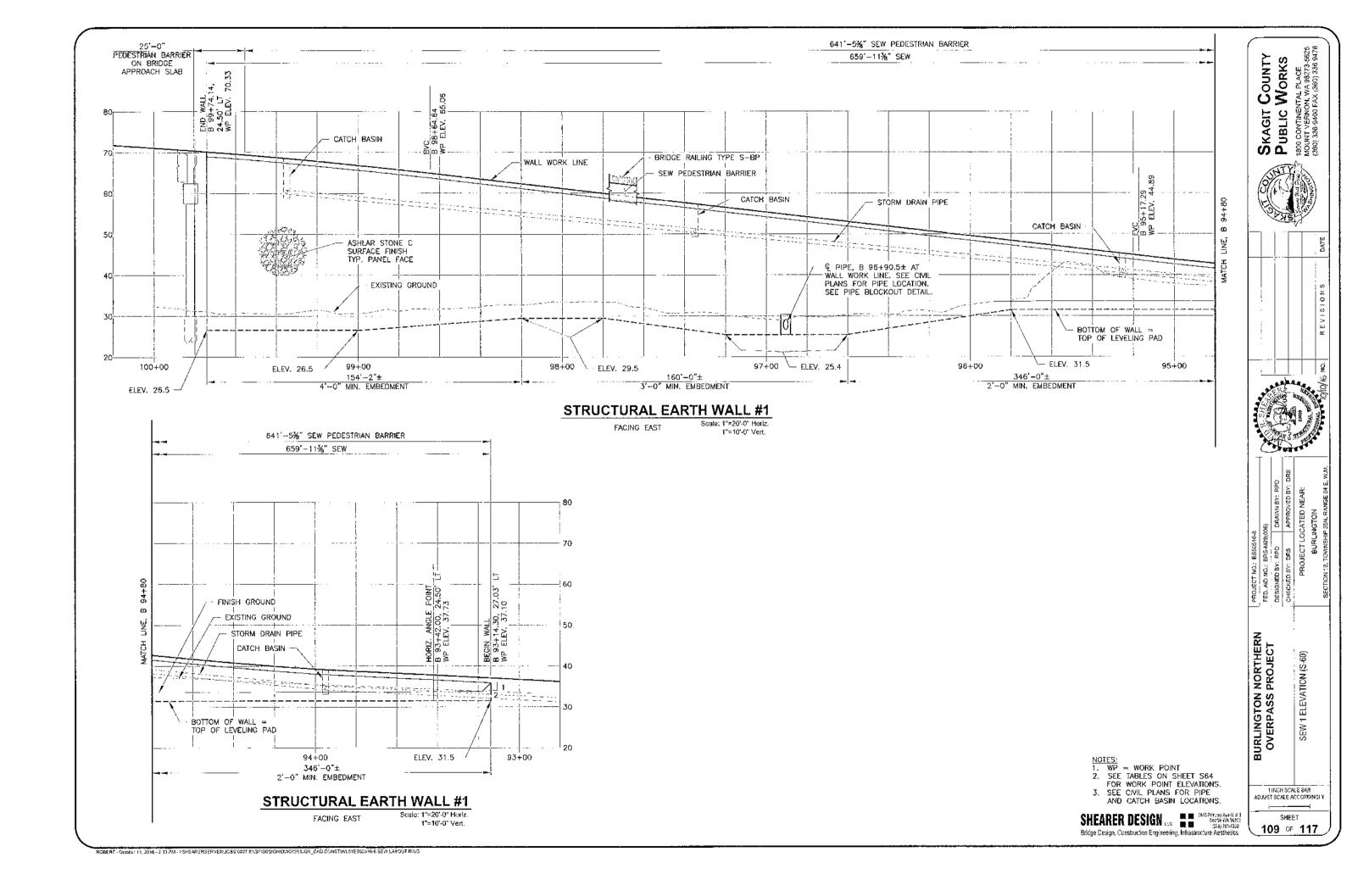
1 INCH SCALE BAR ADJUST SCALE ACCORDINGLY

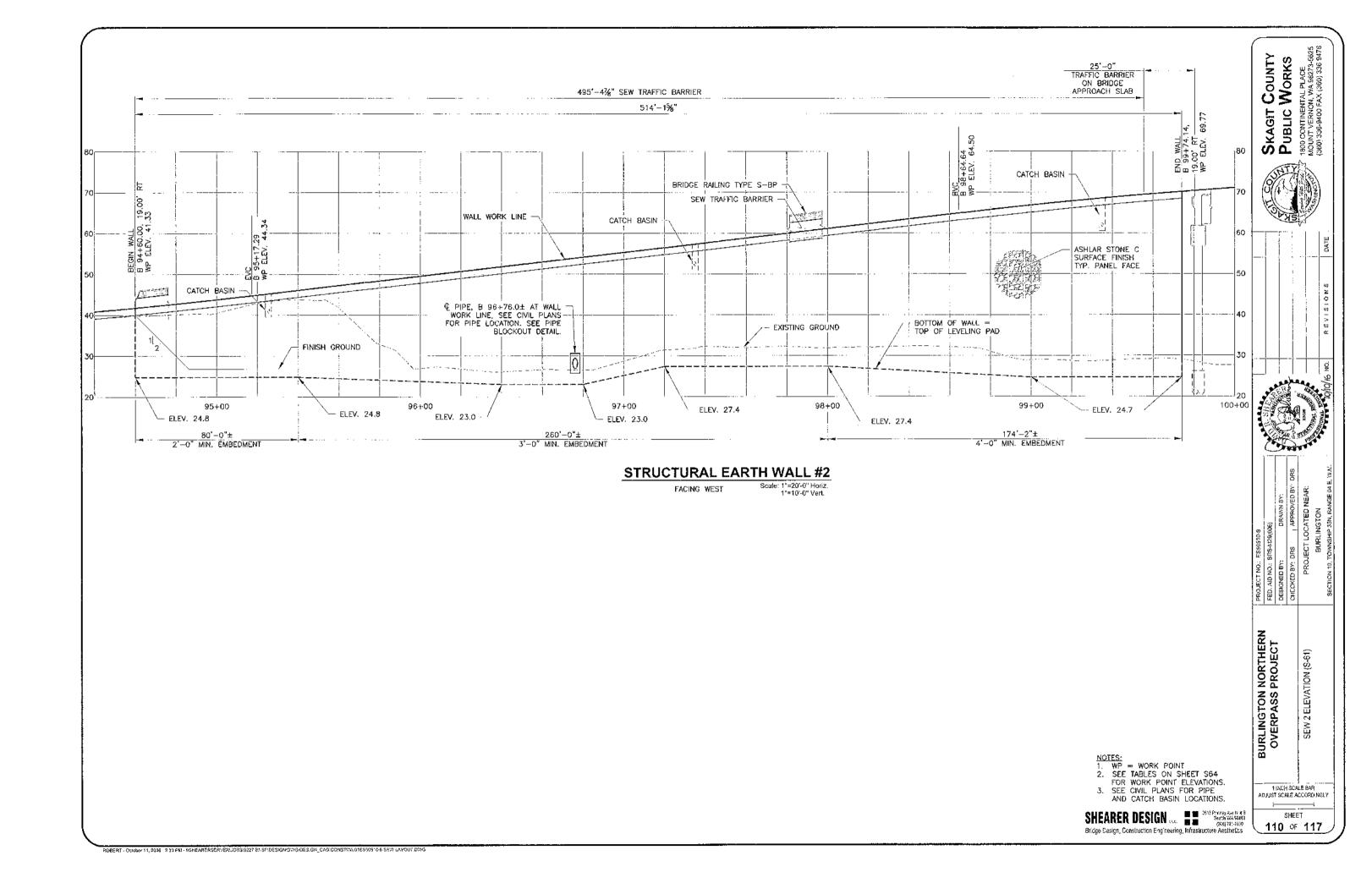
SHEET

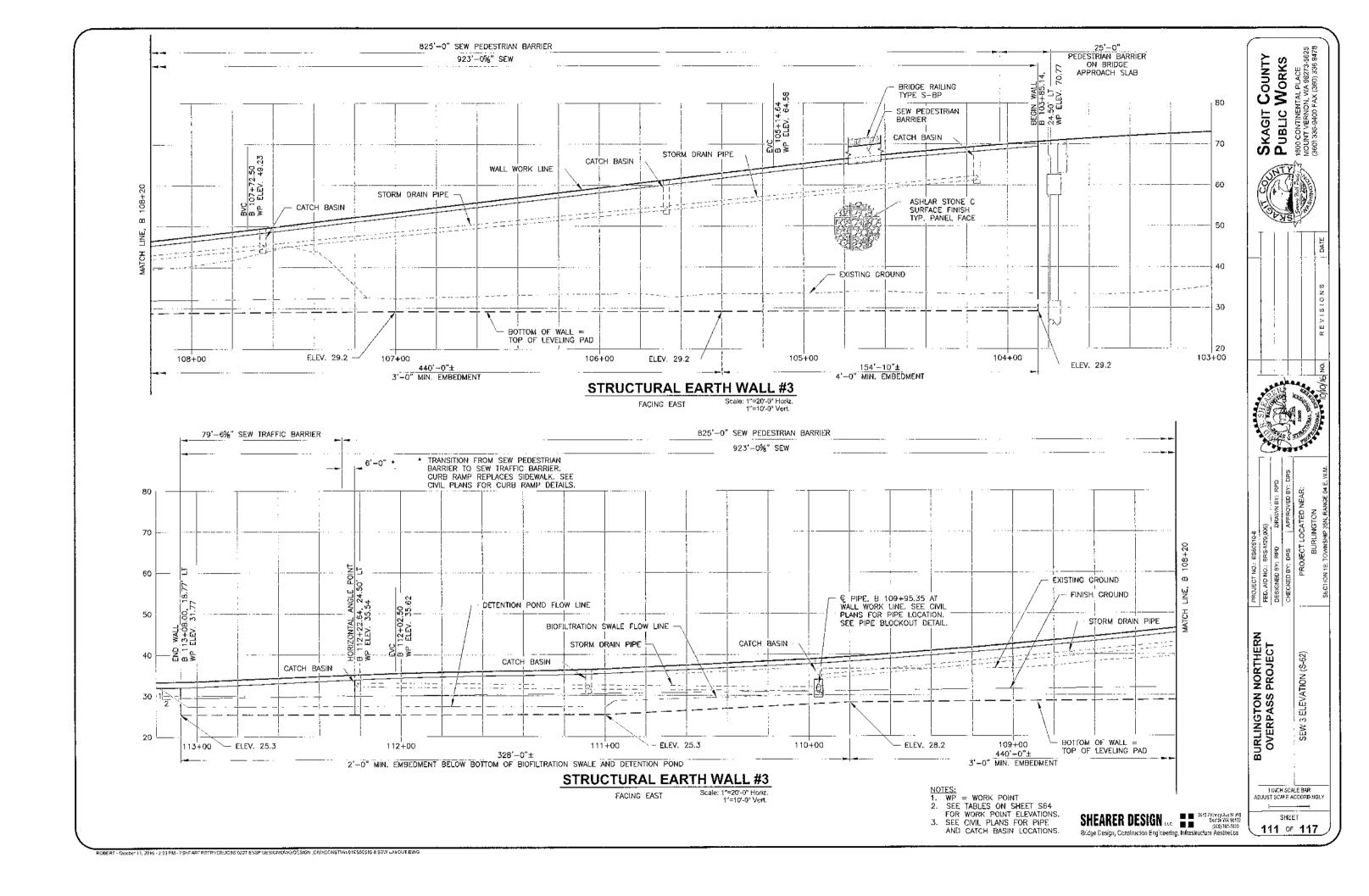
106 of 117

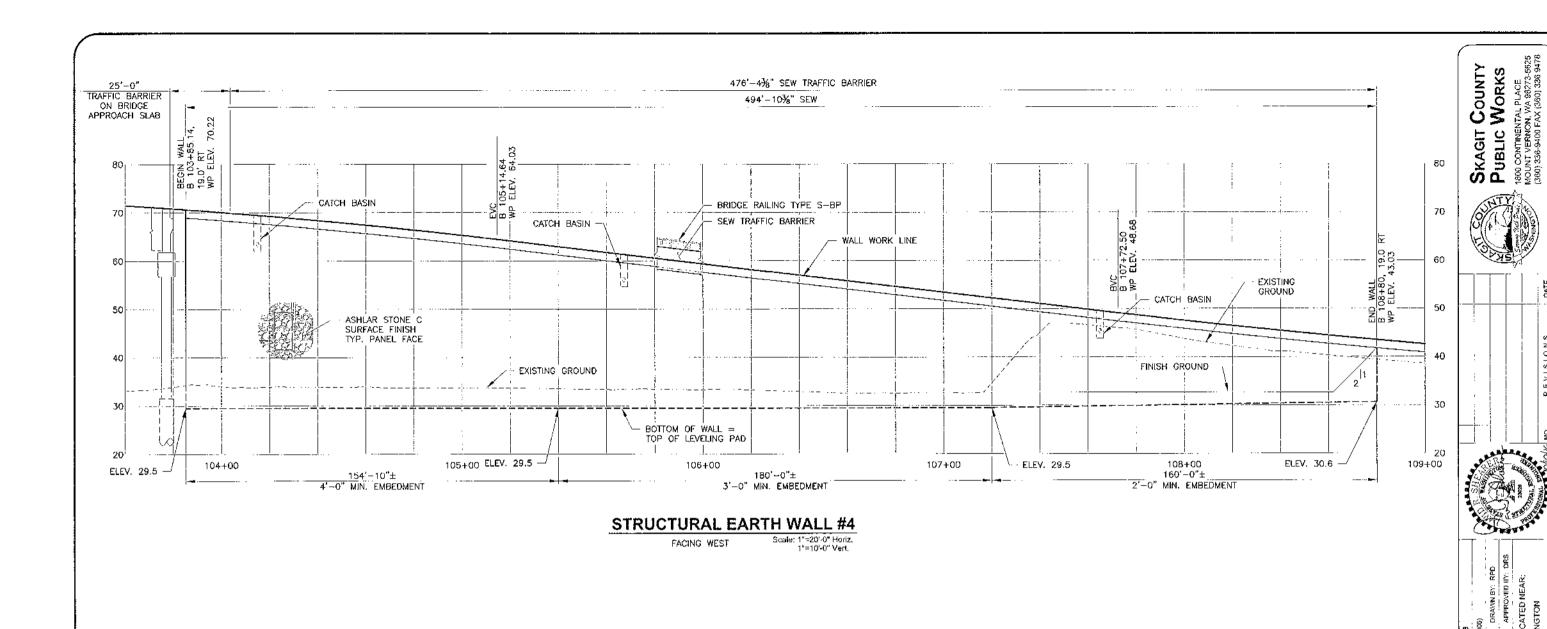




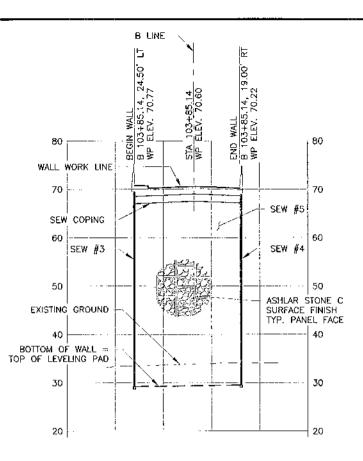




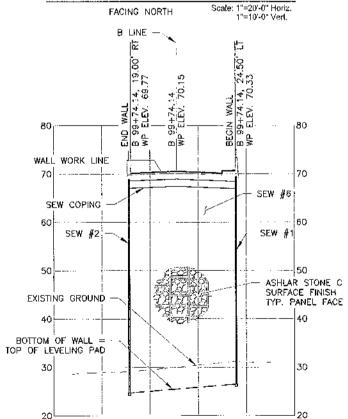




BURLINGTON NORTHERN OVERPASS PROJECT SEW 4 ELEVATION (S-63) NOTES:
1. WP = WORK POINT
2. SEE TABLES ON SHEET S64
FOR WORK POINT ELEVATIONS.
3. SEE CIVIL PLANS FOR PIPE
AND CATCH BASIN LOCATIONS. 1 INCH SCALE BAR ADJUST SCALE ACCORDINGLY SHEARER DESIGN 2543 Princy Act 1.48 South VA 9900 (26) Parker Design ... Construction Engineering, Infrastructure Aesthetics SHEET 112 OF 117



STRUCTURAL EARTH WALL #5



	SEV	V #1	
STATION	WORK POINT ELEVATION	STATION	WORK POINT ELEVATION
B 93+14.30	37.13	B 96+50.00	52 .62
8 93+20.00	37,25	8 96+60.00	53.20
B 93+30.00	37.47	B 96+70.00	53.78
8 93+40.00	37.7J	8 96+80.00	54.36
B 93+42.00	37.76	8 96+90.00	54.94
8 93+50.00	37.96	B 97+00.00	55.53
B 93+60.00	38.22	8 97+10.00	56.11
8 93+70.00	38.50	B 97+20.00	56.69
8 93+80.00	38.80	8 97+30.00	57.27
8 93+90.00	39 ,12	8 97+40.00	57.85
8 94+00.00	39.46	В 97+50.00	58.43
B 94+10.00	39.82	B 97+60,00	59.01
B 94+20.00	40.20	B 97+70,00	59.59
B 94+30.00	40.60	B 97+80.00	60,17
B 94+40.00	41.02	8 97+90,00	60.75
B 94+50.00	41.46	8 98+00.00	61.33
B 94+60.00	41.91	B 98+10.00	61.91
B 94+70.00	42.39	B 98+20.00	62.49
B 94+80.00	42.89	B 98+30.00	63.07
B 94+90.00	43.41	B 98+40.00	63.65
B 95+00.00	43.94	B 98+50.00	64.23
B 95+10.00	44.50	B 98+60.00	64.81
B 95+17.29	44.92	B 98+64.64	65.08
B 95+20.00	45.08	B 98+70.00	65.39
B 95+30.00	45.66	B 98+80.00	65.95
B 95+40.00	46.24	8 98+90.00	66.50
8 95+50.00	46.82	B 99+00.00	67.02
B 95+60.00	47,40	B 99+10.00	67.53
8 95+70.00	47.98	B 99+20.00	68.02
8 95+80.00	48.56	B 99+30.00	68.49
8 95÷90.00	49.14	B 99+40.00	68.95
8 96±00.00	49.72	B 99+50.00	69.38
B 96+10.00	50.30	B 99+60.00	69,80
8 96+20.00	50.88	B 99+70.00	70.20
8 96+30.00	51.46	B 99+74.14	70.36
8 96+40.00	52.04		

SE	W #2
CTATION	WORK POINT
STATION	ELEVATION
8 94+60.00	41.33
B 94+70.00	41.81
B 94+80.00	42.31
B 94+90.00	42.82
B 95+00.00	43.36
B 95+10.00	43.92
8 95+17.29	44.34
8 95+20.00	44.49
8 95+30.00	45.07
8 95+40,00	45.65
8 95+50.00	46.23
B 95+60.00	46.81
8 95÷70.00	47.40
8 95+80.00	47.98
B 95+90.00	48.56
8 96+00.00	49.14
8 96+10.00	49.72
8 96+20.00	50.30
B 96+30.00	50.88
B 96+40.00	51.46
B 96+50.00	52.04
B 96+60.00	52.62
B 96+70.00	53.20
B 96+80.00	53.78
B 96+90.00	54.36
B 97+00.00	54.94
B 97+10.00	55.52
B 97+20.00	56.10
B 97+30.00	56.68
B 97+40.00	57,27
B 97+50.00	57.85
8 97+60.00	58.43
В 97 ⊧70.00	59.01
B 97+80.00	59.59
B 97+90.00	60.17
8 98+00.00	60.75
8 98+10.00	61.33
B 98+20.00	61.91
8 98+30.00	62.49
B 98+40.00	63.07
B 98+50.00	63.65
B 98+60.00	64.23
B 98+64.64	64.50
B 98+70.00	64.31
B 98+80.00	65.37
B 98+90.00	65.92
B 99+00.00	66.44
B 99+10.00	66.95
B 99+20.00	67.44
B 99+30.00	67.91
B 99+40.00	68.36
B 99+50.00	68.80
B 99+60.00	69.22
8 99+70.00	69.61
D 00 - 74 44	CD 27

B 99+74.14

69.77

	SEV	V #3	
STATION	WORK POINT	STATION	WORK POIN
0.103.05.14	ELEVATION	B 108 (EQ 00	ELEVATION 45.04
B 103+85.14	70.80	B 108+50,00	
B 103+90.00	70.62	B 108+60.00	44.55
B 104+00.00	70.24	B 108+70.00	44.07
B 104+10.00	69.85	B 108+80.00	43.61
8 104+20.00	69.43	B 108+90.00	43.16
8 104+30.00	69.00	B 109+00.00	42.73
8 104+40.00	68.55	B 103+10.00	42.30
B 104+50.00	68.08	B 109+20.00	41.89
B 104+60.00	67.59	B 109+30.00	41.49
B 104+70.00	67.09	B 109+40.00	41.11
B 104+80.00	66.56	B 109+50.00	40.74
B 104+90.00	66.02	B 109+60.00	40.38
B 105+00.00	65.46	B 109+70.00	40.03
B 105+10.00	64.88	B 109+80.00	39.70
B 105÷14.64	64.61	В 109+90.00	39.38
B 105+20.00	64.29	B 110+00.00	39.07
B 105+30.00	63.69	B 110+10.00	38.78
B 105+40.00	63.10	B 110+20.00	38.50
B 105+50.00	62.50	B 110+30.00	38.23
B 195+60.00	61.91	B 110+40.00	37.98
B 105+70.00	61.31	B 110+50.00	37.73
B 105+80.00	60.72	B 110+60.00	37.50
B 105+90.00	60.12	B 110+70.00	37.29
B 106+00.00	59.53	B 110+80.00	37.09
B 106+10.00	58.93	B 110+90.00	36.89
8 106+20.00	58.34	B 111+00.00	36.72
B 106+30.00	57.74	B 111+10.00	36.55
8 106+40.00	57.15	B 111+20.00	36.40
8 106+50.00	56.55	B 111+30.00	36.26
8 106+60.00	55.96	B 111+40.00	36.14
B 106+70.00	55.36	B 111+50.00	36.03
8 106+80.00	54.77	B 111+60,00	35.93
8 106+90.00	54.17	B 111+70.00	35.84
8 107÷00.00	53.58	B 111+80.00	35.77
B 107+10.00	52.98	B 111+90.00	35.70
8 107+20.00	52.39	B 112+00.00	35.66
B 107+30.00	51.79	B 112+02,50	35.65
B 107+40.00	51.20	B 112+10.00	35.62
B 107+50.00	50.60		35.58
B 107+50.00	50.01	B 112+20,00	
		B 112+22.64	35.57
B 107+70.00	49,41	B 112+30.00	35.57
B 107+72.50	49.26	8 112+40.00	35.58
B 107+80.00	48.82	8 112+50.00	35.58
B 107+90.00	48.24	8 112+60.00	35.59
B 108+00.00	47.67	B 112+70.00	35.60
B 108+10.00	47.12	B 112+80.00	35.61
B 108+20.00	46.58	B 112+90.00	35.61
B 108+30.00	46.05	B 113+00.00	35.62
B 108+40.00	45.54	B 113+08.00	35.63

SEW #4						
STATION	WORK POINT					
	ELEVATION					
B 103+85.14	70.22					
B 103+90.00	70.04					
B 104+00.00	69.66					
B 104+10.00	69.26					
B 104+20.00	68.85					
B 104+30.00	68.42					
B 104+40.00	67,97					
B 104+50.00	67.50					
B 104+60.00	67.01					
B 104+70.00	66.50					
B 104+80.00	65.98					
B 104+90.00	65.44					
B 105+00.00	64.88					
8 105+10.00	64.30					
B 105+14.64	64.03					
B 105+20.00	63.71					
8 105÷30.00	63.11					
8 105+40.00	62.52					
B 105÷50.00	61.92					
B 105+60.00	61,33					
8 105-70.00	60.73					
B 105÷80,00	60,14					
B 105+90.00	59.54					
B 106+00,00	58.95					
B 106+10.00	58.35					
B 106+20,00	57.76					
B 106÷30.00	57.16					
B 106+40.00	56.57					
B 106+50.00	55.97					
B 105+60.00	55.38					
8 106+70.00	54.78					
B 106+80.00	54.18					
8 106+90.00	53.59					
B 107+00.00	52.99					
8 107+10.00	52.40					
B 107+20.00	51.80					
8 107÷30.00	51,21					
B 107÷40.00	50.61					
B 107÷50.00	50.02					
B 107÷60.00	49.42					
B 107+70.00	48.83					
8 107÷72.50	48.68					
B 107+80.00	48.24					
B 107+90.00	47.66					
B 108+00.00	47.09					
B 108+10.00	46.54					
8 108+20.00	46.00					
B 108+30.00	45.47					
8 108+40.00	44.96					
B 108+50.00	44.46					
B 108+60.00	43.97					
B 108+70.00	43.49					
	43.03					

T N	(SKAGIT COUNTY	Director Money
	(
	- SPARRE		A STATE OF THE STA
		(900)	DESIGNED BY: RPD DRAWN BY: RPD
	PROJECT NO.: ESS0510-8	FED. AID NO.: BRS-1129(006)	DESIGNED BY: RE
	BIJEI INGTON NOBTHERN	OVERPASS PRO IECT	

SEW 5 AND 6 ELEVATION AND ELEVATION TABLES (S-64)

1 INCH SCALE BAR ADJUST SCALE ACCORDINGLY

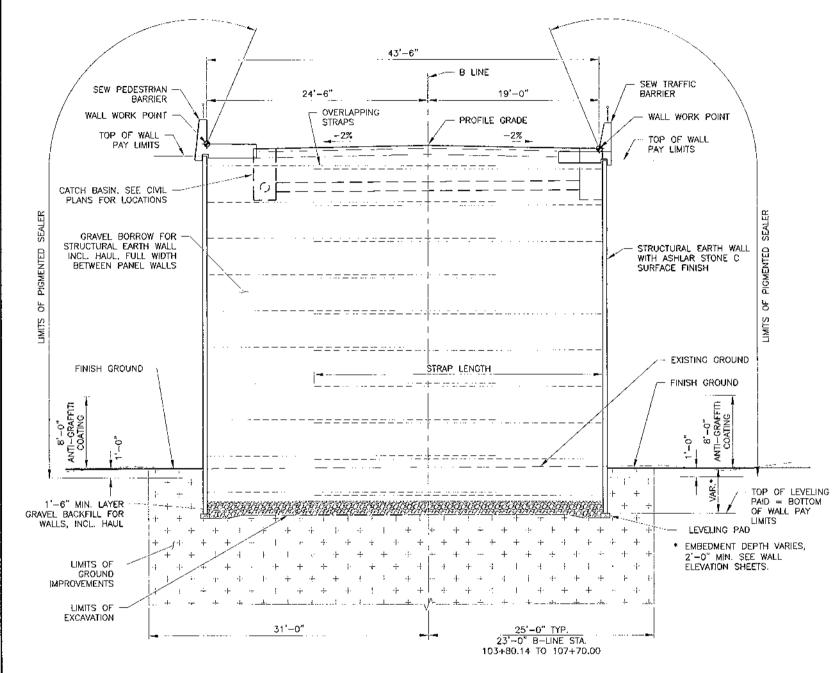
SHEET

113 OF 117

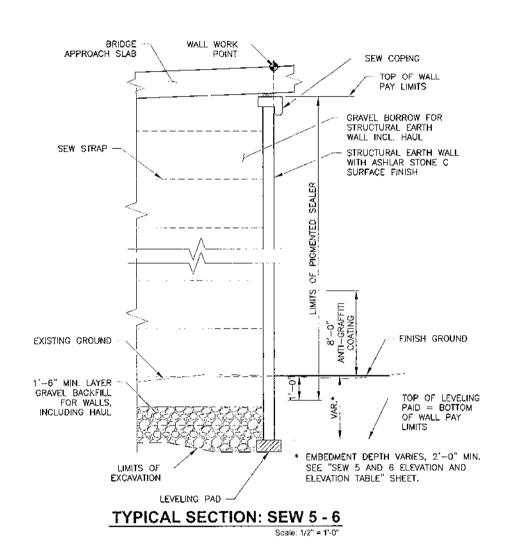
STRUCTURAL EARTH WALL #6

Scale: 1"=20"-0" Horiz. 1"=10"-0" Vert. FACING SOUTH

NOTE: WP = WORK POINT



TYPICAL SECTION: SEW 1 - 4



SHEARER DESIGN CLA STREET STRE SHEET 114 of 117

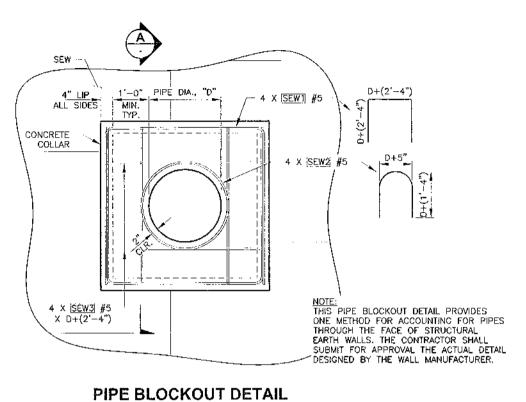
BURLINGTON NORTHERN OVERPASS PROJECT

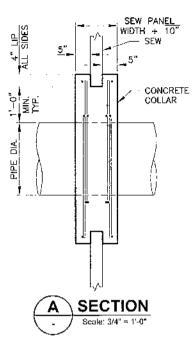
(\$-65)

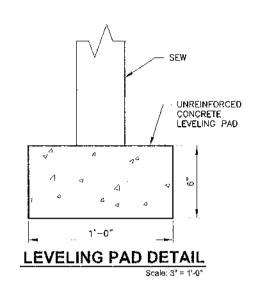
SECTIONS (

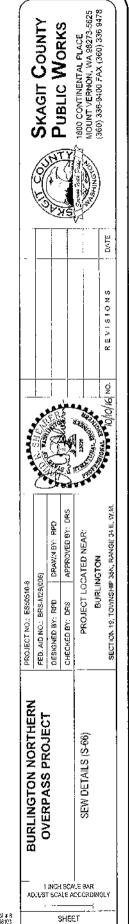
1 INCH SCALE BAR ADJUST SCALE ACCORDINGLY

SKAGIT COUNTY PUBLIC WORKS 1 1800 CONTINENTAL PLAGE MOUNT VERNON, WA 98273-5625 (360) 336-9400 FAX (360) 336 9478



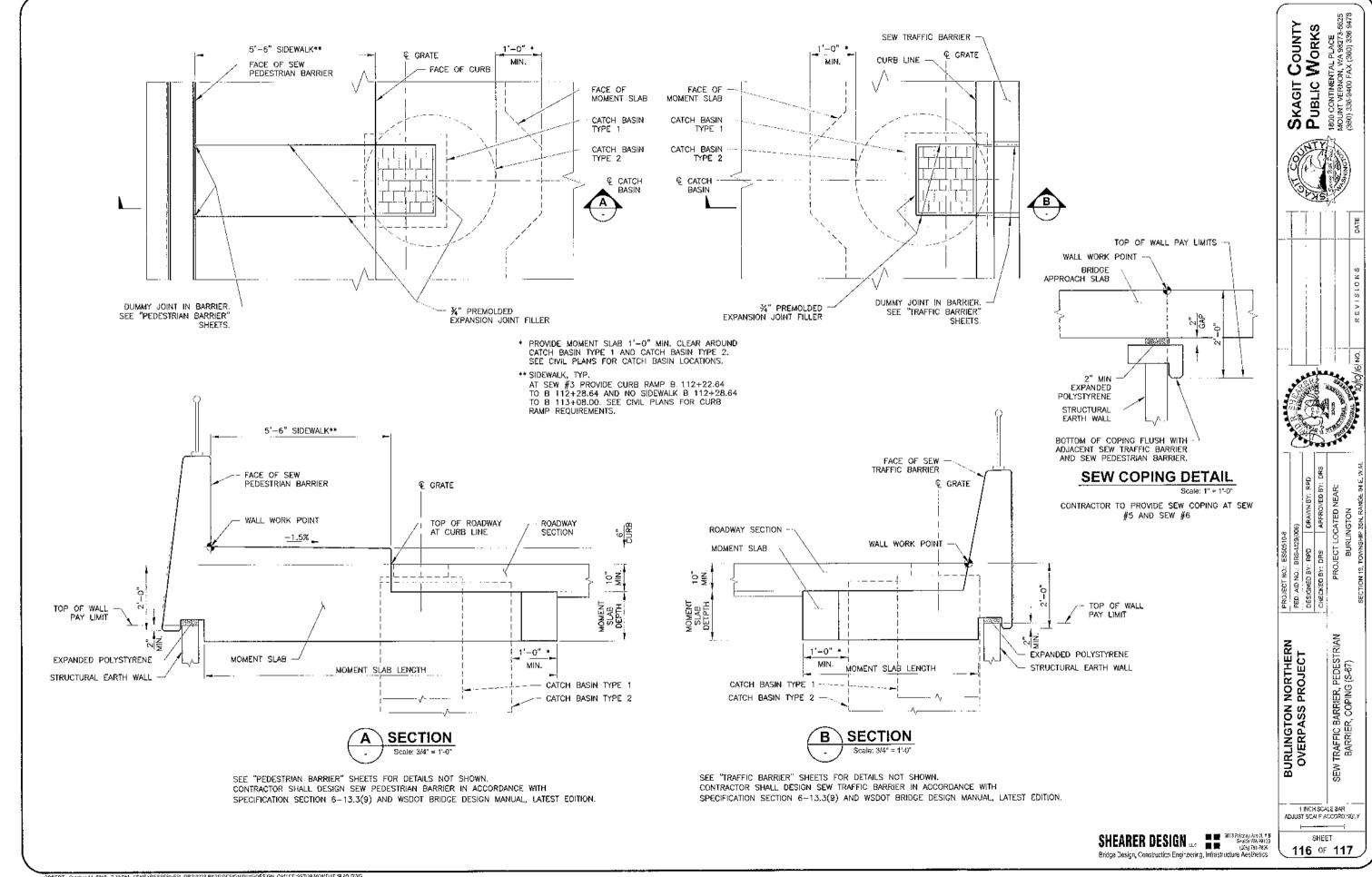


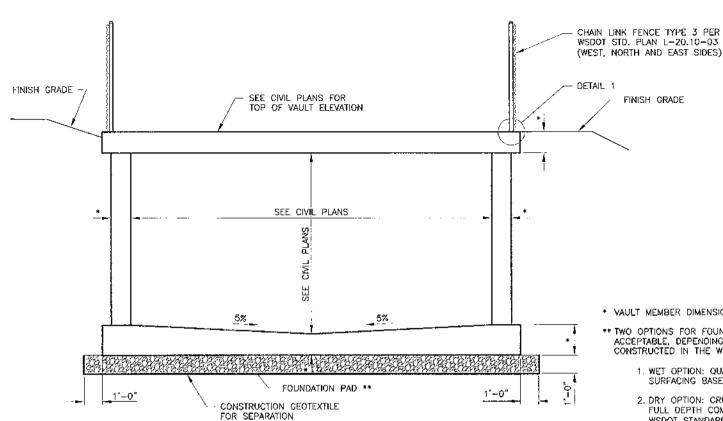




115 of 117

Scale: 3/4" = 1'-0





DETENTION VAULT - TYPICAL SECTION

OUTSIDE PARTIAL ELEVATION

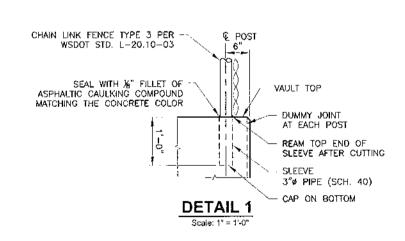
Scale: 3/8" = 1'-0"

* VAULT MEMBER DIMENSIONS PER CONTRACTOR DESIGN.

** TWO OPTIONS FOR FOUNDATION PAD WILL BE ACCEPTABLE, DEPENDING ON WHETHER THE FOOTING IS CONSTRUCTED IN THE WET OR IN THE DRY:

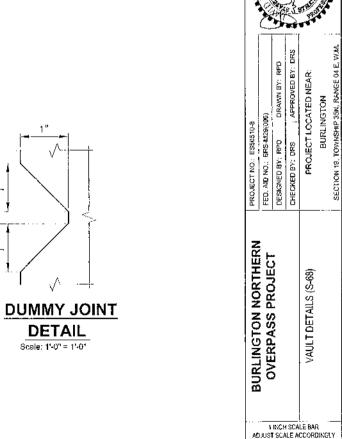
FINISH GRADE

- 1. WET OPTION: QUARRY SPALLS WITH 2"± CRUSHED SURFACING BASE COURSE TO SEAL TOP, OR
- DRY OPTION: CRUSHED SURFACING BASE COURSE FULL DEPTH COMPACTED BY METHOD C PER WSDOT STANDARD SPECIFICATION 2-03.3(14)C.



VAULT GENERAL NOTES

- 1. ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE REQUIREMENTS OF THE WASHINGTON STATE DEPARTMENT OF TRANSPORTATION (WSDOT) 2016 STANDARD SPECIFICATIONS FOR ROAD, BRIDGE, AND MUNICIPAL CONSTRUCTION, AMENDMENTS, AND SPECIAL PROVISIONS.
- 2. THE VAULT SHALL BE CONTRACTOR DESIGNED IN ACCORDANCE WITH THE LINES AND GRADES SHOWN AND THE SPECIFICATIONS.
- 3. PROVIDE 34" CHAMFER ON ALL EXPOSED EDGES OF CONCRETE.
- 4. SEE CIVIL PLANS FOR DETAILS NOT SHOWN.



SKAGIT COUNTY PUBLIC WORKS
1800 CONTINENTAL PLACE MOUNT VERNON, WA 58273-5625 (360) 336-9400 FAX (360) 336 9478

SHEARER DESIGN 11.0. 3919 Priorey Ave 11.48 South 17.4 39103 (2007) 791-7830 Bridge Design, Construction Engineering, Infrastructure Aesthetics

SHEET 117 OF 117

VAULT WALL ----

VAULT BOTTOM -

CHAINLINK FENCE TYPE 3 PER WSDOT STD. PLAN L-20.10-03

DUMMY JOINT AT EACH POST

VAULT TOP